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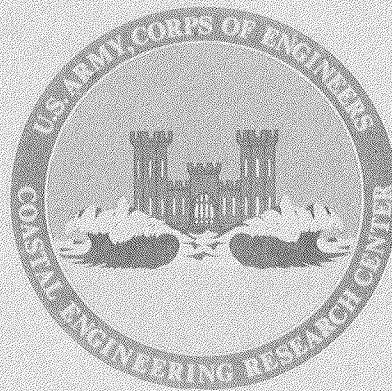
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**Development of Surge II Program With  
Application to the Sabine-Calcasieu Area  
for  
Hurricane Carla and Design Hurricanes**

by

**Robert O. Reid, Andrew C. Vastano, and Thomas J. Reid**

**TECHNICAL PAPER NO. 77-13  
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hurricane surges, is made for the Sabine-Calcasieu region which straddles the Texas and Louisiana boundary. For normal tide conditions, cities such as Beaumont, Orange, and Lake Charles are connected to the sea via rivers, which in the numerical model must be represented as subgrid scale channels as long as the basic grid scale is of the order of a nautical mile. Under hurricane surge conditions, however, the overland flooding can greatly expand their connection to the sea.

Calibration of channel friction is carried out via the astronomical tide simulation. Calibration of the block friction is carried out using data on a previous storm of record, Hurricane Carla. An example application is provided for standard project hurricanes (SPH). The response for a large radius SPH of slow speed and one of moderate speed of translation is examined. Also, the effect of rainfall is examined by running the latter storm with and without rainfall.

## PREFACE

This report is published to assist coastal engineers in the study of storm surge and inland flooding for use in the planning and design of protective coastal works. The work was carried out under the coastal processes program of the U.S. Army Coastal Engineering Research Center (CERC).

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Comments on this publication are invited.

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JOHN H. COUSINS  
Colonel, Corps of Engineers  
Commander and Director

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CONVERSION FACTORS, U.S. CUSTOMARY TO METRIC (SI)  
UNITS OF MEASUREMENT

U.S. customary units of measurement used in this report can be converted to metric (SI) units as follows:

Multiply	by	To obtain
inches	25.4	millimeters
	2.54	centimeters
square inches	6.452	square centimeters
cubic inches	16.39	cubic centimeters
feet	30.48	centimeters
	0.3048	meters
square feet	0.0929	square meters
cubic feet	0.0283	cubic meters
yards	0.9144	meters
square yards	0.836	square meters
cubic yards	0.7646	cubic meters
miles	1.6093	kilometers
square miles	259.0	hectares
knots	1.8532	kilometers per hour
acres	0.4047	hectares
foot-pounds	1.3558	newton meters
millibars	$1.0197 \times 10^{-3}$	kilograms per square centimeter
ounces	28.35	grams
pounds	453.6	grams
	0.4536	kilograms
ton, long	1.0160	metric tons
ton, short	0.9072	metric tons
degrees (angle)	0.1745	radians
Fahrenheit degrees	5/9	Celsius degrees or Kelvins <sup>1</sup>

<sup>1</sup>To obtain Celsius (C) temperature readings from Fahrenheit (F) readings, use formula:  $C = (5/9)(F - 32)$ .

To obtain Kelvin (K) readings, use formula:  $K = (5/9)(F - 32) + 273.15$ .

## SYMBOLS AND DEFINITIONS

A	cross-sectional area of a channel
$A_b$	effective surface area of a block
$A_c$	cross-sectional area of a channel
$A_s$	surface area of an estuary at MSL
$a_o$	amplitude of input tide to an estuary
B	$8/3\pi m(A_s \omega)^2 a_o$ , a parameter which determines the phase lag of tidal response in an estuary
BN	right-hand side of equation (48)
BP	right-hand side of equation (46)
b	$(\partial A / \partial s)_H$ const., a characteristic of a channel
$C_d$	dimensionless discharge coefficient characterizing a constricted opening between bay and sea
$C_g$	admittance coefficient (with dimensions of velocity); nominally represents the wave speed in the sea
$C_o$	dimensionless overflow coefficient (generally less than 0.5 for a broad-crested barrier)
$C_s$	dimensionless discharge coefficient for a submerged barrier (generally less than $\sqrt{2}$ )
D	total depth of water at position $x, y$ at time $t$
$\bar{D}$	a mean depth for the effective fetch across a block; also mean depth for a channel $(D_N + D_p)/2$
$D_b$	depth of water over the crest of a barrier
$D_c$	effective depth of a channel $A_c/w$
$D_{max}$	maximum depth to be expected anywhere in the system during a storm surge
$F_L$	contribution to the forcing term in equation (17) due to lateral transfer of mass and momentum
f	dimensionless bed resistance coefficient for blocks
$f_c$	channel bed friction coefficient

### SYMBOLS AND DEFINITIONS--Continued

G	damping factor for channels, see equation (44)
$G_1$	damping factor for x-transport on blocks, see equation (35)
$G_2$	damping factor for y-transport on blocks, see equation (36)
g	acceleration due to gravity
H	water level elevation relative to local MSL datum
HB	water elevation on the water-connected block of a channel
HC	common water elevation for a channel junction
HM	mean water level anomaly of connected channel and blocks
HX	water level at the lower end of an x-channel
HY	water level at the left end of a y-channel
$H_A$	$H$ at point B in a channel
$H_b$	water level on the high side of a barrier
$H_g$	input tide level at time $t$ outside a bay entrance
$H(i,j)$	water level anomaly $H$ for block identified by x and y indexes $i,j$
$H^*$	tentative predicted $H$ for a ponding block in the absence of any contribution by longitudinal discharge to or from the channel which terminates adjacent to that block
$H'$	value of $H$ at new time level
$H'_P$	new $H$ value at point P in channel
$H_1 \triangleq H_2$	water levels on the two sides of a barrier (both of which exceed $Z_b$ ), equation (10)
i	x-index for grid blocks
j	y-index for grid blocks
K	dimensionless wind-stress coefficient, equation (6)
L	effective fetch length

### SYMBOLS AND DEFINITIONS--Continued

$L_f$	net time rate of gain of water volume per unit distance along the channel by lateral transfer and rainfall
$L_m$	net time rate of gain of momentum (divided by water density) per unit distance along channel
$m$	$fL/gD_C A_C^2$ or $1/g(C_d A_d)^2$
$N$	denotes negative characteristic
$n$	time index
$P$	wind "push" term $X\Delta t$ or $Y\Delta t$ ; also denotes positive characteristic
$Q$	volume transport through cross-sectional area of a channel
$\bar{Q}$	mean $Q$ value for channel, equation (45)
$QCXP_K$	flow at the upper end of an x-channel for channel block $K$
$QCYN_K$	flow at the left end of a y-channel for channel block $K$
$QCYP_K$	flow at the right end of a y-channel for channel block $K$
$QCXN_K$	flow at the lower end of an x-channel for channel block $K$
$Q_A$	$Q$ at point $A$ of positive characteristic
$Q_B$	$Q$ at point $B$ of negative characteristic
$Q_d$	discharge from channel to ponding block
$q_f$	the flow (per unit length of channel) from the channel to the adjacent block
$q_i$	lateral volume flux per unit length into the channel
$q_n$	outward component of volume flux at a boundary
$q_o$	lateral volume flux per unit length out of the channel
$q_t$	flow (per unit length of channel) from the channel block to the channel (across the interior side of the channel)
$Q'$	new $Q$ value
$Q'_N$	new $Q$ at point $N$

SYMBOLS AND DEFINITIONS--Continued

$Q_p$	new $Q$ at point $P$
$Q_r^!$	specified river discharge
$R$	rainfall rate
$R(i,j)$	rainfall rate for block $i,j$
$r$	relative amplitude response
$s$	distance along the axis of a channel
$T$	tidal period
$T_s$	longitudinal component of wind stress (divided by water density) or appropriate wind-stress component ( $X$ or $Y$ ) corresponding to time level $t$ for the associated channel block
$t$	time
$U$	vertically integrated $x$ -component of volume transport per unit width
$UCF(K)$	lateral transport, per unit width per unit time, nominally from an $x$ -channel of block $K$ to an adjacent block; also denoted $UCF_K$
$UCT(K)$	lateral transport, per unit width per unit time, nominally to an $x$ -channel from the interior of block $I$ ; also denoted $UCT_K$
$U_N$	$U$ value on left side of block
$U(i,j)$	value of $U$ at the left side of block $i,j$
$U(i+1,j)$	value of $U$ at the right side of block $i,j$
$u$	typical fluid speed in the bay
$U'$	value of $U$ at new time level
$V$	vertically integrated $y$ component of volume transport per unit width
$VCF(K)$	lateral transport per unit width per unit time, nominally from an $y$ -channel of block $K$ to an adjacent block; also denoted $VCF_K$

SYMBOLS AND DEFINITIONS--Continued

VCT(K)	lateral transport per unit width per unit time, nominally to an y-channel from the interior of block K; also denoted $VCT_K$
$V_{N_I}$	value of V at the lower side of a block
$V(i,j)$	value of V at the lower side of block i,j
$V(i,j+1)$	value of V at the upper side of block i,j
$V'$	value of V at new time level
$W$	windspeed at 10-meter elevation over the water
$W_c$	a critical speed taken as 14 knots (7 meters per second)
w	surface width of a channel (conveyance width)
X	x-component of the wind stress divided by the density of the water
$X(i+1,j)$	value of X for right side of block i,j
x	horizontal Cartesian coordinate nominally alongshore, positive to the right when facing shore
Y	y-component of the wind stress divided by the density of the water
$Y(i,j+1)$	value of Y for top side of block i,j
y	horizontal Cartesian coordinate nominally normal to shore, positive landward
Z	elevation of the seabed relative to MSL datum
$Z(i,j)$	value of Z for block i,j
$Z_b$	barrier crest elevation
$Z_c$	channel bed elevation
$\alpha$	$(gD)^{1/2} \Delta t / \Delta s$ (Courant number); also $L_c/D_c A_c$ , equation (77)
$\Gamma$	$L(C_b D_b)^2 / \overline{D} \Delta t$
$\Delta H$	a head differential dependent upon barrier type
$\Delta q$	net lateral flow to the channel per unit length of channel

SYMBOLS AND DEFINITIONS--Continued

$\Delta s$	grid size for blocks (distance between successive H values in both the x and y directions); also written $\Delta S$ or DELS
$\Delta t$	time step (time interval between successive H values at given location); also written DELT
$\theta$	the angle between the wind velocity vector and the x-axis
$\lambda$	$w (gD)^{1/2}/G$
$\pi$	3.14159 ...
$\sigma$	$wf  Q /A^2$
$\phi$	latitude
$\Omega$	absolute angular speed of the earth
$\omega$	radian frequency $2\pi/T$

DEVELOPMENT OF SURGE II PROGRAM WITH APPLICATION TO THE  
SABINE-CALCASIEU AREA FOR HURRICANE CARLA AND DESIGN HURRICANES

by

*Robert O. Reid, Andrew C. Vastano, and Thomas J. Reid*

I. INTRODUCTION

Numerical techniques for the solution of equations representing storm surges in coastal areas were significantly augmented in 1966 by the development of a two-dimensional model (referred to in this study as the SURGE I program) for the U.S. Army Engineer District, Galveston (Reid and Bodine, 1968). At about the same time a number of bay models emerged. Notable among these are the models of Leenderste (1967) and Masch, et al. (1969), which have been applied to problems of both surge and circulation in bays. These models include the Coriolis force which is neglected in the Reid-Bodine model. However, the Reid-Bodine model produced the first successful inclusion of flooding, recession, barriers, and flow over barriers in the study of inundation of low-lying coasts. The actual model is a nonlinear system of equations and boundary conditions solved by numerical integration of time-dependent, forced motion. Its use produces the water response to stormwinds over the region for a given storm tide at the seaward boundary. The initial application was a hindcast of the Hurricane Carla surge generated in Galveston Bay during 9 to 12 September 1961.

During Hurricane Carla, the wetted perimeter of Galveston Bay essentially doubled, as accurately reproduced in the hindcast computations. Serial observations of water levels for the storm period available from stations throughout the bay were compared to levels computed with the numerical algorithm. These records produced a standard deviation of less than 4 inches, overall. The maximum deviation of the water level prediction was 1.5 feet and occurred at the grid square corresponding to the location of the Pelican Island Bridge which spans the channel between Galveston and the Pelican Islands. Although this disparity was relatively large, its effect on the computations was effectively reduced by the smoothing operation of the numerical integration. However, this difference points out a basic problem confronting any model--the minimum definition of topographic features.

The basic problem of indicating subgrid scale effects in numerical modeling is normally solved by parameterization of the omitted physical mechanism. Often, an analytic relationship is introduced that requires the specification of empirically derived constants; e.g., the wind-stress equation for the transfer of momentum from wind to water. Another simple and pertinent instance is the *a priori* rotation of wind vectors over certain grid squares in the Hurricane Carla computations for Galveston Bay. The model Galveston entrance channel was not in the proper orientation on the Cartesian numerical grid system and, as a result, did not admit a realistic amount of water to the bay. A programmed shift in the wind vectors indicated this subgrid scale feature.

SURGE I has been applied to the study of Texas coastline surge susceptibility. The topographic features of this region are characterized by barrier islands and shallow, river-fed bay systems surrounded by near sea level land and marshes. The specific applications of the program have therefore centered interest on the immediate environs of a bay. The requirement for surge studies of appreciable distances inland from the bay system has only recently been placed on the numerical model. The propagation of the surge to higher ground through necessary subgrid scale topographic features has required an extension of the basic algorithm.

The new algorithm developed for the study of the Sabine-Calcasieu region is referred to as the SURGE II program. This program incorporates all the features of SURGE I with the further option of representing variable depth and width channels along the sides of each grid square. The flow computations for the channels interact with the normal grid square computations and permit a complete suite of flooding conditions for overtopping of levees. In this manner SURGE II provides a time-dependent, subgrid scale transport of water through the model.

## II. THEORETICAL DEVELOPMENT FOR SURGE II

### 1. Summary of Two-Dimensional Theory.

The development of SURGE II was based on the SURGE I concept by Reid and Bodine (1968). A part of this study is presented here to provide a complete description of SURGE II.

The advection of momentum (or field acceleration) is considered negligible except at singular regions of the bay (submerged barriers and narrow channels) where the effect is included implicitly through the use of appropriate nonlinear discharge relations. The effect of the earth's rotation is also neglected; this approximation appears justifiable for systems of small spatial scale and shallow depth where frictional forces are more dominant.

Within the normal domain of the bay and immediate adjoining sea, the vertically integrated equations of motion and of continuity appropriate to the problem are taken as follows:

$$\frac{\partial U}{\partial t} + gD \frac{\partial H}{\partial x} = X - fqUD^{-2} \quad (1)$$

$$\frac{\partial V}{\partial t} + gD \frac{\partial H}{\partial y} = Y - fqVD^{-2} \quad (2)$$

$$\frac{\partial H}{\partial t} + \frac{\partial U}{\partial x} + \frac{\partial V}{\partial y} = R, \quad (3)$$

where

$x$  and  $y$  = horizontal Cartesian coordinates;

$t$  = time;

$U$  and  $V$  = vertically integrated  $x$  and  $y$  components, respectively, of transport per unit width;

$g$  = gravity;

$H$  = water level elevation relative to the local mean sea level (MSL) datum;

$D$  = depth of water at position  $x$ ,  $y$  at time  $t$ ;

$q$  = magnitude of the transport per unit width;

$f$  = dimensionless bed-resistance coefficient;

$R$  = rainfall rate;

$X$  and  $Y$  =  $x$  and  $y$  components of the wind stress divided by the density of the water (the density assumed constant).

Normal values of  $f$  are in the range  $10^{-3}$  to  $10^{-2}$  for typical seabed conditions.

The value of  $q$  is obtained from  $U$  and  $V$  by

$$q = (U^2 + V^2)^{1/2} \quad (4)$$

which is a positive quantity.

The kinematic forms of the wind-stress components in the absence of rainfall are taken as

$$X = K W^2 \cos \theta$$

$$Y = K W^2 \sin \theta, \quad (5)$$

where  $W$  is the windspeed at a 10-meter elevation over the water, and  $\theta$  is the angle between the wind velocity vector and the  $x$ -axis. The dimensionless coefficient,  $K$ , used in the calculations is presumed to be a function of windspeed as implied by the van Dorn (1953) relation for wind stress. Specifically, it is assumed that

$$K = K_1 \quad \text{for } W \leq W_c$$

$$K = K_1 + K_2 \left(1 - \frac{W_c}{W}\right)^2 \quad \text{for } W \geq W_c, \quad (6)$$

where the constants  $K_1$  and  $K_2$  are taken as  $1.2 \times 10^{-6}$  and  $1.8 \times 10^{-6}$ , respectively, and  $w_c$  is a critical speed which is taken as 14 knots (7 meters per second). For large windspeeds,  $K$  approaches the limiting value of  $3.6 \times 10^{-6}$  which corresponds to a resistance coefficient of about  $3.0 \times 10^{-3}$  if the ratio of air density to water density is taken as  $1.2 \times 10^{-3}$ .

In the presence of rainfall an added flux of momentum proportional to  $RW$  occurs (van Dorn, 1953). The effect can be included by augmenting  $K$  by  $R/W$ . For heavy rainfall, the resulting  $K$  is increased about 10 percent.

The variables  $H$  and  $D$  are related by the simple expression,

$$D = H - Z , \quad (7)$$

where  $Z$  is the elevation of the seabed relative to the MSL datum. Presumably,  $Z$  is a function of  $x$  and  $y$  only; i.e., the time-dependent scour of the seabed is ignored.

The above equations ignore the direct effect of variable atmospheric pressure which is relatively minor in a small, shallow bay. The effect over the sea is included implicitly through the specification of an appropriate surge height versus time in the adjoining sea where the combined effects of winds and differential atmospheric pressure give rise to a coastal storm surge. This is presumed to be determined independently of the detailed calculations for the bay and enters as a boundary condition.

a. Boundary Conditions. Four different types of boundary conditions are used in this system of computations. Two of these conditions apply to the water-land boundary, one condition applies to the artificial boundary representing the seaward end of the bay system, and one applies at partial barriers internal to the system. (Additional internal conditions are needed in the presence of imbedded channels as discussed later in Section III,2.) All four conditions relate the normal component of flow at the boundary to the state of the water level at the boundary.

In general, the boundary between bay water and land depends on the water elevation and the land topography. The shoreline for different uniform elevations of the surface of the bay is readily established from a knowledge of the topography. For a bay with low-lying terrain, the rate of increase of surface area of water per unit increase of water level can be considerable. In the actual rising stage of storm tide the amount of inundation is controlled by the rate at which the water can flow into the potential ponding areas. In the present scheme, which uses a representation of the bay in terms of a discrete grid, the elevation of the seabed or land is regarded as uniform over each grid square, thus forming a two-dimensional, staircase-type approximation of the actual topography. The boundary condition on the normal component of flow,  $q_n$ , at the juncture of a flooded square and a dry square is taken as

$$q_n = 0, \quad (8)$$

if the elevation,  $H$ , of the water is less than that of the adjacent dryland. However, if the water level is greater than that of the dryland, then the rate of flooding,  $q_n$ , per unit length of land barrier, is given by

$$q_n = \pm C_o D_b (g D_b)^{\frac{1}{2}}, \quad (9)$$

where  $D_b$  is water depth over the crest of the barrier, and  $C_o$  is an appropriate dimensionless overflow coefficient, generally less than 0.5 for a broad-crested barrier. The choice of sign depends on whether the flooding is from bay to land or from flooded land back to the bay during the recession stage.

Equation (9) is considered valid for any barrier within or at the boundary of the system for which the water level on one side of the barrier is greater than the barrier crest elevation,  $Z_b$ , and for which the water level on the other side is less than  $Z_b$ . Moreover,  $D_b$  is simply  $H_b - Z_b$ , where  $H_b$  is the water level on the high side.

In the case where the water level on both sides of an internal barrier exceeds the barrier-crest elevation, the discharge is taken as that for a submerged wier,

$$q_n = \pm C_s D_b (g |H_1 - H_2|)^{\frac{1}{2}}, \quad (10)$$

where  $D_b$  is the water depth over the crest of the barrier,  $H_1$  and  $H_2$  are the water levels on the two sides of the barrier (both of which exceed  $Z_b$ ), and  $C_s$  is an appropriate dimensionless discharge coefficient for the submerged barrier (generally less than  $\sqrt{2}$ ). In this case,  $D_b$  is taken as  $(H_1 + H_2)/2 - Z_b$ . Again, the sign is taken such that the flow is directed toward the low-head side of the barrier. Both equations (9) and (10) presume that the velocity of approach to the barrier is much less than the velocity over the barrier.

In the numerical computational scheme, emphasis is placed on the evaluation of flow and water levels within a bay which is connected to a sea of essentially unlimited extent. An appropriate boundary condition is required either at the mouth of the bay system or along some line within the sea which delineates the outer limit of the computational grid. The correct approach would be to treat the development of the surge in the sea and bay as a single problem. However, the difference in spatial resolution required for the two different regions of the system, as well as computer storage limitations, makes this impractical. The assumption is made that the effect of the conditions in the bay has only a minor influence on the development of the surge in the sea and over the Continental Shelf. The evaluation of the latter can be determined independently of the bay problem or obtained from observation and used as an outer boundary condition for the bay.

The simplest condition at the seaward boundary is of the form

$$H = H_g , \quad (11)$$

where  $H_g$  is the prescribed water level which would exist in the absence of the bay at time  $t$  at the outer boundary of the bay system. SURGE II presently uses this condition at the seaward boundary and at lateral boundaries on the limited shelf part of the system. An alternative condition for the lateral boundaries on the shelf is to prescribe that  $\partial U / \partial x = 0$  at these boundaries where  $x$  is taken alongshore (Jelesnianski, 1966, 1967). An alternative condition for the seaward boundary is one which allows for radiation of energy to the sea. The latter condition is of the form

$$H = H_g + q_n / C_g , \quad (12)$$

where  $q_n$  is taken positive outwards from the bay to the sea, and  $C_g$  is an appropriate admittance coefficient (with dimensions of velocity). Nominally,  $C_g$  represents the wave speed in the sea. The generalized condition (eq. 12) is nearly equivalent to the simplest condition (eq. 11) if  $C_g$  greatly exceeds the wave speed for the bay.

b. Initial Conditions. Since the system includes allowance for frictional dissipation as well as radiation of energy, the solution for given fields of  $X$  and  $Y$  and given boundary function,  $H_g$ , should be reasonably insensitive to the nature of the initial conditions after a suitable lapse of time from the initial state. Thus, the initial conditions can be somewhat arbitrary. As in the laboratory model experiments, it is reasonable to start from a state of equilibrium in which  $U$  and  $V$  are zero and  $H$  is uniform throughout the system, in order to minimize the introduction of transient oscillations related to the starting conditions. Moreover, a reasonable period (depending on the characteristic decay time) can be allowed for the system to reach that state where its response reflects only the effect of the forcing functions.

## 2. Theory of Embedded Channels.

Let  $s$  denote distance along the axis of a channel whose cross-sectional area is  $A$  and surface width is  $w$  at position  $s$  and time  $t$ . Let  $Q$  be the volume transport through  $A$  in the positive sense of  $s$ , and let  $H$  be the water elevation above MSL datum at the same section. In general,  $A$  and  $w$  are known functions of  $H$  for a given cross section, as determined by the geometry of the cross section (Fig. 1). In particular,  $\partial A / \partial H = w$  for given  $s$ . The width  $w$  is to be the "conveyance" width, as used by Dronkers (1964).

The channel is considered an "open system" in the sense that water and momentum may enter or leave the channel laterally; i.e., exchange of fluid with adjacent bay area or flooded land can exist. If the longitudinal velocity in the channel is considered uniform for evaluating the

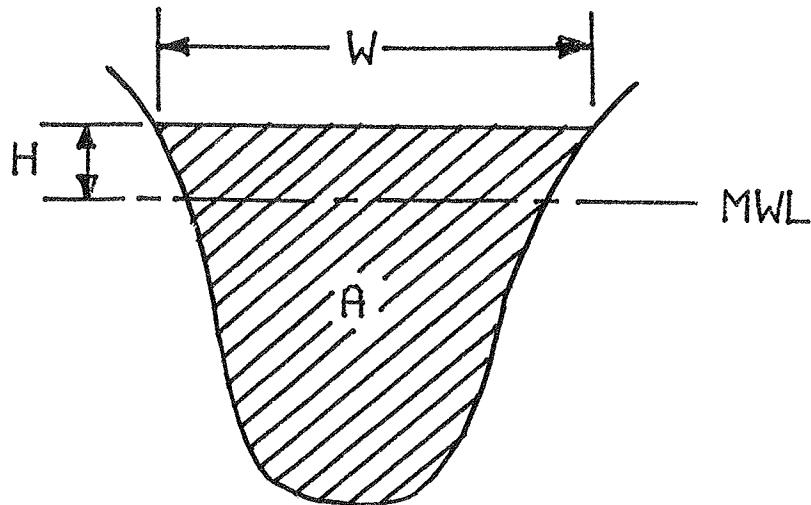


Figure 1. Schematic channel cross section showing pertinent parameters.

longitudinal transport of momentum, then the equations of motion and continuity for a given channel reach are (Stoker, 1957, Ch. 11; Dronkers, 1964, Ch. 9)

$$\frac{\partial Q}{\partial t} + \frac{\partial}{\partial s} (Q^2/A) + gA \frac{\partial H}{\partial s} = wT_s - \sigma Q + L_m \quad (13)$$

$$\frac{\partial A}{\partial t} + \frac{\partial Q}{\partial s} = L_f, \quad (14)$$

where

$T_s$  = longitudinal component of wind stress (divided by water density);

$\sigma = wf|Q|/A^2$  where  $f$  is a dimensionless channel-friction coefficient;

$L_f$  = net time rate for gain of water volume per unit distance along the channel by lateral transfer and rainfall;

$L_m$  = associated net time rate of gain of momentum (divided by water density) per unit distance along channel.

The units of  $L_f$  are square feet per second;  $L_m$  has the units cubic feet per second squared.

It is convenient in the analysis of the channel dynamics to transform the above equations into a characteristic form. There are several different possible characteristic forms. The approach used by Stoker (1957) is to work with  $u$  and  $H$  (where  $u \equiv Q/A$ ) as the dependent variables. Dronkers (1964) works with either  $Q$  and  $H$  directly or with  $Q$  and total head ( $H + (Q/A)^2/2g$ ). Each method has certain

advantages and disadvantages. In the present analysis, the variables  $Q$  and  $H$  are used to be as consistent as possible with the computations in the two-dimensional regions of the system.

In transforming equations (13) and (14) to characteristic form, it is noted that

$$\begin{aligned}\frac{\partial A}{\partial t} &= w \frac{\partial H}{\partial t} \\ \frac{\partial A}{\partial s} &= w \frac{\partial H}{\partial s} + b,\end{aligned}\quad (15)$$

where

$$b \equiv \left( \frac{\partial A}{\partial s} \right)_{H \text{ const}}. \quad (16)$$

(For a channel of uniform cross section the latter quantity would be zero.) It can be shown, following Dronkers' (1964) analysis and considering equation (15), that a characteristic form of equations (13) and (14) is

$$\frac{dQ}{dt} + w \left( -\frac{Q}{A} \pm \sqrt{\frac{gA}{w}} \right) \frac{dH}{dt} = \left\{ wT_s - \sigma Q + L_m + b \left[ \left( \frac{Q}{A} \right)^2 + \left( -\frac{Q}{A} \pm \sqrt{\frac{gA}{w}} \right) L_f \right] \right\} \quad (17)$$

along the path  $s(t)$  where

$$\frac{ds}{dt} = \frac{Q}{A} \pm \sqrt{\frac{gA}{w}}. \quad (18)$$

The path line where the plus or minus sign is taken in equation (18) is referred to as the positive P characteristic or the negative N characteristic path, respectively. These are illustrated in Figure 2 where  $x$  corresponds to  $s$ , the two paths having point C in common. Equation (17) with the upper sign applies along P and equation (17)

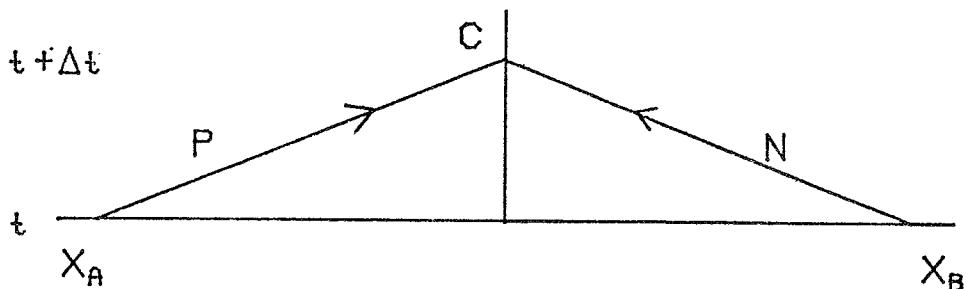


Figure 2. Schematic positive and negative characteristic paths to a common point in the  $x$ ,  $t$  diagram.

with the lower sign applies on path N. Thus, information with regard to Q and H at points  $x_A$  and  $x_B$  at time t and along the two paths can, in principle, be used to predict the values of Q and H at point C from two equations.

For a laterally closed channel ( $L_f, L_m = 0$ ) of a uniform cross section ( $b = 0$ ) without friction ( $\sigma = 0$ ), in the absence of wind stress ( $T_s = 0$ ), then the quantity in braces on the right-hand side of equation (17) vanishes. In this case only the information at points A and B of Figure 2 is needed to predict values of H and Q at C. To show that equation (17) is consistent with Stoker's (1957) analysis for this special case, let  $u = Q/A$  and  $D = A/w$ . For a uniform cross section at given H,  $dH/dt = dD/dt$ , so equation (17) reduces to .

$$\frac{d(DU)}{dt} + (-u \pm \sqrt{gD}) \frac{dD}{dt} = 0 \quad (19)$$

along

$$\frac{ds}{dt} = u \pm \sqrt{gD}. \quad (20)$$

Equation (19) simplifies further to

$$wD \frac{d}{dt} (u \pm 2\sqrt{gD}) = 0. \quad (21)$$

Thus, for this special case  $(u \pm 2\sqrt{gD})$  is conserved along P where  $dx/dt = u + \sqrt{gD}$ , while  $(u - 2\sqrt{gD})$  is conserved along N where  $dx/dt = u - \sqrt{gD}$ . Thus, u and D (hence, Q and H) can readily be evaluated at C.

In the more general case the time integral of the right-hand side of equation (17) must be estimated in a rational way. This is considered later in Section III,2. Also, in the general case it is usually not possible to put the left-hand side of equation (17) in the simple form shown in equation (21).

a. Lateral Transfer Terms. In the absence of direct rainfall,  $L_f$  must equal the net gain of volume per unit length per unit time due to lateral flow into the channel on either or both sides. Let  $q_i$  and  $q_o$ , respectively, represent the volume fluxes per unit length into and out of the channel. Then,  $L_f = q_i - q_o$  in the absence of rainfall, or

$$L_f = q_i - q_o + wR \quad (22)$$

with rainfall. The corresponding lateral transfer of momentum (divided by water density) is

$$L_m = q_i u_i - q_o u_o, \quad (23)$$

the transfer from rainfall being included in the wind-stress term as discussed in Section II,1. In equation (23) the quantity  $u_o$  is simply  $Q/A$  for the channel while  $u_i$  is the channel-directed component of velocity of fluid from the adjoining block water area. In equation (17) the terms  $L_m$  and  $L_f$  contribute to the right-hand side the quantity,

$$F_L \equiv L_m - \frac{Q}{A} L_f \pm \sqrt{\frac{gA}{w}} L_f . \quad (24)$$

Using equations (22) and (23) yields

$$F_L = q_i (u_i - u_o) - wR u_o \pm \left( \frac{gA}{w} \right)^{\frac{1}{2}} (q_i - q_o + wR) . \quad (25)$$

The lateral flows into or out of the channel can be evaluated by relations such as equations (8), (9), and (10). This is also discussed in Section III,2.

b. Simplifications. The SURGE II program uses certain simplifications of the above equations. For normal conditions, the propagational speed  $(gA/w)^{\frac{1}{2}}$  significantly exceeds the speeds  $u_i$  or  $u_o$ ; i.e.,  $Q/A$ . Accordingly,  $F$  is approximated by

$$F_L = \pm \left( \frac{gA}{w} \right)^{\frac{1}{2}} L_f . \quad (26)$$

Elsewhere in equations (17) and (18),  $Q/A$  is neglected compared with  $(gA/w)^{\frac{1}{2}}$ . Moreover, each channel reach within a grid block is considered of uniform width and bottom elevation  $Z_c$ ; however,  $w$  and  $Z_c$  vary from one reach to another. Thus,  $b = 0$  for each reach and

$$A/w = D = H - Z_c . \quad (27)$$

Under these conditions equations (17) and (18) take the form,

$$\frac{dQ}{dt} \pm w\sqrt{gD} \frac{dH}{dt} = \{ wT_s - f |Q| Q / (D^2 w) \pm \sqrt{gD} (q_i - q_o + wR) \} \quad (28)$$

along

$$\frac{ds}{dt} = \pm \sqrt{gD} \quad (29)$$

where  $T_s = X$  or  $Y$  as  $s = x$  or  $y$ , depending on channel orientation. Equation (28) can also be expressed in the form,

$$\frac{d}{dt} (Q \pm \frac{2}{3} wD\sqrt{gD}) = F \quad (30)$$

for a given channel reach where  $F$  is the right-hand side of equation (28). The neglect of  $Q/A$  relative to  $\sqrt{gD}$  in the above approximate channel equations is tantamount to neglect of longitudinal advection of momentum in the original equation (13), an approximation already made in the two-dimensional equations in Section II, 1.

### III. SURGE II PROGRAM

Numerical algorithms for two-dimensional blocks and subgrid scale channels are given in this section, and the coupling between these is discussed. A complete listing of the SURGE II program is in Appendix A. A description of the program, as adapted for the GE-400 computer, and the required input and output options are discussed in Appendix B. Appendix C is a user's guide to the SURGE II program. The block algorithm is essentially as discussed by Reid and Bodine (1968) except for a change in the barrier computation and incorporation of coupling with the subgrid scale channels.

#### 1. Block Algorithm.

In the numerical analog of the prognostic equations (1), (2), and (3), values of  $H$  are evaluated on a uniform Cartesian mesh at spacing,  $\Delta s$ , for uniform time steps,  $\Delta t$ . The values of  $H$  are representative of the water level for the grid square  $i, j$  which is centered at  $x = (i - 1/2) \Delta s$ ,  $y = (j - 1/2) \Delta s$ , at time  $n\Delta t$ , in which  $i$ ,  $j$ , and  $n$  are integers. Values of  $Z$  are specified as permanent storage for the same locations as  $H$  so that  $D$  can be evaluated as needed at these locations. Values of  $U$  are evaluated at even half steps of  $x$ , odd half steps of  $y$ , and odd half steps of  $t$  (Fig. 3). This staggered system gives the least storage consistent with a given spatial resolution. It corresponds to the simplest scheme discussed by Platzman (1958) and requires only half the storage compared with the coupled scheme used by Miyazaki (1963).

The variables  $X$  and  $Y$  are supplied at spatial locations consistent with  $U$  and  $V$ , respectively, but at even half steps of  $t$ . Values of  $H_g$  are supplied for positions and times on the outer boundary of the bay consistent with the locations and times for the  $H$  values on that line. Values of  $R$  are supplied at locations consistent with  $H$  but at a one-half time step out of phase with  $H$ . Arrays of  $X$ ,  $Y$ , and  $R$ , for a single value of  $j$  and  $n$ , and the array of  $H_g$  values for given  $n$  are read from tape as required. The fields of  $X$  and  $Y$  are generated from a coarse spatial and temporal array evaluated from the basic meteorological data and then evaluated for the detailed mesh by linear interpolation.

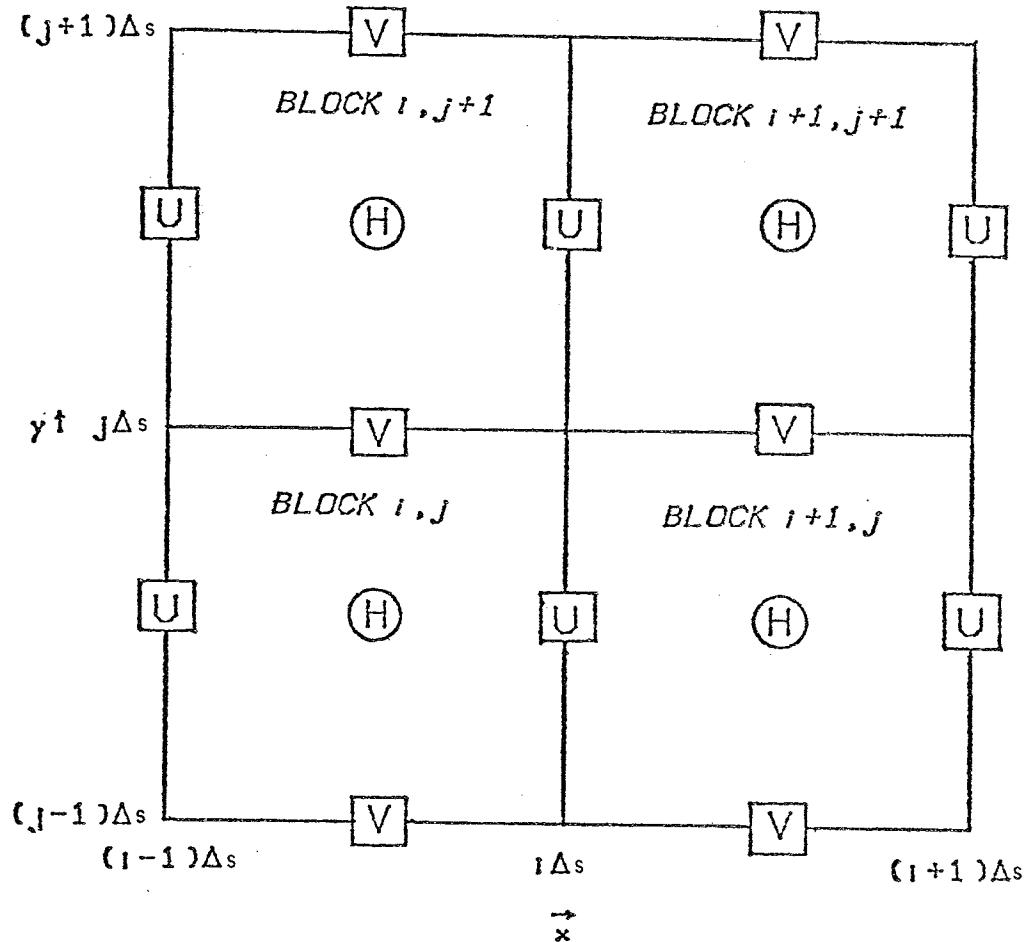


Figure 3. Example of grid blocks showing staggered arrangement of variables  $U$ ,  $V$ , and  $H$ .

Information pertinent to the position, elevations, and discharge coefficients for barriers (those not resolved by the limitations of the grid system) is stored as permanent storage along with the field of  $Z$ .

The numerical analogs of equations (1), (2), and (3) use values of  $U$ ,  $V$ ,  $H$ ,  $Z$ ,  $X$ ,  $Y$ , and  $R$  at locations shown in Figure 4 for a typical calculation. In the present application a common value of  $R$  for given time is used for the whole spatial array. The following notation is used in the recursion equations:  $H(i,j)$  represents  $H$  centered in block  $i, j$  at  $t = n\Delta t$ ;  $U(i,j)$  represents  $U$  for the left side of block  $i,j$  at  $t = (n - 1/2) \Delta t$ ;  $v(i,j)$  represents  $V$  for the lower side of block  $i,j$  at  $t = (n - 1/2) \Delta t$ .

Primed symbols are used to denote values of these variables at time step  $\Delta t$  later. Thus, the difference  $U' - U$  is centered in time at the level of  $H$ , and the difference  $H' - H$  is centered in time at the level of  $U'$  or  $V'$ . The notation for  $Z$  or  $D$  is consistent with that of  $H$ .

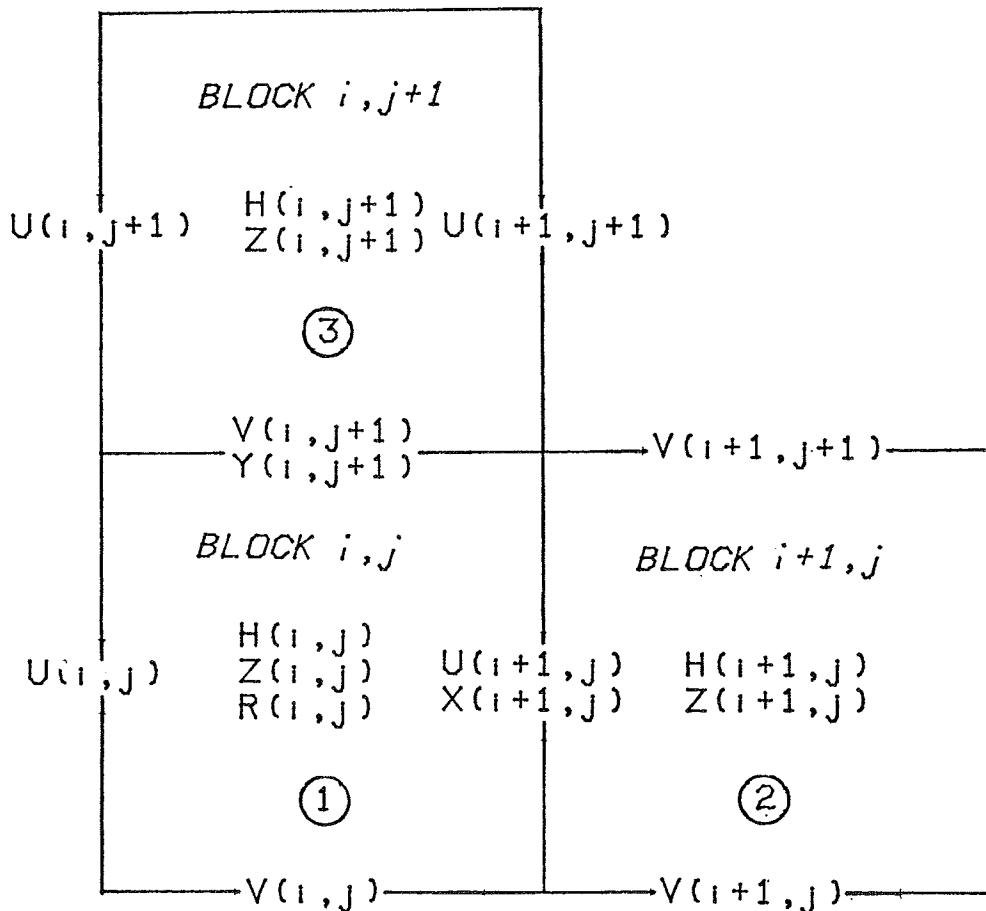


Figure 4. Basic block triad showing variables used in computation of  $U$ ,  $V$ , and  $H$  for block 1.

The frictional terms in equations (1) and (2) are represented by  $fAU'D^{-2}$  and  $fQV'D^{-2}$ , respectively, where the estimation of  $Q$  and  $D$  is centered spatially at the position for  $U'$  or  $V'$ . Since  $U$ ,  $V$ , and  $D$  are not available at common locations, this requires a suitable spatial average in order to obtain centered values of  $Q$  and  $D$ . The resulting recursion equations for  $U$ ,  $V$ , and  $H$ , using centered differences for the spatial derivatives, are as follows:

$$U'(i+1,j) = \frac{1}{G_1(i,j)} \left\{ U(i+1,j) + \frac{g\Delta t}{2\Delta s} [D(i+1,j) + D(i,j)] [H(i,j) - H(i+1,j)] + X(i+1,j)\Delta t \right\} \quad (31)$$

$$V'(i,j+1) = \frac{1}{G_2(i,j)} \left\{ V(i,j+1) + \frac{g\Delta t}{2\Delta s} [D(i,j+1) + D(i,j)] [H(i,j) - H(i,j+1)] + Y(i,j+1)\Delta t \right\} \quad (32)$$

$$H'(i,j) = H(i,j) + \frac{\Delta t}{\Delta s} [U'(i,j) + V'(i,j) - U'(i+1,j) - V'(i,j+1)] + R(i,j)\Delta t, \quad (33)$$

where

$$D(i,j) = H(i,j) - Z(i,j), \quad (34)$$

and  $G_1$  and  $G_2$  are the factors which incorporate the effect of the friction. These are given by:

$$G_1(i,j) = 1 + f\Delta t \{ [4U(i+1,j)]^2 + [V(i,j) + V(i+1,j) + V(i,j+1) + V(i+1,j+1)]^2 \}^{1/2} [D(i,j) + D(i+1,j)]^{-2} \quad (35)$$

and

$$G_2(i,j) = 1 + f\Delta t \{ [4V(i,j+1)]^2 + [U(i,j) + U(i+1,j) + U(i,j+1) + U(i+1,j+1)]^2 \}^{1/2} [D(i,j) + D(i,j+1)]^{-2}. \quad (36)$$

The latter factors are always somewhat greater than unity unless the flow or friction factor vanishes.

The prediction relation for  $H$  given by equation (33) does not consider any possible contribution of flow to or from the block due to the presence of a subgrid scale channel. This will be considered in a subsequent section.

It should also be emphasized that the effect of Coriolis force is not considered. The relative importance of the Coriolis force compared with bottom friction can be estimated in terms of the ratio,  $r$ , of these two forces which is of the order,

$$r = \lambda D / fu, \quad (37)$$

where

$\lambda$  = Coriolis parameter ( $2\Omega \sin \phi$ ,  $\Omega$  being the absolute angular speed of the earth and  $\phi$  the latitude);

$D$  = mean depth;

$f$  = bottom-friction coefficient;

$u$  = typical fluid speed in the bay.

For  $30^\circ$  latitude  $\lambda = 7.3 \times 10^{-5}$ ; typical  $D$  and  $f$  for gulf coast bays are 10 feet and  $2 \times 10^{-3}$ , respectively. For  $u = 3$  feet per second, which is reasonable for storm conditions,  $r$  is only 1/10. However, for normal circulatory regimes  $u$  may be only a fraction of 1 foot per second and  $r$  is of order unity. Hence, while it may be justifiable to neglect the Coriolis term for short-duration storm surge studies for shallow bays of limited horizontal dimensions it cannot be neglected in long-term circulatory studies.

Although it does not appear difficult to add the effect of Coriolis force, it can be shown (Platzman, 1958) that a different scheme for the  $U$ ,  $V$ , and  $H$  arrays is necessary for numerically stable computations using an explicit time-marching procedure as used here. The coupled scheme required for stable explicit computations at least doubles the computing time. The present scheme could be used with an implicit time-marching procedure to maintain stability and similar accuracy, but this too can be achieved only at the cost of an increase in computing time by a factor of at least two. In the presence of friction, the destabilizing effect of the Coriolis terms in an explicit scheme such as that used by Masch (1969) is suppressed; however, this is accomplished only at the sacrifice in rendition of the frictional terms. Thus, the omission of the Coriolis force from a program intended primarily for gulf coast estuaries is motivated primarily for reasons of economy of operation, in respect to surge calculations.

a. Stability. Numerical stability requires that  $\Delta t$  be taken at less than the value  $\Delta S / (2gD_{\max})^{1/2}$ , where  $D_{\max}$  is the maximum depth to be expected anywhere in the system during the storm surge (Platzman, 1958).

b. Barrier Algorithm. Equations (9) and (10) are assumed to apply for values of  $q_n$ ,  $D_b$ , and  $\Delta H$  at the same time and in the immediate vicinity of the barrier. In the grid scheme used, however, the flow and the water level are staggered in time; moreover, the water levels like  $H_1$  and  $H_2$  represent in effect the spatial average for blocks 1 and 2, respectively, at a given time rather than local values in the vicinity of a given barrier, which in the schematization are presumed to occur on lines separating two blocks. As a consequence the above relations cannot be applied directly. Instead, the evaluation of  $U$  or  $V$  across a barrier (if the water level allows such flow) is carried out by a modified version of the predictive equations (1) and (2), or their numerical counterparts, equations (31) and (32), where  $f$  is replaced by an effective value related to the barrier discharge coefficient so as to be consistent with equations (9) or (10). The effect is to maintain proper time phasing and to consider possible tilt of water level across the block; i.e., difference of  $H$  at barrier relative to the mean value for the block.

Specifically, the frictional terms in equation (1) or (2) are taken as  $(D/LC_b^2) |q'| q' / D_b^2$  where  $C_b$  is the barrier discharge coefficient

( $C_o$  or  $C_s$ , depending on type of barrier),  $q'_n$  is the transport per unit width normal to the barrier (either  $U'$  or  $V'$ , depending on barrier orientation),  $D_b$  is the water depth over the barrier, and  $\bar{D}$  is a mean depth for the effective fetch  $L$  across the blocks. The gravitational slope term involves the same scale length,  $L$ , and mean depth,  $\bar{D}$ . The resulting relation for prediction of  $q'_n$  at a barrier, given  $q_n$  at the previous time step, is:

$$|q'_n| q'_n + \Gamma q'_n = F , \quad (38)$$

where

$$\Gamma \equiv \frac{L(C_b D_b)^2}{\bar{D} \Delta t} \quad (39)$$

and

$$F \equiv g(C_b D_b)^2 \Delta H + \Gamma \cdot (q_n + P) , \quad (40)$$

$P$  being the wind "push" term ( $X\Delta t$  or  $Y\Delta t$ ), and  $\Delta H$  a head differential dependent on barrier type. For steady state ( $q'_n = q_n$ ) and no wind ( $P = 0$ ), the above reduces to

$$q'_n = \pm C_b D_b \sqrt{g|\Delta H|} , \quad (41)$$

which is consistent with equation (9) or (10) with  $C_b$  and  $\Delta H$  taken as  $C_o$  and  $D_b$  or  $C_s$  and  $(H_1 - H_2)$ , respectively, depending on the barrier. The more general relation (eq. 38) provides an added effect of the wind and of the inertia of the water on the blocks. For a submerged barrier,  $L$  is taken equal to  $\Delta S$ ; i.e., from the center of block 1 to the center of block 2. For an overflow barrier,  $L$  is taken as half this distance since the inertia and wind setup are effective only on the higher of the two blocks.

Thus,  $C_b$ ,  $L$ ,  $H$ , and  $D_b$  are taken as follows:

Submerged barrier ( $H_1 > Z_b$  and  $H_2 > Z_b$ )

$$\begin{aligned} C_b &= C_s \\ L &= \Delta S & \Delta H &= H_1 - H_2 \\ D_b &= [(H_1 + H_2)/2] - Z_b \end{aligned} \quad (42)$$

Overflow barrier ( $H_1 > Z_b$  or  $H_2 > Z_b$ )

$$\begin{aligned} C_b &= C_o \\ L &= \Delta S/2 \\ D_b &= |\Delta H| \end{aligned} \quad \Delta H = \begin{cases} H_1 - Z_b & (\text{a}) \\ \text{or} \\ Z_b - H_2 & (\text{b}) \end{cases}$$

where  $Z_b$  is the elevation of the barrier crest, relation (a) being for  $H_1 > Z_b$  and (b) for  $H_2 > Z_b$ . If  $Z_b$  exceeds both  $H_1$  and  $H_2$ , then  $q_n' = 0$ . The meaningful solution of the quadratic equation (38) is

$$q_n' = \pm \{ [|F| + (\Gamma/2)^2]^{1/2} - \Gamma/2 \}, \quad (43)$$

where the sign is taken as that of  $F$ , as verified from equation (38).

The above relations for barriers differ from that used in Reid and Bodine (1968) and in the original SURGE I program. The present barrier relations have a more realistic response when applied to the numerical simulation of a natural oscillation of a bay having a submerged barrier across it.

c. Barrier Specification. Since only certain blocks contain barriers, they must be identified by  $I, J$  location; specifically, the program identifies the  $K$ th barrier block by location  $I = IB(K)$  and  $J = JB(K)$ ,  $K = 1, 2 \dots KM$ . A given barrier block potentially has a barrier on the right and upper side of the block in an  $x, y$  plot. These are designated  $x$  and  $y$ , respectively; i.e., an  $x$  barrier is one normal to the  $x$ -axis (the flow over it being in the  $x$  sense). For both potential barriers on a barrier block, values of  $Z_b$ ,  $C_o$ , and  $C_s$  must be prescribed. A real barrier is one where  $Z_b$  is larger than the  $Z$  value for either of the adjoining blocks. A null barrier is one where  $Z_b$  equals the larger of the  $Z$  values for the adjoining blocks (thus, in effect, the higher block is a potential barrier). The program requires that information pertinent to both null barriers ( $Z_b$ ,  $C_o$ , and  $C_s$ ) and real barriers be provided.

d. Volume Check. During the recession stage of flooding when water is draining off flooded blocks (via the barrier overflow relation), it is possible for the volume leaving in one time step as computed from  $q_n' \Delta t$  to exceed the available volume. Therefore, a test is included in the program such that if this occurs, the flow is adjusted to only drain the block dry ( $D = 0$ ), and the flow to adjacent blocks adjusted to be consistent.

e. Depth Check. When the water depth is very shallow the effect of the wind is such that a given block could become partially dry unless the fluid is flowing fast enough for the bottom stress to balance the wind stress. To avoid anomalous computations for very small  $D$  (e.g., in areas where rainfall is occurring over regions above the surge level), the wind stress is arbitrarily set zero when  $D$  is less than 0.1 foot.

## 2. Channel Algorithm.

a. Channel Specification. As in the case of barriers, those blocks on which channels occur are identified by the  $I$  and  $J$  values; for channel block  $K$  these are denoted by  $ICG(K)$  and  $JCG(K)$ , respectively, where  $K = 1, 2 \dots KCM$ . Also each "channel block" may contain two channels, one on the right denoted the  $x$  channel and one on the upper side denoted the  $y$  channel. Each of these channel reaches is characterized by a

channel width ( $w$ ), a channel-bed elevation ( $Z_c$ ), and a channel-friction coefficient ( $f_c$ ). Figure 5 shows a schematic of a channel block indicating nomenclature for dimensions as used in the SURGE II program. Figure 6 shows the dependent variables pertinent to the channels as used in the program and stored for the channel block  $K$ . These include the channel flows,  $Q$ , at each end of the channel, one end designated  $N$ , the other  $P$  (corresponding to the negative and positive characteristic ends of the channel, respectively). Also included is the height,  $H$ , of the water level at the point in common to the two channels for block  $K$  ( $HC(K)$ ). The lateral transport (per unit width per unit time) nominally to the channel from block  $K$  and from the channel is also indicated:  $UCT(K)$  and  $UCF(K)$ , respectively, for the channel normal to the  $x$ -axis, and  $VCT(K)$  and  $VCF(K)$ , respectively, for the channel normal to the  $y$ -axis. In the formulas in this study, these are referred to as  $q_t$  and  $q_f$ , respectively. Note that  $UCF(K)$  and  $VCF(K)$  correspond to  $U$  and  $V$ , respectively, on the right and upper sides of the general block flow. Also, the quantity  $HP(K)$  corresponds to the block (pool) height for the channel block. Values of  $H$  at the "negative" ends of the channels for channel block  $K$  are stored as  $HC$  values in adjacent channel blocks to minimize duplication of storage.

b. Computation of Channel Variables. The time phasing of block variables versus channel variables is indicated in Table 1. The  $H$  values occur at common times thus facilitating evaluation of head differentials used in determining lateral flow between channel and adjacent blocks.

Table 1. Time phasing of computations for blocks and channels.

Time	Block	Channel
$t + \Delta t$	$H$	$H, Q$
$t + \Delta t/2$	$Q$	
$t$	$H$	$H, Q$
$t - \Delta t/2$	$Q$	
$t - \Delta t$	$H$	$H, Q$

For a given channel reach, application of equations (28) and (29) can be made for two characteristic paths, as shown schematically in Figure 7. As in the case of the block computations, the friction term in equation (28) is taken proportional to the product of a new  $Q$  and

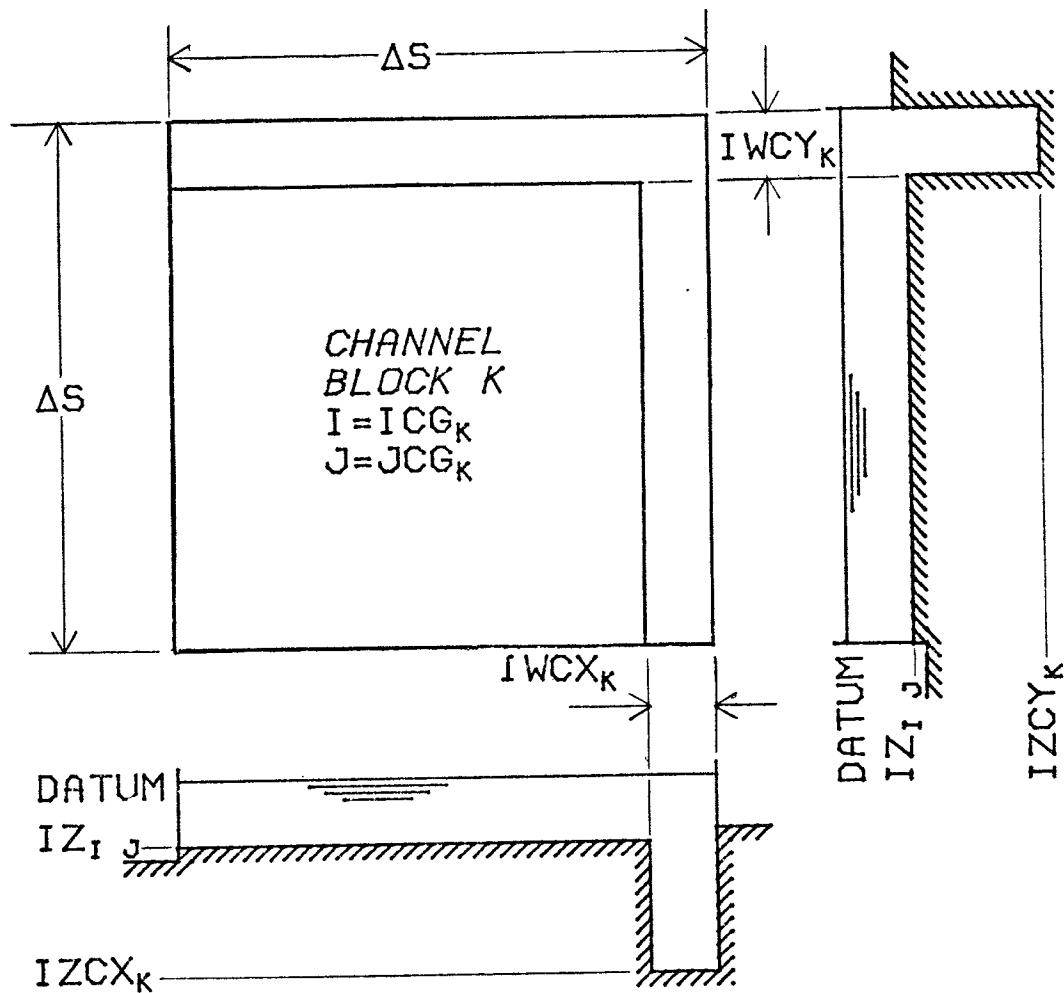


Figure 5. Channel block, showing channels and their dimensions.

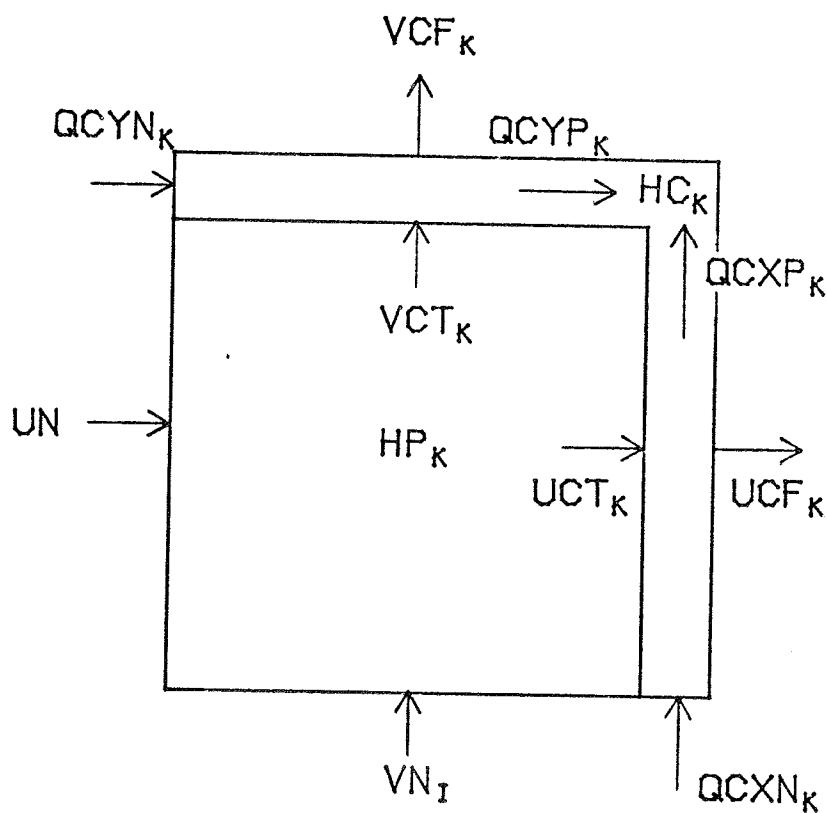


Figure 6. Channel block  $K$  at coordinates  $I = ICG(K)$  and  $J = JCG(K)$ , showing associated flows and water level variables.

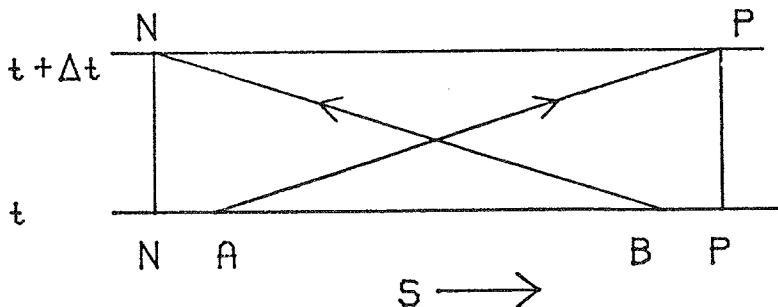


Figure 7. Characteristic paths on the time-distance diagram for an individual channel reach.

the absolute value of the old  $Q$ . Specifically, for the positive characteristic path from  $A$  to  $P'$  in Figure 7, equation (28) is approximated by

$$(Q'_P - Q_A) + w\sqrt{gD} (H'_P - H_A) = [WT_s - f_c |\bar{Q}| Q'_P / (\bar{D})^2 w + \sqrt{gD} \Delta q] \Delta t, \quad (44)$$

where  $\bar{D} = (D_N + D_P)/2$ ,  $T_s$  is the appropriate wind-stress component ( $X$  or  $Y$ ) corresponding to time level  $t$  for the associated channel block,  $\Delta q$  is the net lateral flow per unit width, and  $\bar{Q}$  is taken as

$$\bar{Q} = [(Q_N^2 + Q_P^2)/2]^{1/2}. \quad (45)$$

The subscripts on  $Q$ ,  $H$ , and  $D$  designate the points at which these apply (see Fig. 7) and primes denote new time level.

After regrouping terms, equation (44) can be written as

$$Q'_P + (w\sqrt{gD}/G) H'_P = [(Q_A + w\sqrt{gD} H_A) + (WT_s + \sqrt{gD} \Delta q) \Delta t]/G, \quad (46)$$

where

$$G \equiv 1 + f_c \Delta t |\bar{Q}| / (\bar{D})^2 w. \quad (47)$$

Similarly, for the negative characteristic from  $B$  to  $N'$ ,

$$Q'_N - (w\sqrt{gD}/G) H'_N = [(Q_B - w\sqrt{gD} H_B) + (WT_s - \sqrt{gD} \Delta t)]/G, \quad (48)$$

where  $\bar{D}$  and  $G$  are as defined for the positive characteristic.

The values of  $Q$  and  $H$  at points  $A$  and  $B$  are determined by interpolation from values at  $N$  and  $P$  at time  $t$ , using equation (29) for the path. The distance from  $A$  to  $P$  or  $B$  to  $N$ , using the mean wave speed for the channel at time  $t$  is  $\sqrt{gD}\Delta t$ . The interval  $N$  to  $P$  is equal to  $\Delta s$ . Let

$$\alpha \equiv \sqrt{gD} \Delta t / \Delta s; \quad (49)$$

this should always be less than or at most unity for stability of computation. The linearly interpolated values at A and B are then

$$Q_A = \alpha Q_N + (1 - \alpha) Q_P \quad (50)$$

$$Q_B = (1 - \alpha) Q_N + \alpha Q_P,$$

and similarly for  $H_A$  and  $H_B$  in terms of  $H_N$  and  $H_P$ .

The evaluation of  $\Delta q$  is the most sensitive part of the computations and is discussed in a subsequent section. Presuming  $\Delta q$  is known, the problem of evaluating the new Q and H individually at the channel-end points is considered. Note that equations (46) and (48) yield predictions for linear combinations of Q and H at two different points. Thus, information from adjoining channels, or other information in the case of channel end points, is needed to solve for the new channel Q and H. For a simple continuous channel without branches and consisting of a series of reaches of length  $\Delta s$  but not necessarily of equal width or depth, then Q and H are readily solved at a common junction, using the information from the positive characteristic from one channel and the negative characteristic from the adjoining channel. However, branches do occur and it is therefore desirable to use a sufficiently general procedure which will accommodate either branching channels or continuous channels.

In the scheme chosen for representing channels in SURGE II it is possible to have four channels merging at a common junction. Figure 8 shows this junction with four different volume transports, but with a common H. The designation of the different Q shown in this figure is that used in the coded program (see App. B); QC for channel transport, X or Y denoting the channel (not the direction of flow), and N or P denoting whether the flow is at the negative or positive end of a given channel reach. Each is identified by a channel block index K.

For any given channel reach equations (46) and (48) predict, for a given point, values of the quantities

$$BP \equiv Q' + \lambda H' \quad (51)$$

$$BN \equiv Q' - \lambda H' ,$$

where

$$\lambda \equiv w \sqrt{gD}/G . \quad (52)$$

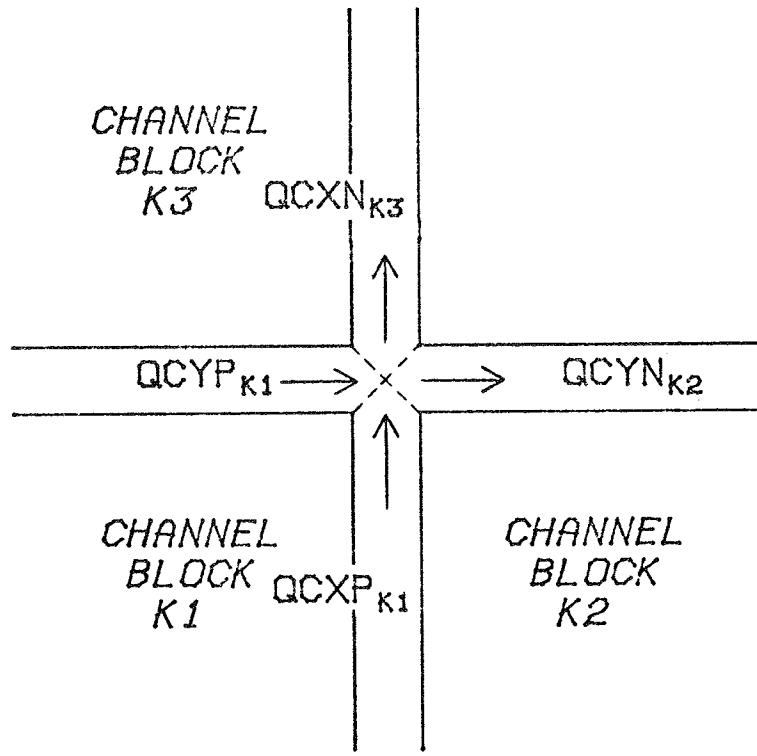


Figure 8. General channel junction, showing flows and channel identification.

For simplicity of notation let 1, 2, 3, and 4 denote the merging channels with 1 being the lower channel, 2 the left channel, 3 the upper channel, and 4 the right channel (Fig. 8). Then, with this notation

$$\begin{aligned} Q1' + \lambda_1 \cdot H' &= BP1 \\ Q2' + \lambda_2 \cdot H' &= BP2 \\ Q3' - \lambda_3 \cdot H' &= BN3 \\ Q4' - \lambda_4 \cdot H' &= BN4 . \end{aligned} \tag{53}$$

Now, continuity requires that

$$Q1' + Q2' - Q3' - Q4' = 0 \tag{54}$$

at a common junction. Thus,

$$(\lambda_1 + \lambda_2 + \lambda_3 + \lambda_4) H' = BP1 + BP2 - BN3 - BN4 \tag{55}$$

from which  $H'$  can be calculated at the junction. With  $H'$  known, the values of  $Q1'$ ,  $Q2'$ ,  $Q3'$ , and  $Q4'$  are readily evaluated from equation (40).

For those cases where one or two of the above merging channels do not exist (i.e., null channels), then their width and  $\lambda$  value are zero. Moreover, the program yields zero for the BP or BN values of any null channel. Thus, equations (55) and (53) can apply for a general junction consisting of from two to four real merging channels.

c. Net Lateral Flow. The net time rate of water accumulation in the channel per unit length due to lateral exchange with blocks and by rainfall is

$$\Delta q = q_t - q_f + wR , \tag{56}$$

where  $q_t$  corresponds (if positive) to the flow (per unit length of channel) from the channel block to the channel (across the "interior" side of the channel, Fig. 6) and  $q_f$  (if positive) is the flow (per unit length of channel) from the channel to the adjacent block. These flows can be positive, negative, or zero. To allow for channels which have widths  $w$  much smaller than the block grid size  $\Delta s$ , and since the above  $q$  values are comparable to those which exist across the sides of blocks, the change in channel water level can be very sensitive to the difference  $q_t - q_f$ . Hence, special care must be taken in the model to avoid possible instabilities caused by improper calculation of these transverse flows. However, there is no particular difficulty with the rainfall term in equation (56) which is generally at least one order of magnitude smaller than that of the "net" lateral flow. In a sense, the potential difficulty with the transverse flows,  $q_t$  and  $q_f$ , arises because the  $\Delta t$  chosen for stable calculation on the blocks is usually

too large for stable calculation for narrow channels, unless the coupling with blocks exists only in respect to longitudinal flow from the channels to blocks at end points of such channels.

On a given side of a channel, basically four physically distinct situations can occur: (a) a barrier (levee) or block ground level of sufficient height exists to prevent lateral flow; (b) overflow exists from an adjacent flooded block into a channel where the water level is less than the adjacent barrier or ground level; (c) overflow of adjacent barrier (levee) exists from the channel to an adjacent dry block or one where the water level is lower than the barrier elevation; or (d) both the channel water level and the water level on the adjacent block exceed the height of any intervening barrier and the lateral flow depends on the difference of water level. These four situations are illustrated in Figure 9. In the fourth situation, the water level could also be lower on the channel side with the associated lateral flow reversed.

For situation (a) there is no problem, the appropriate lateral flow ( $q_t$  or  $q_f$ ) being constrained to zero value. For situation (c), the predictive-type barrier relation (eq. 55), with auxiliary relations (eqs. 39 and 40), could be used. In principle, the above predictive barrier relations should apply for situation (b) as well, provided that  $L$  in equation (39) is taken as the channel width  $w$ . However, since  $w$  can be much less than  $\Delta s$  for many applications,  $\Gamma$  can be so small that the relation for  $q_n^*$  reduces virtually to a diagnostic-type relation of equation (40), or more specifically of equation (9) for barrier overflow. Since situation (b) might occur on one side of the channel and situation (c) on the other, and since both should be evaluated by relations compatible with a common time level, the simple diagnostic relation (eq. 9) has been adopted for both situations in the SURGE II program. This, however, still demands special checks and possible adjustments, as will be discussed later. Finally for situation (d), a submerged barrier-type calculation might seem appropriate if the depth over such a barrier is small compared with that of the channel or adjacent block; however, use of such relations in preliminary versions of the program proved to be very vulnerable to numerical instability. The reason for this is related to the above discussion concerning the usual case where  $w/\Delta s$  is very small. As a consequence, for situations of type (d), a special calculation is required which treats the channel as essentially an integral part of the associated channel block or the adjacent block.

As stated above, for overflow situations (b) or (c), i.e., to or from the channel, the relation,

$$q_n = \pm C_0 D_b (g D_b)^{\frac{1}{2}}, \quad (57)$$

is used where  $D_b = H - Z_b$ ,  $H$  being the water level on the high side of the barrier. While this relation gives a valid value of  $q_n(q_t$  or  $q_f)$  at the time  $t$ , the value of  $q_n$  may change significantly over the prediction interval  $\Delta t$  if  $(g D_b)^{\frac{1}{2}} > w/\Delta t$ .

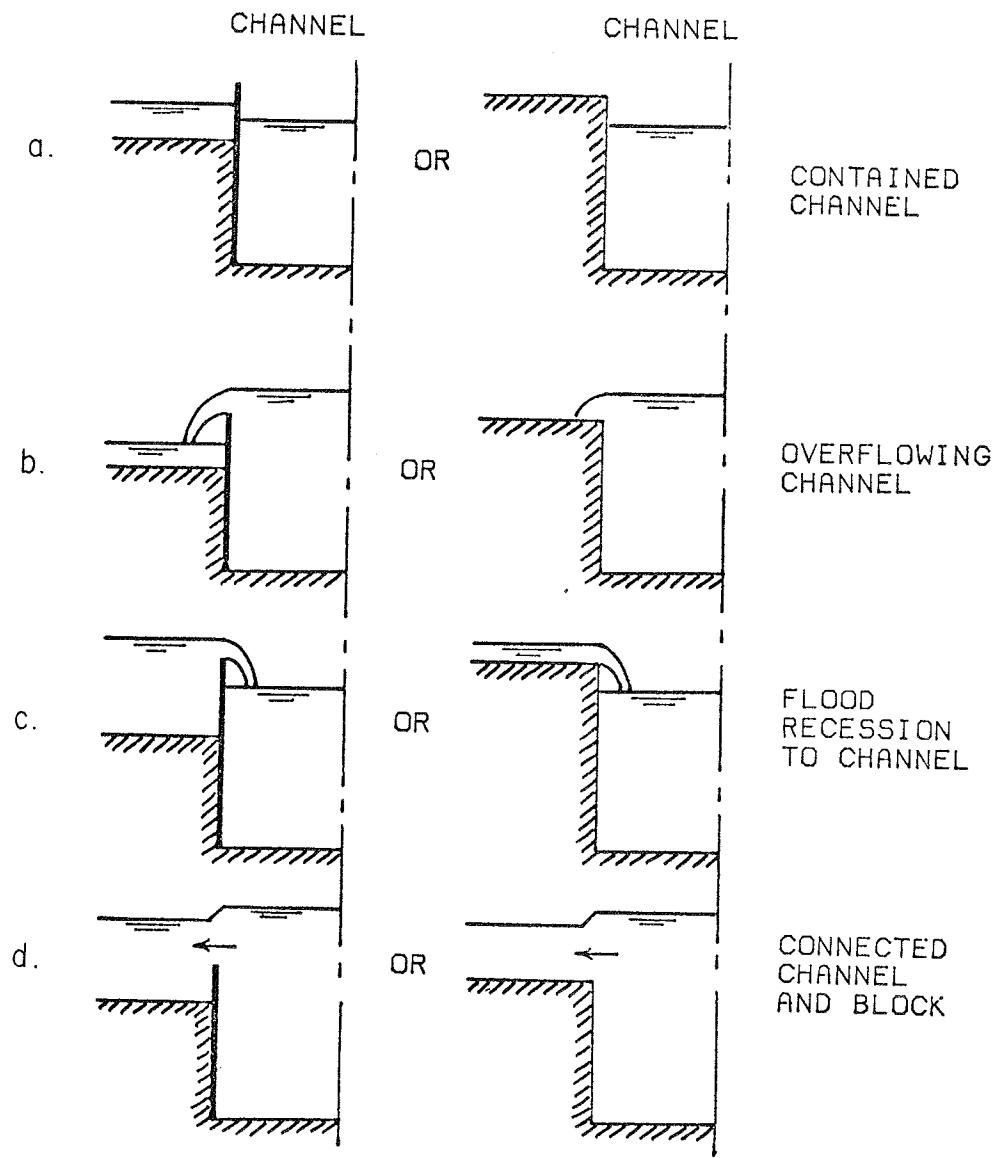


Figure 9. Different situations along a given side of a channel.

Under such circumstances, an approximate prediction based on the initial values of  $q_n$  could lead to physically impossible changes of channel level. Thus, tests are included in the program to constrain the lateral flow, such that  $q_t - q_f$  alone will not cause the channel level,  $H_c$ , to fall below a minimum possible value nor rise above a maximum possible value, depending on the situation. Six different situations requiring tests are illustrated in Figure 10 (the "mirror" version of each is also a possible situation). Situations where one side of the channel is blocked are special cases of those indicated. For situations A, C, and E, outflow exceeds inflow and the horizontal dashline represents a minimum level based on the sill depth of the channel. On the other hand, for situations B, D, and F, the horizontal dashline represents a maximum possible level. In each case, the maximum possible change in  $H_c$  is indicated as  $\Delta H_c$ .

For any of the situations illustrated in Figure 10, the SURGE II program compares  $|q_t - q_f|$  with  $|wH_c/\Delta t|$ . If the latter is exceeded by the trial value of  $|q_t - q_f|$  then an adjustment is made in  $q_t$  or  $q_f$  such that  $|q_t - q_f|$  equals  $|w\Delta H_c/\Delta t|$ . For cases A, B, C, and D, both  $q_t$  and  $q_f$  are prorated by a common factor to satisfy the above constraint. For cases E and F, only the overflow  $q$  is adjusted to be consistent with the above constraint.

For situation (d) where the channel and block are connected by a continuous water surface (Fig. 10), the net lateral flow to the channel,  $\Delta q$ , is taken to be that which would be required to bring  $H_c$  to a value equal to the existing mean level,  $HM$ , of the connected channel and block. For a channel connected to a block on one side only then,

$$HM = \frac{HB \cdot L + HC \cdot W}{(L+W)}, \quad (58)$$

where  $HB$  is the water elevation on the water-connected block,  $L$  is its width, while  $HC$  and  $W$  are the water elevation and width for the channel. The block width  $L$  is  $\Delta S - W$  if the connected block is the channel block containing the channel, or is  $\Delta S$  for an adjacent water-connected block. If the channel is water connected on both sides, then the above relation is replaced by an appropriate average over both blocks plus the channel.

The  $\Delta q$  for either of these situations is taken as

$$\Delta q = (HM - HC)w/\Delta t. \quad (59)$$

To determine the individual  $q_t$  and  $q_f$  on either side of the channel, the mean of these is taken to be that which is calculated as the flow

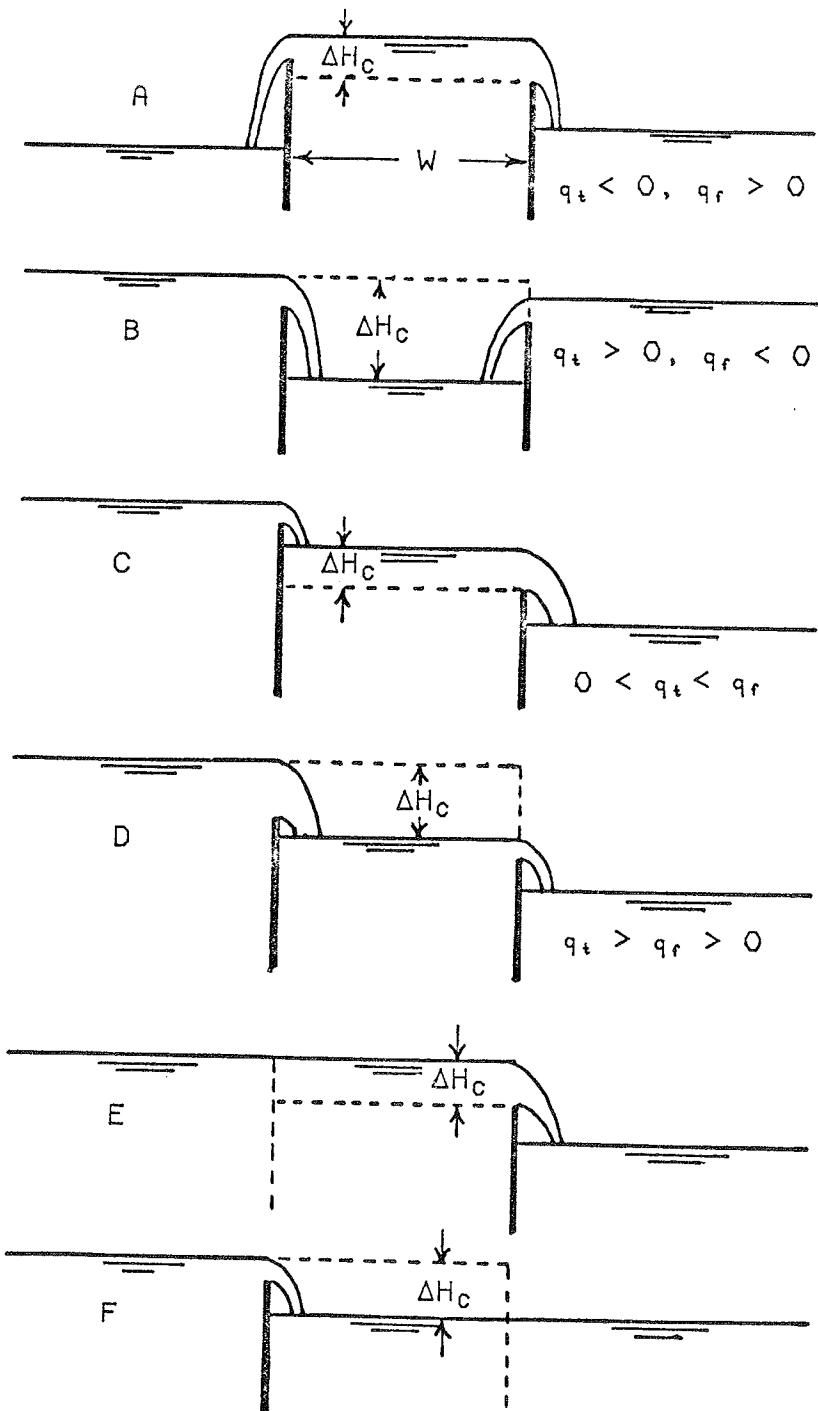


Figure 10. Situations involving overflow to or from a channel which require special checks.

between blocks, ignoring the presence of the channel (but considering barriers). Letting this be denoted  $q_m$ , then

$$q_t = q_m + \Delta q/2 \quad (60)$$

and

$$q_f = q_m - \Delta q/2 .$$

This system of calculation leads to stable results.

d. Channel End-Point Computations. At the end point of a given channel system, special computations are required. Two types of end conditions are used: an "H-end condition" is used where a channel discharges into a lake, bay, or sea, in which case the channel H value at the end point is taken equal to the H of the adjacent channel block into which the channel discharges (or vice versa); a "Q-end condition" is used at the head of a channel or river at which point the discharge is specified.

For a Q-end point

$$\begin{aligned} Q' &= \pm Q'_r \\ H' &= (Q' - B)/\lambda , \end{aligned} \quad (61)$$

where  $Q'_r$  is the specified river discharge (taken as zero if not specified); B equals BP or -BN, as defined by equation (51), for end points occurring at the positive or negative end of the channel reach, respectively, and  $\lambda$  is as defined in equation (52). The sign of  $Q'$  is taken such that  $Q'$  is directed into the channel, depending on the channel-end orientation. There are four possible orientations (see App. B, Fig. B-3).

The H-end points also have four possible configurations; these are depicted along with the associated adjacent "ponding" areas (i.e., a block with  $Z < 0$ ) in Figure 11. For an H-end point neither the longitudinal flow to or from the channel nor the H at the junction with the ponding block is specified *a priori*. It is required only that the predicted H at the channel-end point and that of the ponding block be the same. Let  $H^*$  be the (tentative) predicted H for the ponding block in the absence of any contribution by longitudinal discharge to or from the channel which terminates adjacent to that block. Thus,  $H^*$  corresponds to the H resulting from the routine block calculation using equation (33) with appropriate adjustments for contained channels as might occur for situations 3 and 4 shown in Figure 11. These adjustments are discussed in a subsequent subsection. The correct predicted H for the ponding block in the presence of longitudinal discharge from a channel is given by

$$H' = H^* + (Q'_d + Q_d)\Delta t/2A_b , \quad (62)$$

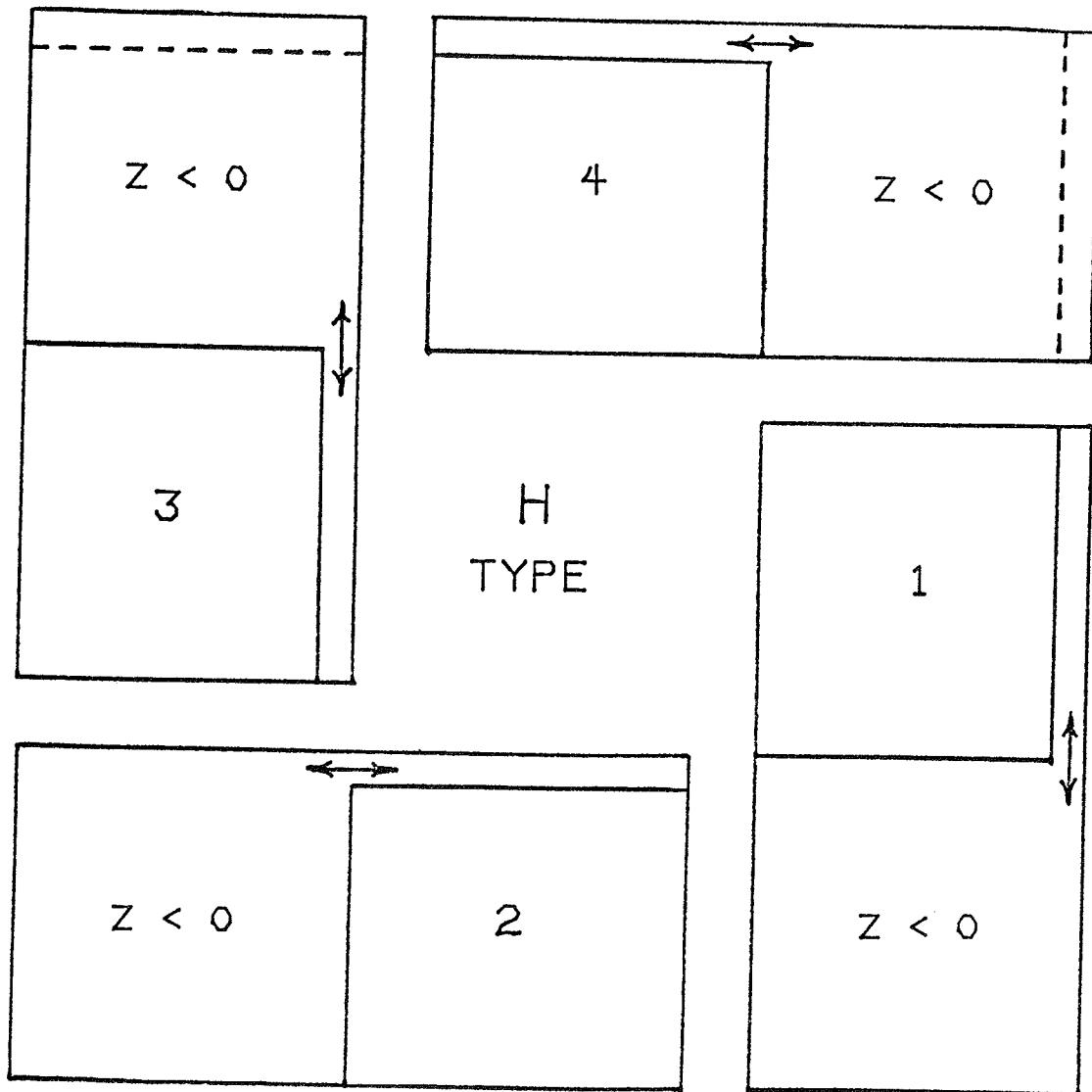


Figure 11. Possible end-point configurations and index identification (1 to 4).

where  $Q'_d$  and  $Q_d$  are the new and previous values, respectively, of the discharge from channel to ponding block, and  $A_b$  is the effective surface area of the block. For situations 1 and 2 in Figure 11,  $A_b = (\Delta s)^2$ , but for situations 3 and 4 a channel might exist on the ponding block in which case  $A_b = (\Delta s - w)\Delta s$ .

Equation (62) involves two unknowns  $H'$  and  $Q'_d$ . However, for the channel,

$$Q'_d + \lambda H' = B , \quad (63)$$

where  $B = -BN$  for end-point type 1 or 2 and  $B = BP$  for end-point type 3 or 4,  $BN$ ,  $BP$  and  $\lambda$  being those quantities defined by equations (46) to (52). Note that for end-point type 1 or 2,  $Q'_d$  is the negative of the QC value for the channel.

The resulting  $H'$  and  $Q'_d$  for an "H-end" condition are

$$H' = (F + B\Delta t/2A_b)/(1 + \lambda\Delta t/2A_b) \quad (64)$$

$$Q' = (B - \lambda F)/(1 + \lambda\Delta t/2A_b) , \quad (65)$$

where

$$F \equiv H^* + Q_d \Delta t/2A_b . \quad (66)$$

e. Calculation of H on Channel Blocks. For blocks with  $D > 0$  and containing one or two channel reaches, the prediction relation for  $H$  given by equation (33) is not valid. The correct relation for a channel block  $k$  having location  $i,j$  is

$$\begin{aligned} H'(i,j) = H(i,j) + [U'(i,j) - UCT'(k)]\Delta t/(\Delta s - wx) \\ + [V'(i,j) - VCT'(k)]\Delta t/(\Delta s - wy) \end{aligned} \quad (67)$$

where  $UCT$  and  $VCT$  are as shown in Figure 6 and correspond to the  $q_t$  discussed previously. If only one channel exists (i.e., if  $wx$  or  $wy$  is zero), then

$$UCT'(k) = U'(i+1,j) \text{ if } wx = 0$$

or

$$VCT'(k) = V'(i,j+1) \text{ if } wy = 0 .$$

#### IV. APPLICATION TO THE SABINE-CALCASIEU SYSTEM

##### 1. Adopted Grid and Simulated Topography.

The Sabine-Calcasieu system geographically bridges the Texas-Louisiana border and is physically linked by a system of manmade channels and a low-lying region extending 25 miles between Sabine Lake and Lake Calcasieu.

A local chart of the region is shown in Figure 12. The rectangular border indicates the region included in the numerical analog. The selection of the size of this rectangle is dictated by the basic hydrodynamic features required to adequately represent the region and then the logistical and economic limitations placed on the computations by the availability of computer storage. The region selected is  $56 \times 40$  nautical miles. The grid size (DELX) is taken as 2 nautical miles, so that  $IM = 28$  and  $JM = 20$ .

Figure 13 is a contoured plot of the schematized topography superimposed on the selected grid system. The offshore topography is regular with the exception of a shallow region adjacent to Sabine Pass and a slight embayment lying between Sabine Pass and the outlet from Lake Calcasieu at Cameron. Both lakes are adequately represented by the grid interval of 2 nautical miles. Figure 14 clearly delineates three high topographic areas in the numerical model: the Beaumont rise in the northwest, the Orange rise, and a more gradual rise northeastward to the Lake Charles area. The low-lying region between the lakes, immediately behind the shoreline barrier, and forward of the rises, forms a large ponding area during the inundation sequences. Between each rise a major channel is present, the Neches River, the Sabine River, and in the Lake Charles region, the Calcasieu River runs northeastward from Lake Calcasieu.

The deepest block in the system is -24 feet (MSL). Assuming a 10-foot surge, a value of DELT equal to or less than 260 seconds (Sec. III, 1,b) is required. The value chosen for DELT is 240 seconds.

## 2. Channel and Barrier Schematization.

The numerical discretization of the area shown in Figure 12 is given as an overlay in Figure 15. In this illustration the channel network (shown by full lines) shows the landward interconnection of Sabine and Calcasieu as well as the link with the Intracoastal Waterway as the lower left- and right-hand channels. Each channel segment has been provided the physical characteristics of width and cross-sectional area that best reproduce the pertinent information for the channel reach that was provided by the Corps of Engineers. The extent of the channel system was chosen on the basis of past inundation history and the judgment of the authors.

The barrier system, also shown in Figure 15, represents the major manmade and natural obstructions to flow above MSL. At the shoreline the major dune line is continuous with the exception of an apparent open area east of Sabine Pass. The block elevation of that area equals the adjacent barrier heights. Jetties are included at each of the openings to the Gulf of Mexico. Within the region the majority of barriers are manmade levees erected for protection. The heights of all barriers were chosen on the basis of data provided by the Corps of Engineers.

Appendix D has a listing of all data used for the Sabine-Calcasieu region in the simulation of the Hurricane Carla surge. The topography,

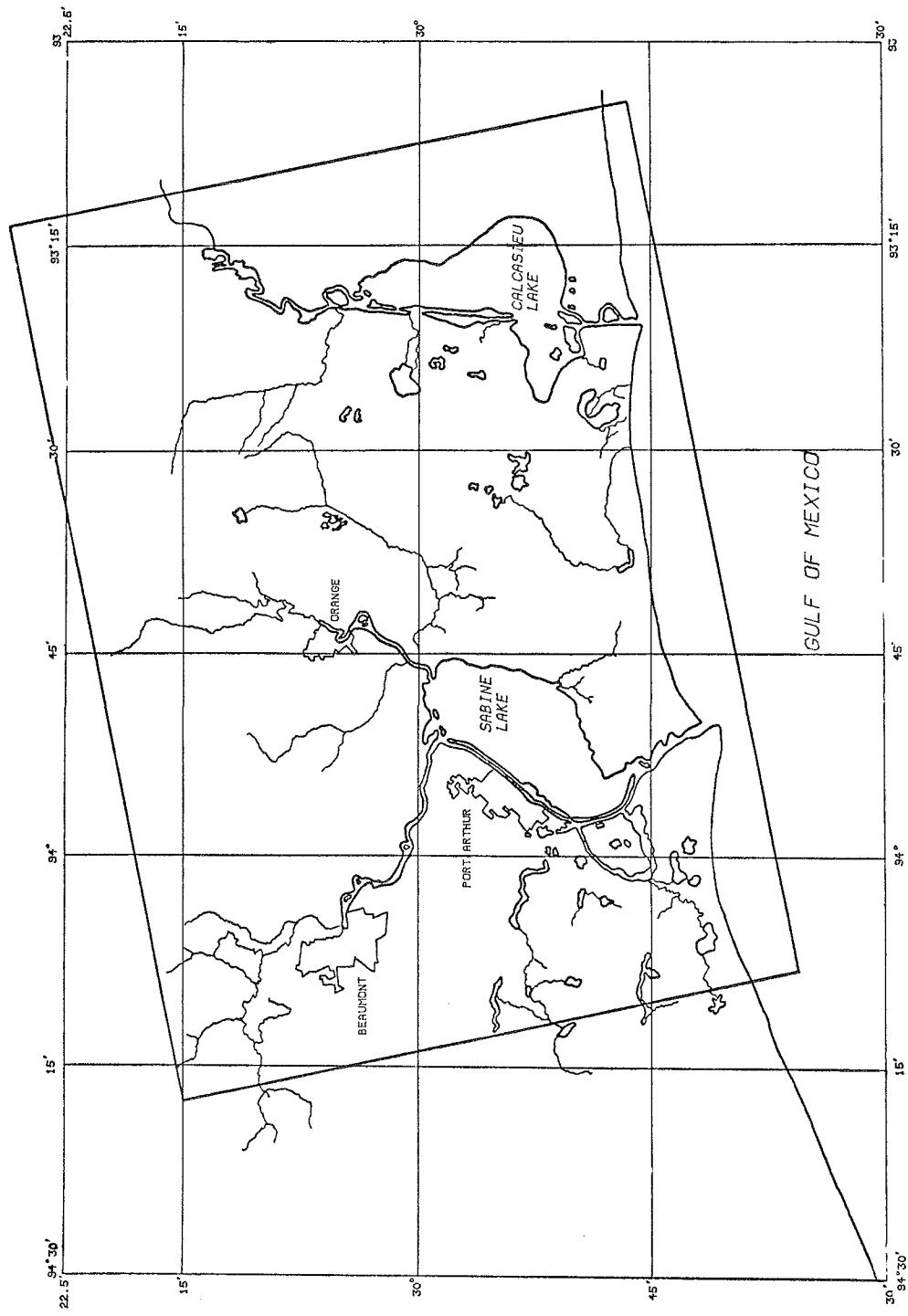


Figure 12. Map of Sabine-Calcasieu region showing grid boundary.

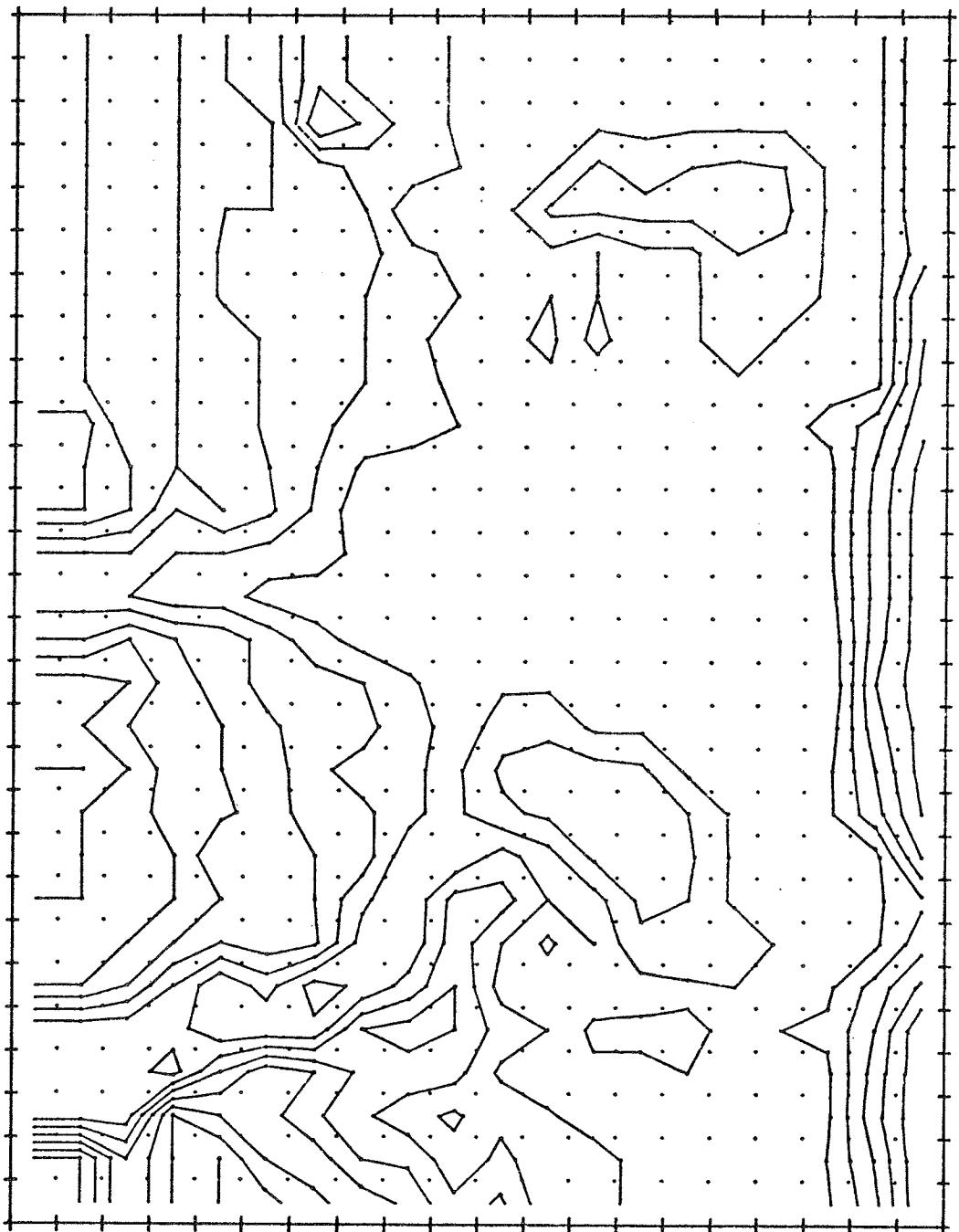


Figure 13. Topography contours at 5-foot intervals for  
Sabine-Calcasieu region (broad uncontoured area  
between Lakes Sabine and Calcasieu has elevations  
between 0 and 5 feet).

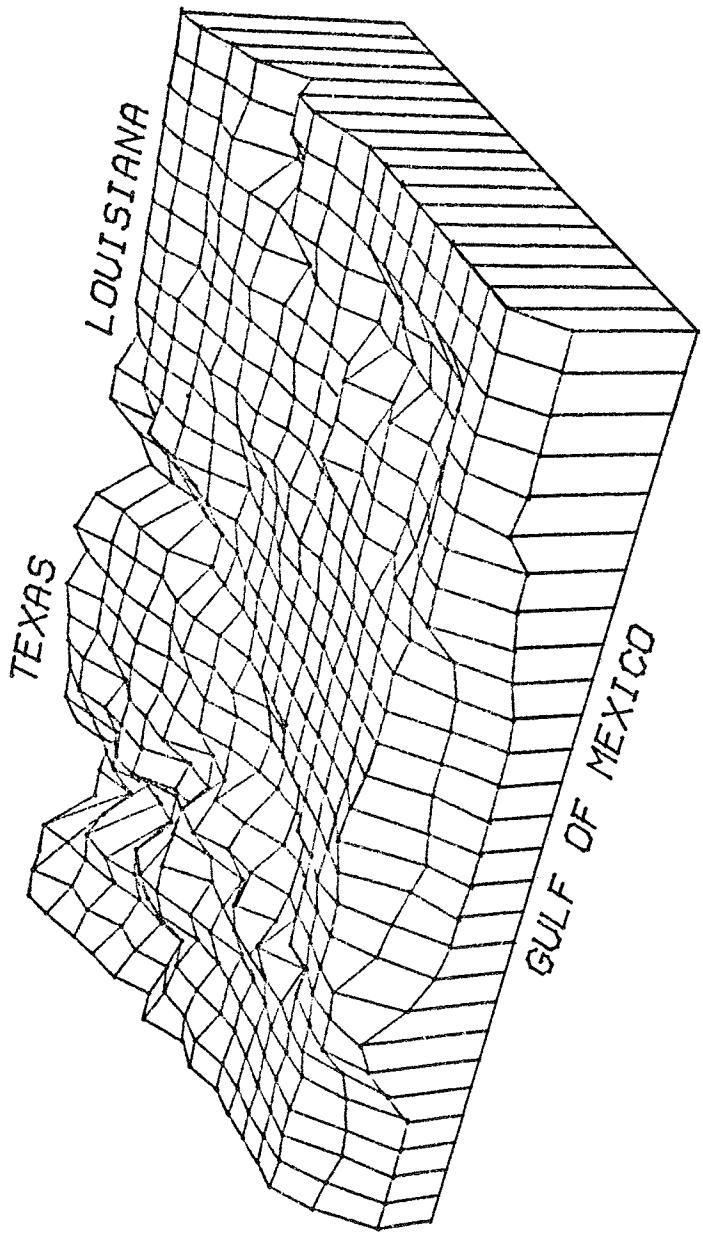


Figure 14. Topography in perspective for the Sabine-Calcasieu region.

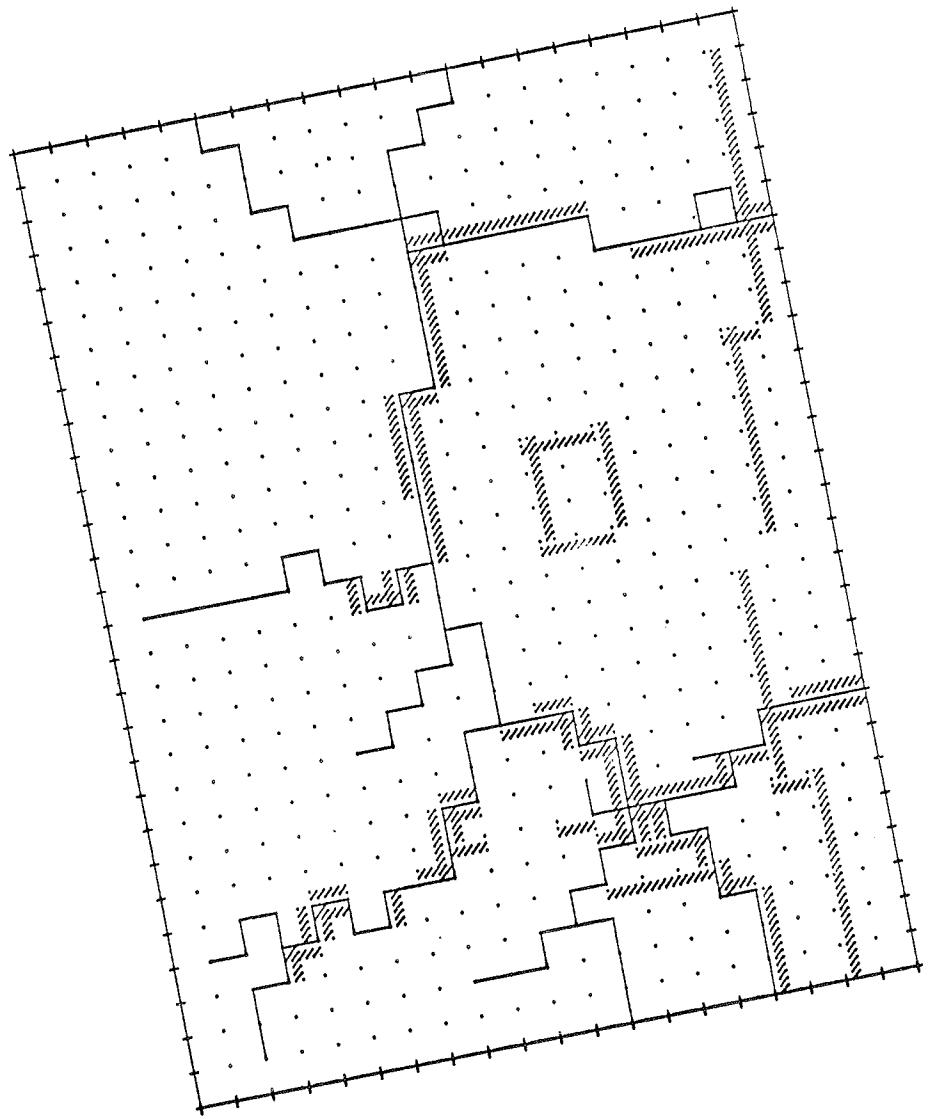


Figure 15. Overlay of grid (dots), channels (full lines) and barriers (hatched) on scale of Figure 12.

barrier data, and channel data are the same for the astrotide simulations and the standard project storms. There are 91 barrier blocks and 121 channel blocks of which 53 are common to barrier blocks. Examples of null channel blocks are for K = 4, 6, 18, 21, 24, etc., a total of 19.

Appendix E shows a plot of the block topography with the channel and barriers superimposed. This plot is given on two pages; x (or I) runs from left to right and y (or J) runs from bottom to the top of the page (I values are indicated along the top of both pages and J along the left side of the first page). Also in Appendix E is a listing of the key arrays for channels as generated by the program. Note that the final array size for channels is 128 (KCMC), there being 6 channels which terminate on the boundary of the grid.

As an illustration of barrier input note from Appendix E that for block (2,2) a y barrier exists, but not an x barrier. The bed elevation of block (2,2) is -10 feet while that of block (3,2) is -13 feet. Thus, a value of ZX of -10 feet should have been input for this block. The listing of the barrier input data in Appendix D gives the information for block (2,2) at K = 12 with ZX = -100 (tenths of feet) which checks. The actual barrier on the upper side indicates a positive 6 feet. However, barrier block K = 13 at the adjacent block (3,2) shows a ZX value of -12 feet. Reference to the topography in Appendix E indicates that this is the elevation of adjoining block (4,2) which is higher than block (3,2) and hence is the correct entry.

For an illustration of the sign coding concerning barriers along channels, refer to the channel input data in Appendix D and the plot in Appendix E. Channel block K = 1 located at (8,1) shows a negative IWGX and a negative IZCX which is the coding for double levees of equal height with the channel in between. This is the location of the double jetty entrance channel for the Sabine region. Channel block 5 at location (7,4) shows a (+,-) signature for the x channel and a (+,+) signature for the y channel. Hence, the barrier for the x channel is on the inner lateral boundary while that for the y channel is on the outer lateral boundary (see App. C,6). Reference to Appendix E key array listings shows KCB = 37 for channel block 5. Barrier block 37 has the same location (7,4) and indicates valid barriers of a 5-foot elevation above MSL for both the x and y channels.

### 3. River Input and Hydrograph Gage Locations.

There are three river discharge locations provided for the Sabine-Calcasieu region. These locations, as given in block 9 of the input (App. D), are (28,15), (4,19), and (14,19) which are respectively for the Calcasieu River near Lake Charles, the Neches River north of Beaumont, and the Sabine River north of Orange.

Nine gage locations for the astrotide calibration and Hurricane Carla simulation are shown as small circles in Figure 16. All of these with the exception of the North Sabine Lake gage are located on channels.

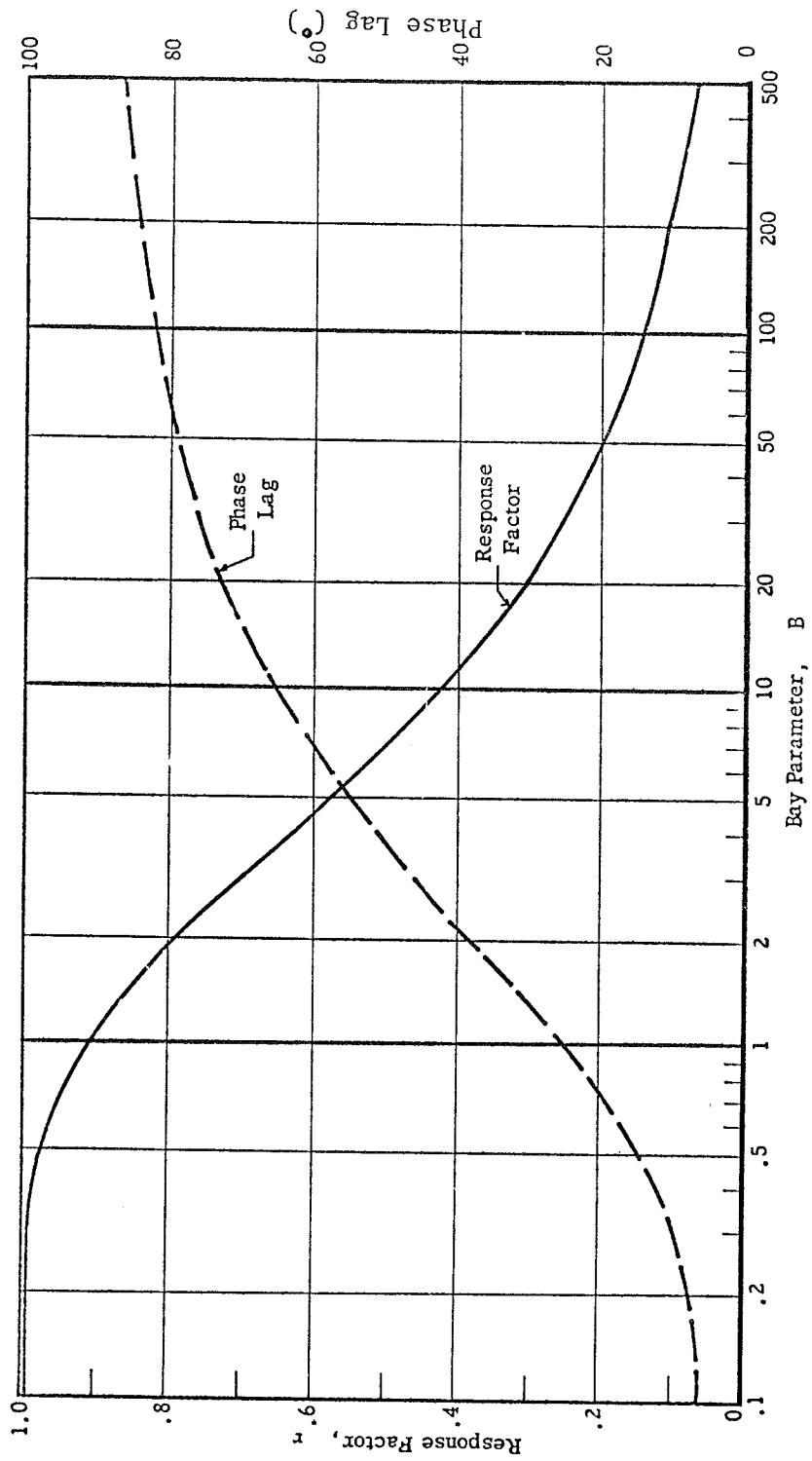


Figure 17. Amplitude response factor ( $r$ ) and phase lag ( $\phi$ ) versus the dimensionless parameter  $B$  characterizing a constricted bay.

feet for the Calcasieu part). The two parts of the system are coupled via the Intracoastal Waterway and their responses are about the same, so the combined system is treated as one.

A summary of data and calculations pertinent to the entrance channels for the Sabine-Calcasieu system is given in Table 2 (see also Fig. 15 and App. D). The simulated Sabine Pass between the gulf and Lake Sabine consists of two sections (1 and 2 in Table 2) of different dimensions in series. However, Calcasieu Pass consists of a pair of parallel channels (4 and 5 in Table 2) in series with a simple channel (3 in Table 2). The individual  $\alpha^2$  for each channel is also shown in Table 2. The effective  $\alpha^2$  for Sabine Pass is the first partial sum shown in the last column. The effective value of  $\alpha^2$  for the parallel part of Calcasieu Pass is shown in the last column, opposite entries 4 and 5. The effective value for Calcasieu Pass is the partial sum indicated in the last column. The effective value for the entire pass system is evaluated from the Sabine Pass and Calcasieu Pass values, using equation (79) for parallel systems:

$$\alpha^2 = 0.32 \times 10^{-6} \text{ (square feet)}^{-1} .$$

Table 2. Data on simulated Sabine Pass and Calcasieu Pass.

n	$W_c$ (ft)	$D_c$ (ft)	$A_c$ (ft <sup>2</sup> )	$L_c$ (ft)	$\alpha^2 \times 10^6$ (ft <sup>-2</sup> )	$\alpha^2 \times 10^6$ (ft <sup>-2</sup> )
Sabine Pass						
1	2,330	20	46,600	24,360	0.561	0.561
2	2,860	21	60,060	36,480	0.482	0.482
						1.043
Calcasieu Pass						
3	800	32	25,600	24,360	1.162	1.162
4	500	40	20,000	12,160	0.760	0.455 <sup>1</sup>
5	1,000	16	16,000	34,480	8.960	
						1.617

<sup>1</sup>Evaluated by parallel channel relation.

The observed ranges and times of minimum tide for 25 August 1973 for the Sabine-Calcasieu system are given in Table 3. Gage 1 is used as the input gulf tide. The average of all other gages is used as the response. The indicated amplitude response is

$$r = \frac{1.50}{2.59} = 0.58 .$$

Using a tidal period of 25 hours the indicated phase lag is

$$\phi = (20.8 - 17.5) \frac{360}{25} = 47^\circ .$$

Table 3. Ranges and times (c.d.t) of available observed tides in the Sabine-Calcasieu system for 25 August 1973.

Gage No.	Place	Range (ft)	Time (hr)
1	Sabine Pass, southwest jetty	2.59	17.5
2	Port Arthur	1.53	19.0
3	North Sabine Lake	1.40	21.5
4	Beaumont	1.52	21.5
5	Orange	1.40	23.0
6	Cameron	2.05	17.5
7	Hackberry	1.06	22.0
8	Calcasieu Lock, west	1.45	20.5
9	Lake Charles	1.60	21.5
Average of 2 to 9, inclusive		1.50	20.8

From Figure 17 the corresponding values of  $B$  are 4.7 and 3.6, respectively, with an average of 4.1. The tidal frequency is

$$\omega = \frac{2\pi}{25 \times 3,600} = 7.0 \times 10^{-5} \text{ radians per second}$$

and  $a_0 = 2.57/2$  or 1.3 feet. Consequently, the estimated  $f_c$  for the entrance channels is from equation (76):  $f_c = 0.0018$ .

The final selected value of  $f_c$  for the entrance channels is 0.0015 as determined by trial runs. This is somewhat less than the above estimate. The difference might be accounted for by the fact that the tidal hydrograph is not really simple harmonic but contains compound tides (of higher frequency) giving the sharp minimum and broad or double-peaked maxima. The effective frequency is consequently somewhat greater than the  $\omega$  given above, thus yielding a smaller  $f_c$  closer to 0.0015.

### 3. Final Calibration for Tide.

The major control on the response of the bay to the tides are the dimensions and friction factor for the entrance channels as discussed above. In this connection, it should be pointed out that channel dimensions (width and depth) were taken such that the average cross-sectional area (under MSL conditions) for a given reach is represented by the product of these dimensions. Thus, if the depth is taken as the mean for the reach, then the width will be somewhere between the width of the dredged channel and the surface width of the natural channel.

The values of channel friction for the remaining channels and of the block friction were selected by a trial-and-error procedure, starting with a uniform value throughout. The final values of channel friction for the upper reaches of the Neches and Sabine Rivers were taken as

0.0025 to give a reasonable agreement for the Beaumont and Orange tide response; it was necessary to use a low value (0.0005) for the upper reach of the Calcasieu River to reproduce the Lake Charles tidal hydrograph. The latter three gages (Beaumont, Orange, and Lake Charles) have connections to the inner bay areas only via channels, hence their responses are fairly sensitive to the channel friction. The low value for the Calcasieu River may be due to underestimates of the effective channel widths, which would demand a less than normal friction factor.

The block friction for the tide calculations was taken as 0.0015 to get a reasonable agreement for the north Sabine Lake gage. However, later calculations for the Hurricane Carla simulation (which is more sensitive to block friction than the astrotide) indicated that 0.0025 (as used in the Galveston Bay simulations) was more appropriate.

The results of the final astronomical tide simulations for a 96-hour period starting 0000 hours c.d.t., 22 August 1973, are given in Figures 18 to 26, and Appendix F. The input tide (Fig. 18) corresponds to the observed tide for the period at Sabine Pass (southwest jetty). In the subsequent eight figures the computed (full line) and observed (line with circles) are compared for the eight different gages within the system; the gages are identified in the figures. Note that the observed values for each gage have been adjusted with respect to a local datum, taken as the gage mean for a 120-hour period starting 0000 hours c.d.t., 22 August 1973. In all cases, the computed ranges are in fairly good agreement with the observed; however, there seems to be a consistent tendency for the computed to lag the observed. This might be due to a possible time-shift error for the input gage. Although a lowering of the frictional coefficient for the entrance channel would decrease the lag within the system, it would also increase the range of the tide everywhere in the system. It was felt that it was more important to reproduce the range than the times of high and low water, and hence the value of  $f_c = 0.0015$  for the entrance channels was retained.

For the upper Calcasieu River (Figs. 25 and 26) the computed water level (which refers to a common MSL datum for the system) and the observed water level display an apparent vertical shift. This could be related to possible wind effects in the second part of the record, which have been ignored in the computations.

The steady river discharges adopted in the astrotide runs were 800 cubic feet per second for Calcasieu River, 1,100 cubic feet per second for Neches River, and 1,500 cubic feet per second for Sabine River.

Serial listings of the computed water levels at the gages discussed above are given in Appendix F, along with listings of volume transport at six channel positions. Flow at points 1 and 2 correspond to input (if positive) to the system through Sabine Pass and Calcasieu Pass, respectively. Since the tide amplitude is less than the seaward barriers, the two passes represent the only source of water for normal conditions.

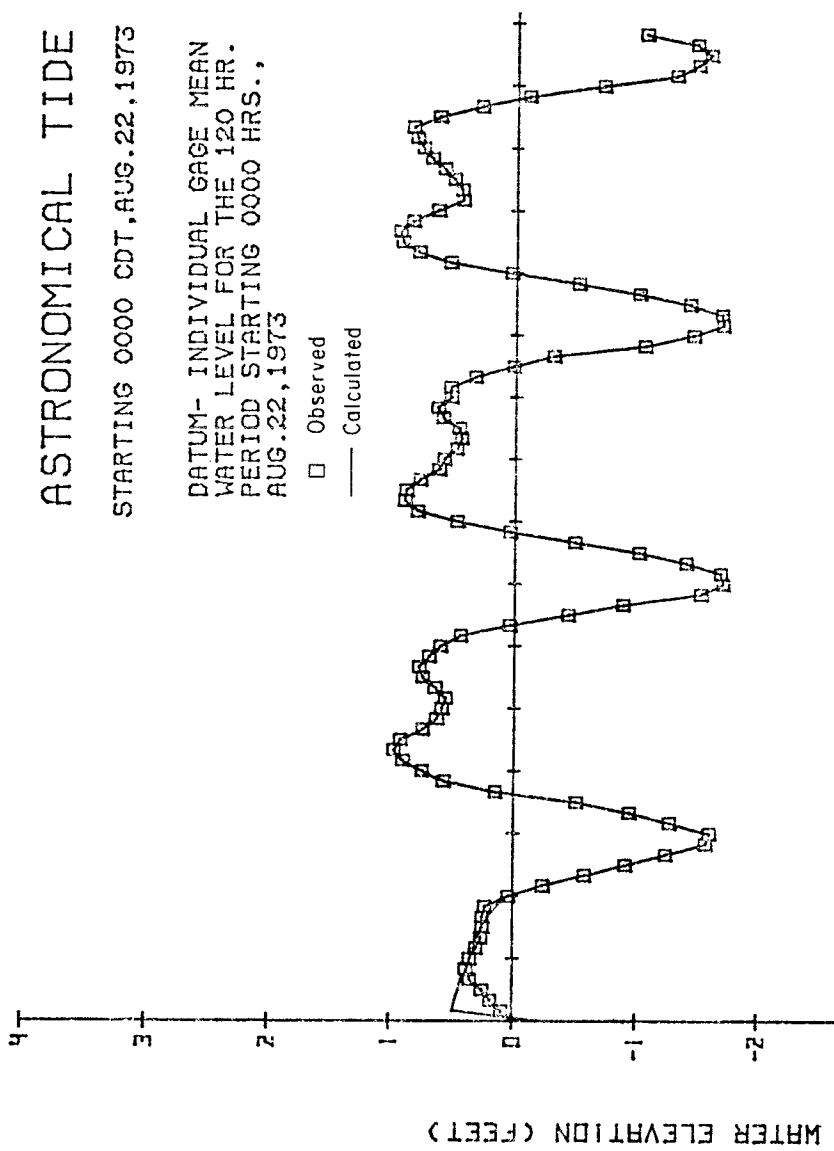


Figure 18. Astronomical tidal hydrograph for Sabine Pass, southwest jetty (input for tide calibration).

## ASTRONOMICAL TIDE

STARTING 0000 CDT, AUG. 22, 1973

DATUM- INDIVIDUAL GAGE MEAN  
WATER LEVEL FOR THE 120 HR.  
PERIOD STARTING 0000 HRS.,  
AUG. 22, 1973

□ Observed  
— Calculated

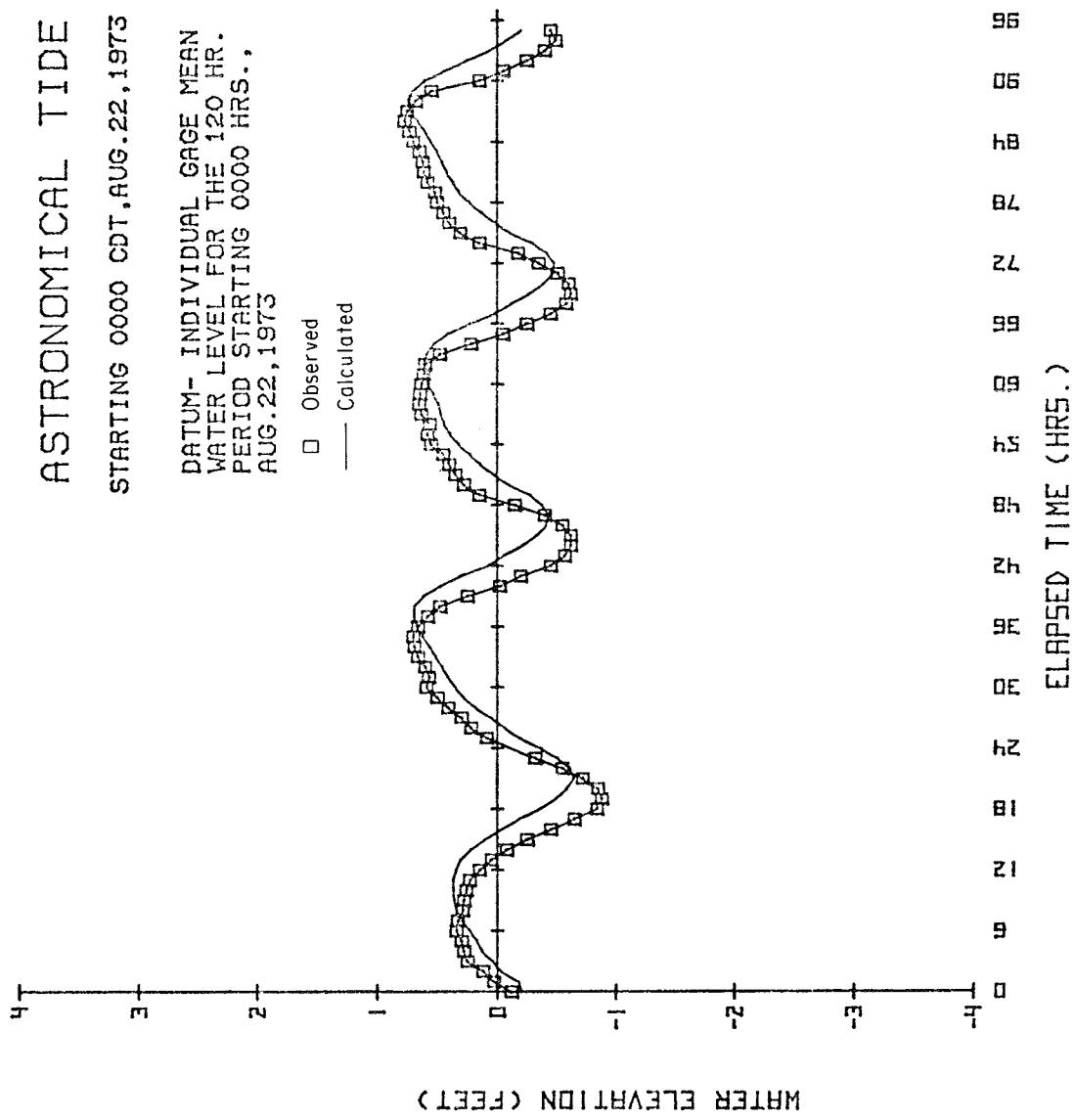


Figure 19. Astronomical tide for Port Arthur corresponding to input of  
Figure 18.

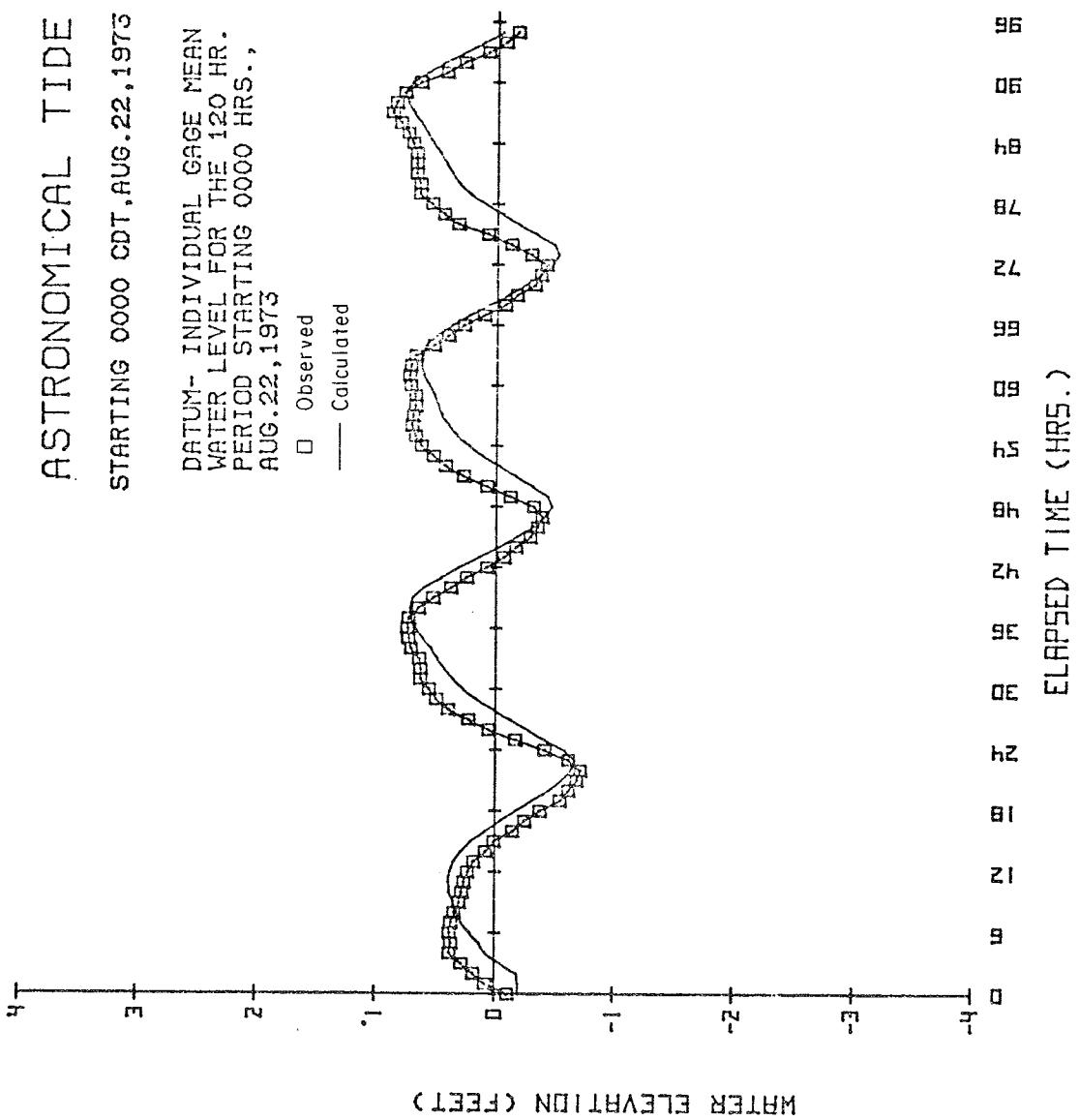


Figure 20. Astronomical tide for north Sabine Lake.

# ASTRONOMICAL TIDE

STARTING 0000 CDT, AUG. 22, 1973

DATUM- INDIVIDUAL GAGE MEAN  
WATER LEVEL FOR THE 120 HR.  
PERIOD STARTING 0000 HRS.,  
AUG. 22, 1973

□ Observed  
— Calculated

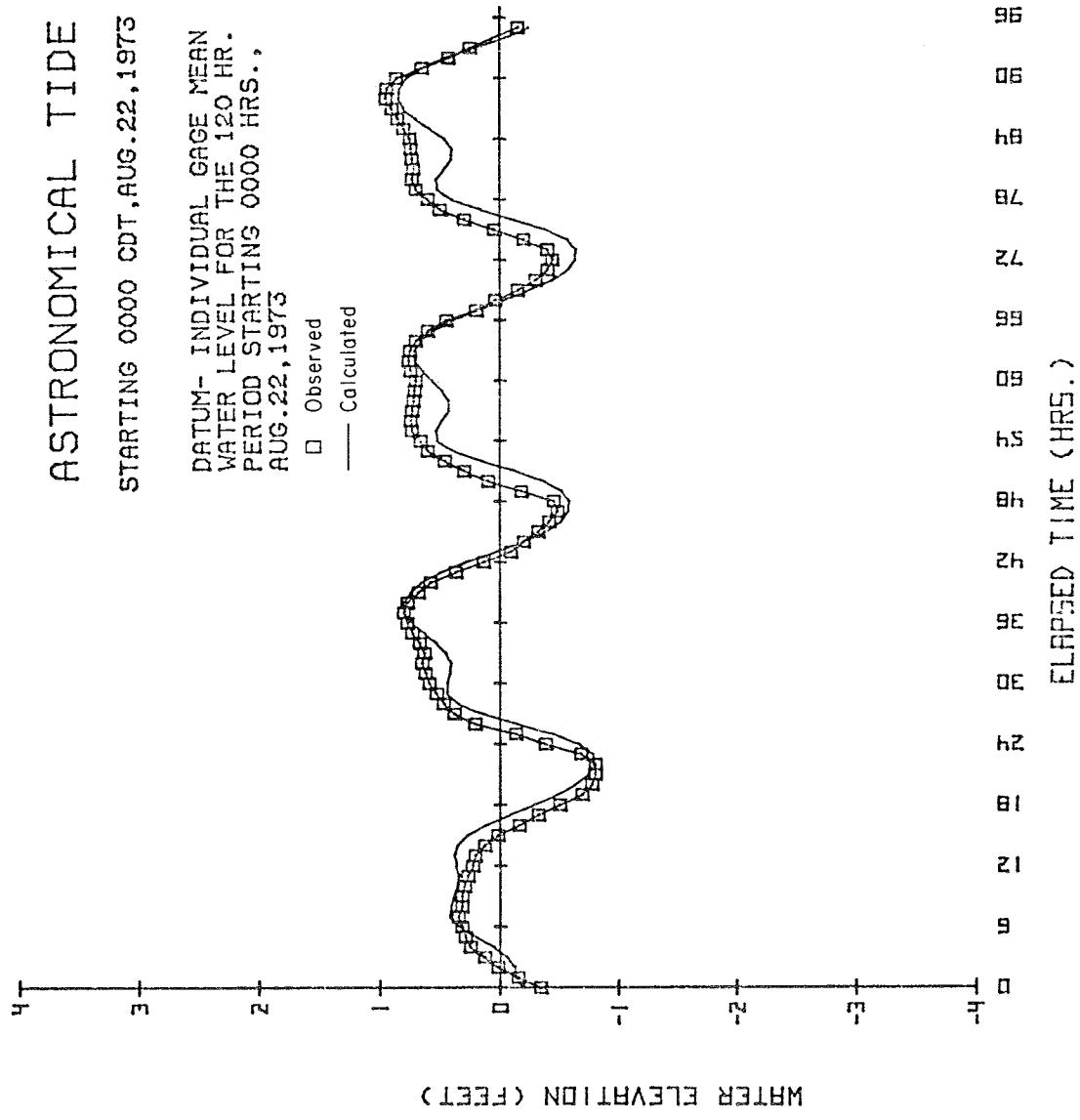


Figure 21. Astronomical tide for Beaumont, Neches River, and Brakes Bayou.

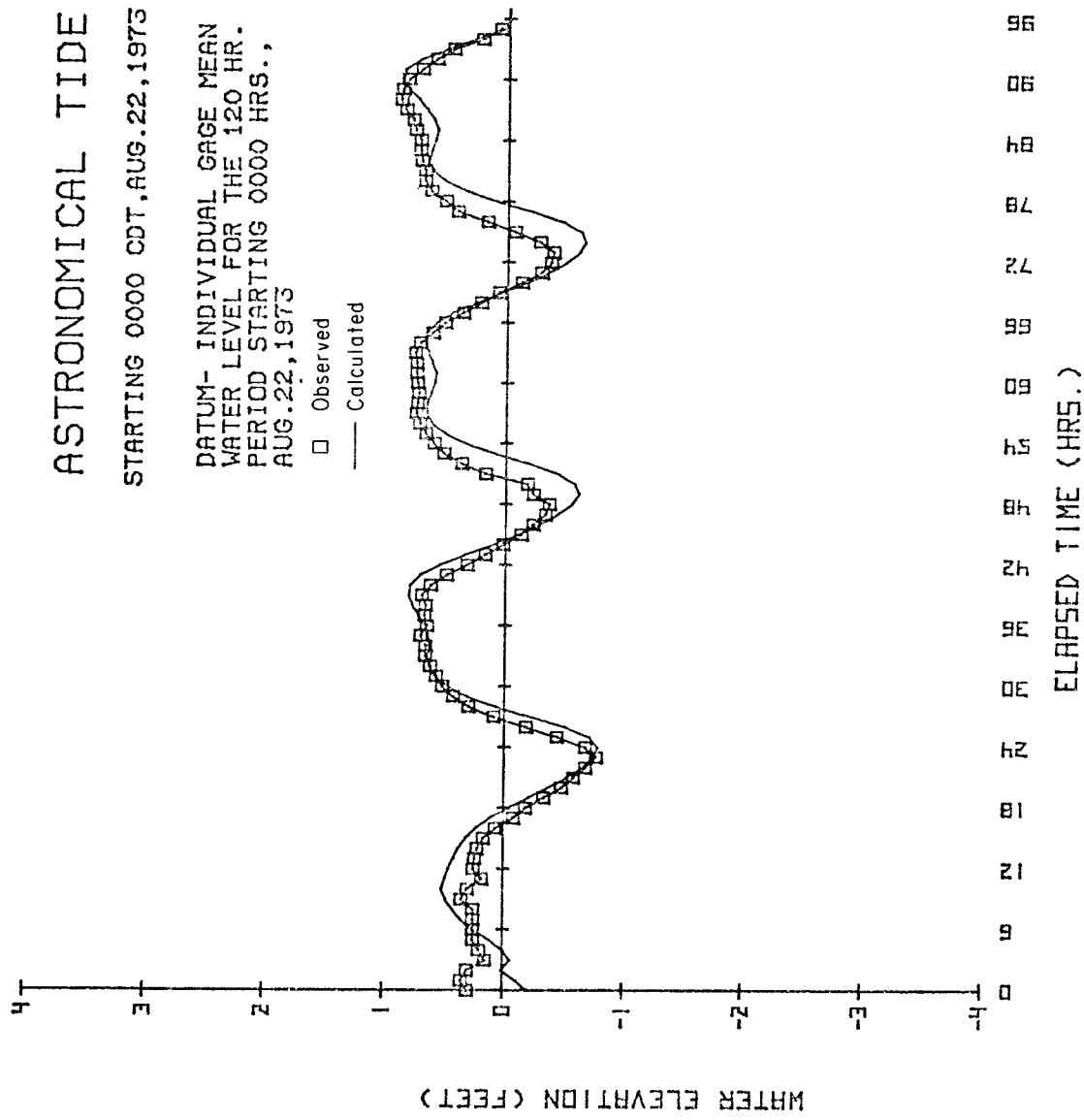


Figure 22. Astronomical tide for Orange Naval Station, Sabine River.

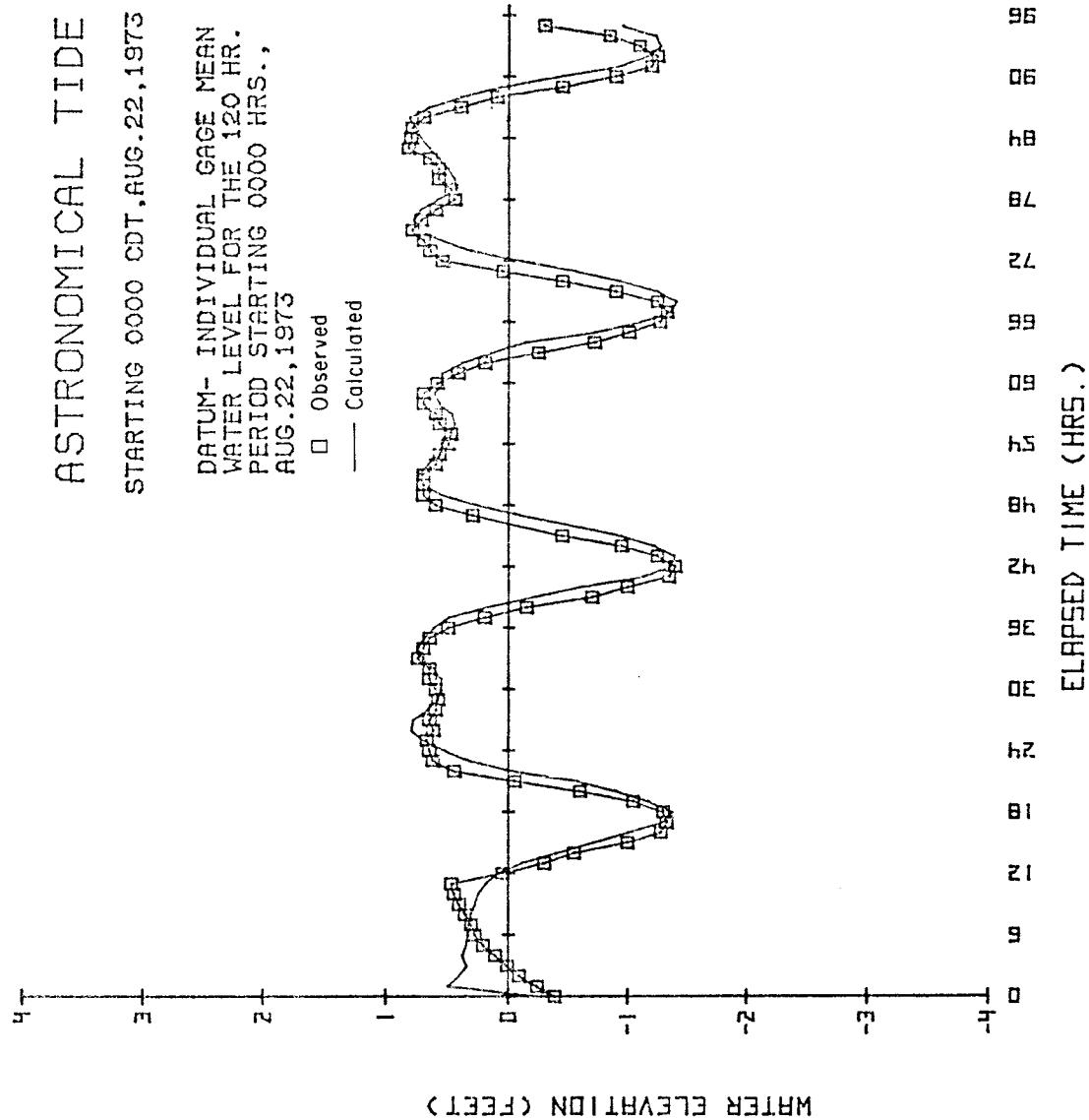


Figure 23. Astronomical tide for Cameron, Calcasieu Pass.

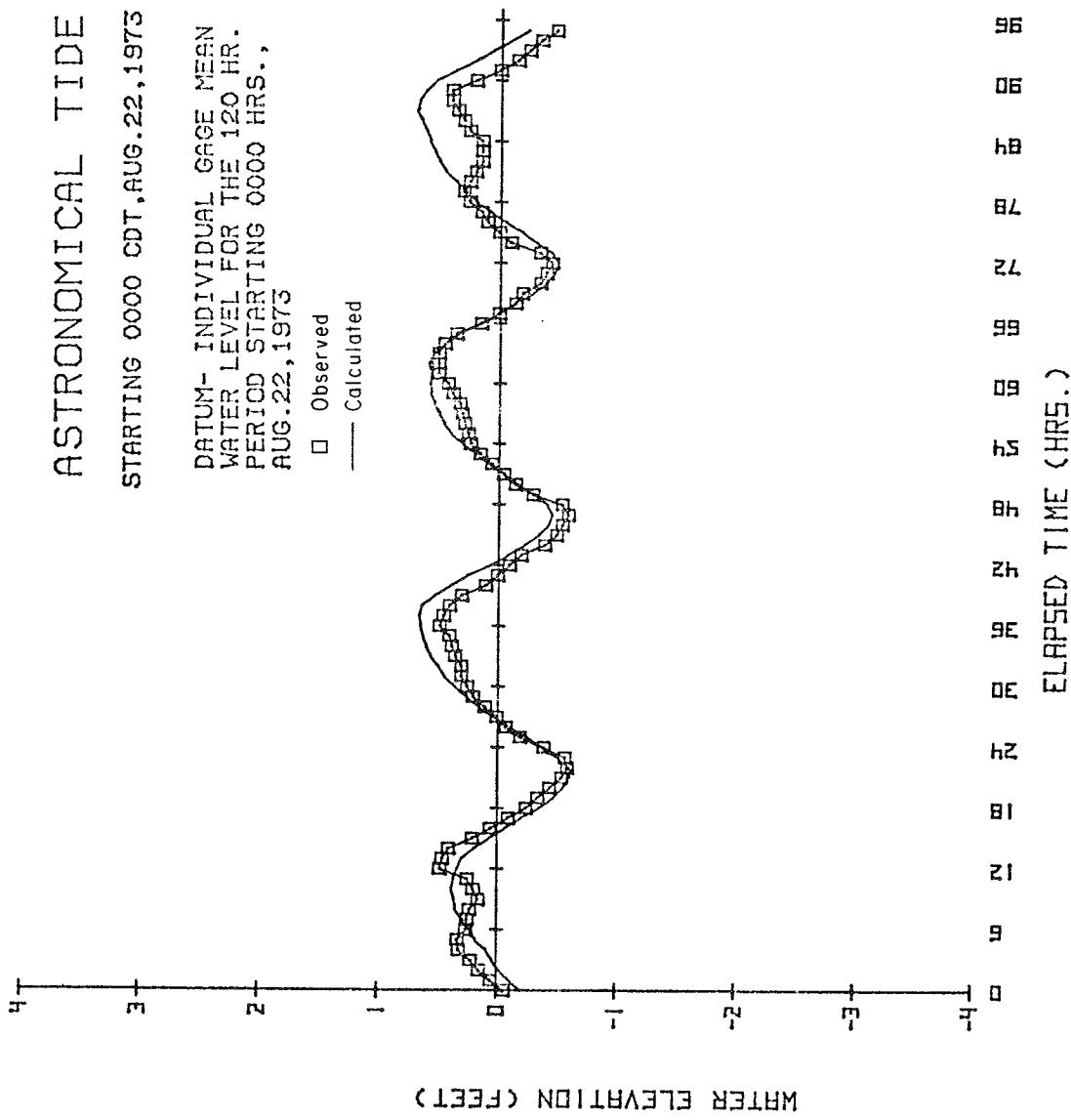


Figure 24. Astronomical tide for Hackberry, Calcasieu River and Pass.

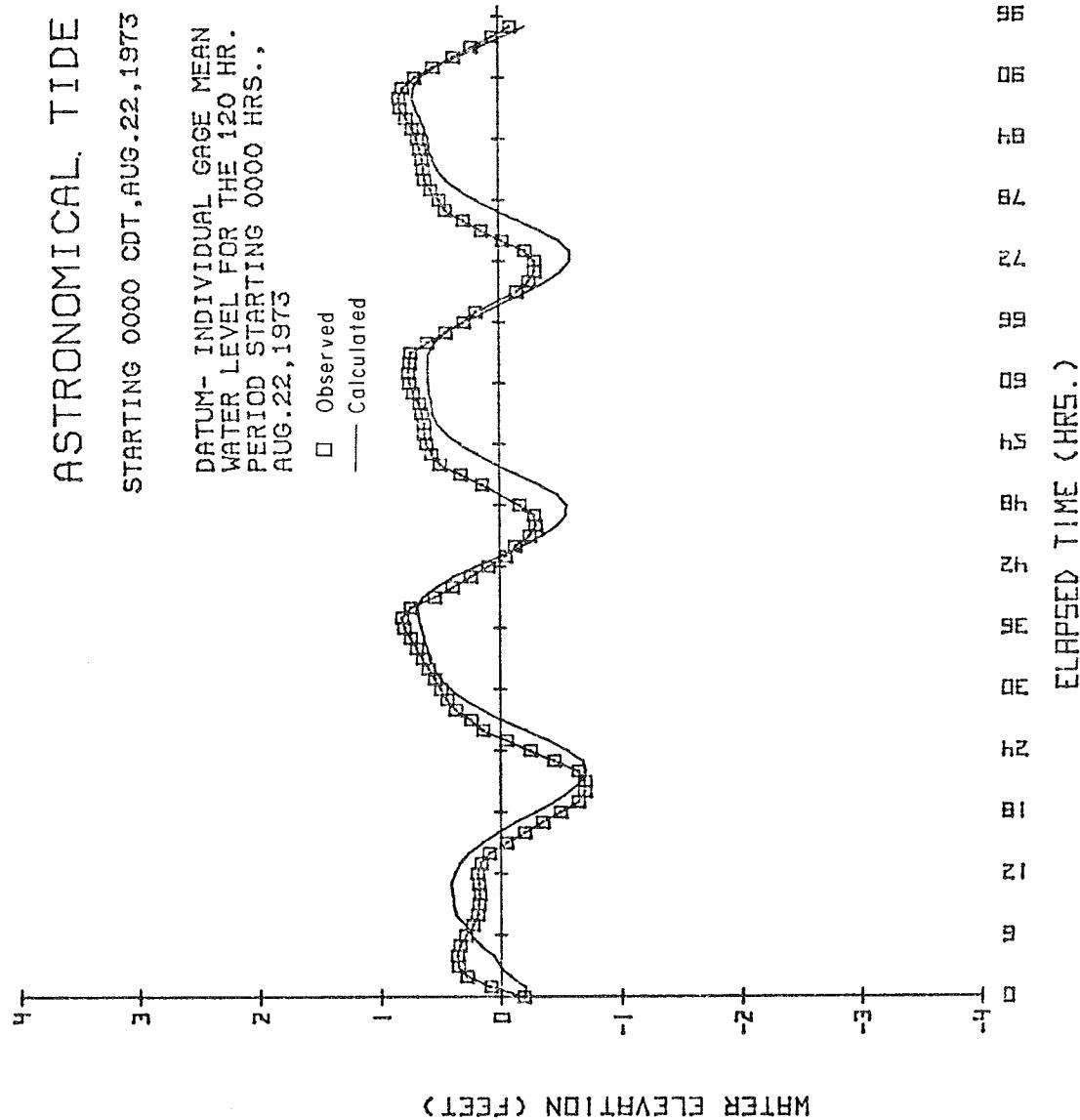


Figure 25. Astronomical tide for Intracoastal Waterway at Calcasieu Lock, west.

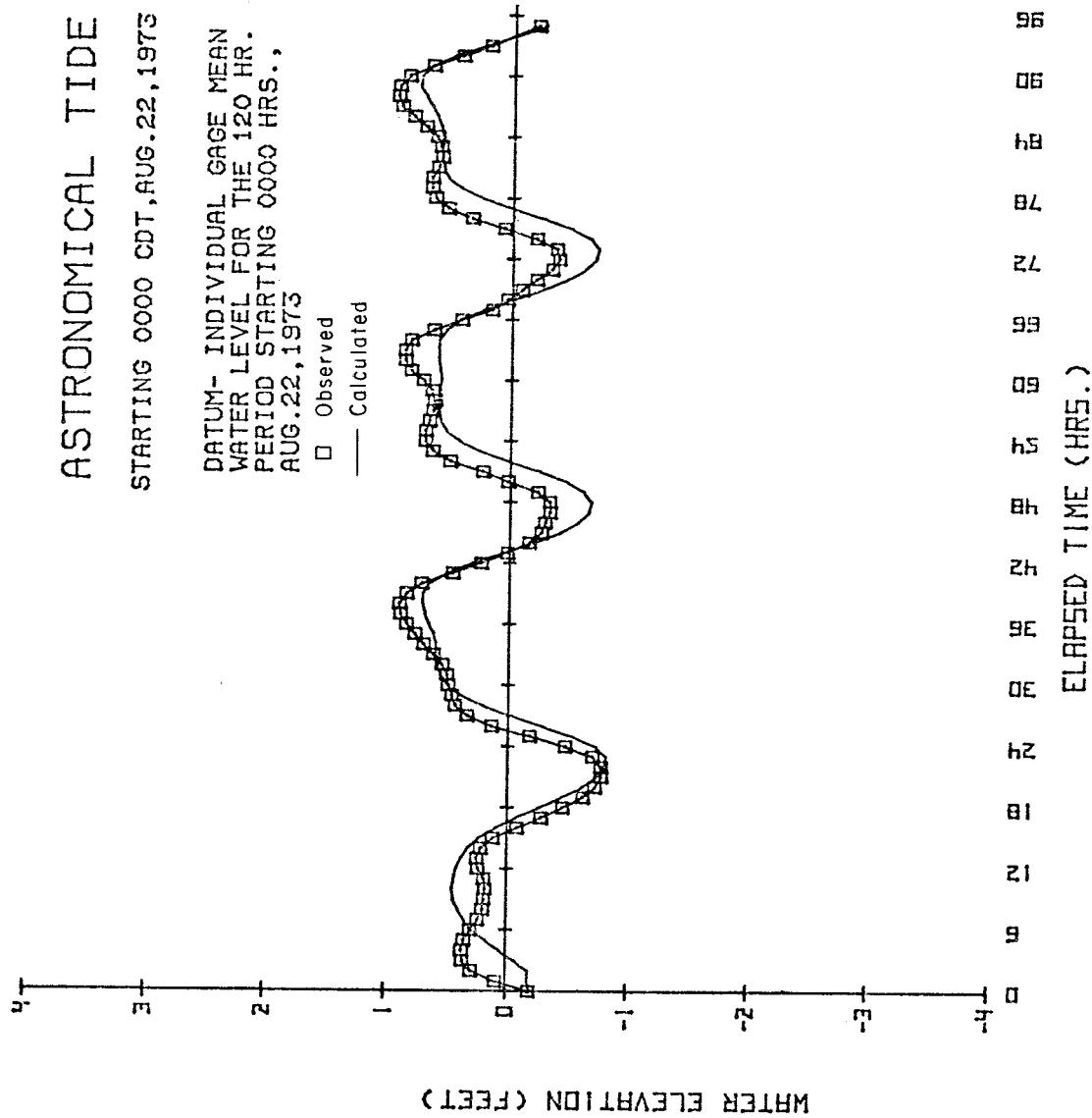


Figure 26. Astronomical tide for Lake Charles, Calcasieu River.

Reproductions of channel output at three different times (30, 60, and 90 hours from start) are shown in Appendix F. The output shows flows (in cubic feet per second), direction of flow, and water level along the various channel reaches at the specified times.

## VI. HURRICANE CARLA VERIFICATION

### 1. Forcing Function Input.

a. Wind-Stress Fields. The x and y components of the wind stress for each 3 hours in a 72-hour period for an 8 by 6 coarse grid for Hurricane Carla are given in the input listings in Appendix D. For convenience in spotting possible errors in input, the wind-stress vectors were plotted, based on the above input, by a special subprogram. Samples of these plots for each 12 hours are shown in Figures 27 to 32. The plots showed suspect entries, which were subsequently corrected before any runs were attempted, and have I increasing upward and J increasing to the left; i.e., the seaward boundary is on the right.

b. River Discharge Input. The river discharges for the Calcasieu River, Nечес River, and Sabine River for each 3 hours are listed as block (IDENT) 12 in Appendix D.

c. Gulf Hydrograph Input. The final input for HG, the water level input along the seaward boundary, was taken as interpolated values between Sabine Pass and Calcasieu Pass with input sequences at those passes adjusted to match the observed values at the Sabine Pass U.S. Coast Guard Station and Cameron after some modification due to flow through these passes. The input is given sequentially at 3-hour intervals along with the wind-field input in Appendix D.

### 2. Further Adjustments and Results.

a. Adjustments. In the series of runs for the Hurricane Carla simulation, it was necessary to make some adjustments in the block topography, particularly in the upper reaches of the Nечес River, in order to provide more ponding area at the levels of flooding encountered. These changes, which are reflected in the final topography (App. D), do not change the results of the astronomical tide calibration because the changes were at levels well above those encountered with the astrotide runs.

A further modification was the reduction of the wind-stress values to 80 percent of those shown in the listings and in the vector plots for the upper left-hand region of the grid. Specifically for I.LE.3 and J.GE.4, the wind-stress components were so reduced in the final runs for Hurricane Carla. This reduction was also used in the later application for Standard Project Hurricane (SPH) simulations. The rationale for this adjustment is based on the greater sheltering in this region due to both topography and vegetation. The initial H for all locations in the bay was taken as 3.2 feet.

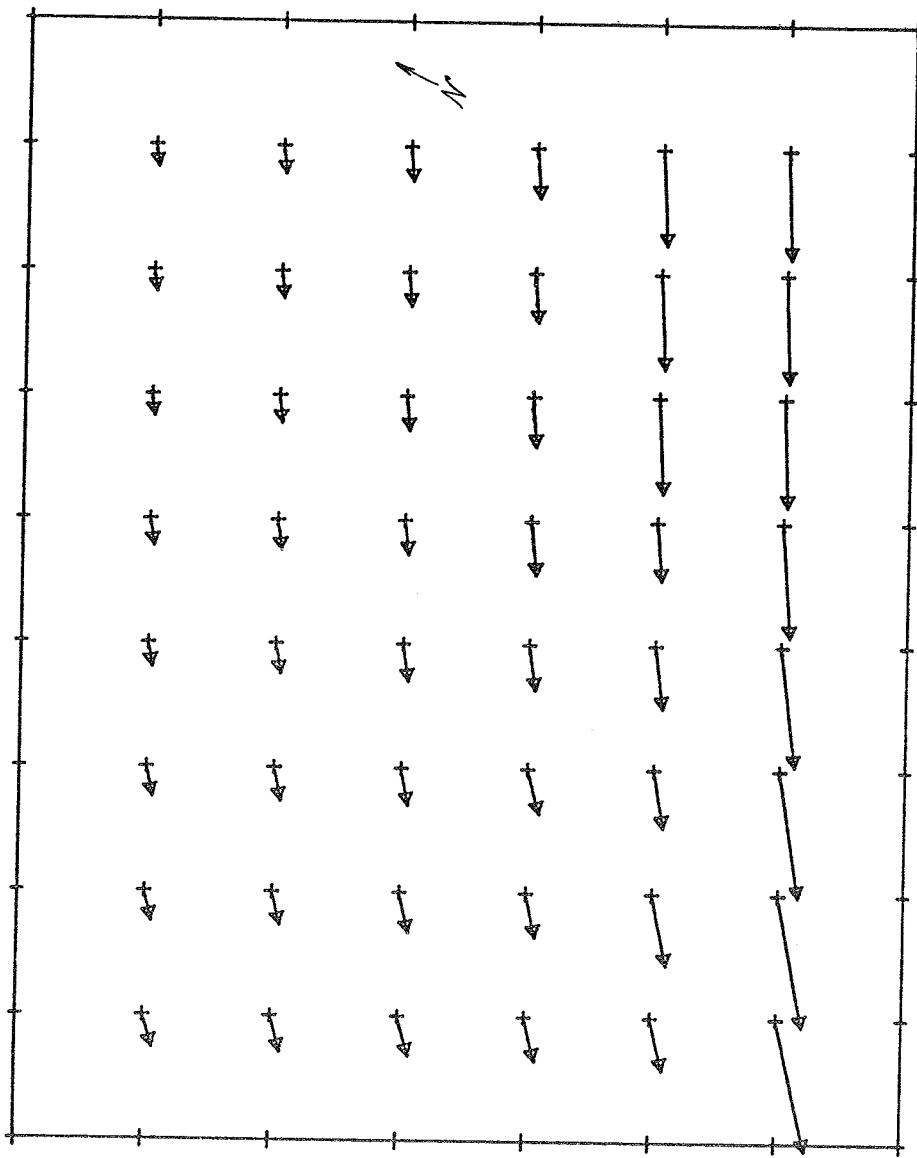


Figure 27. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 12 hours.

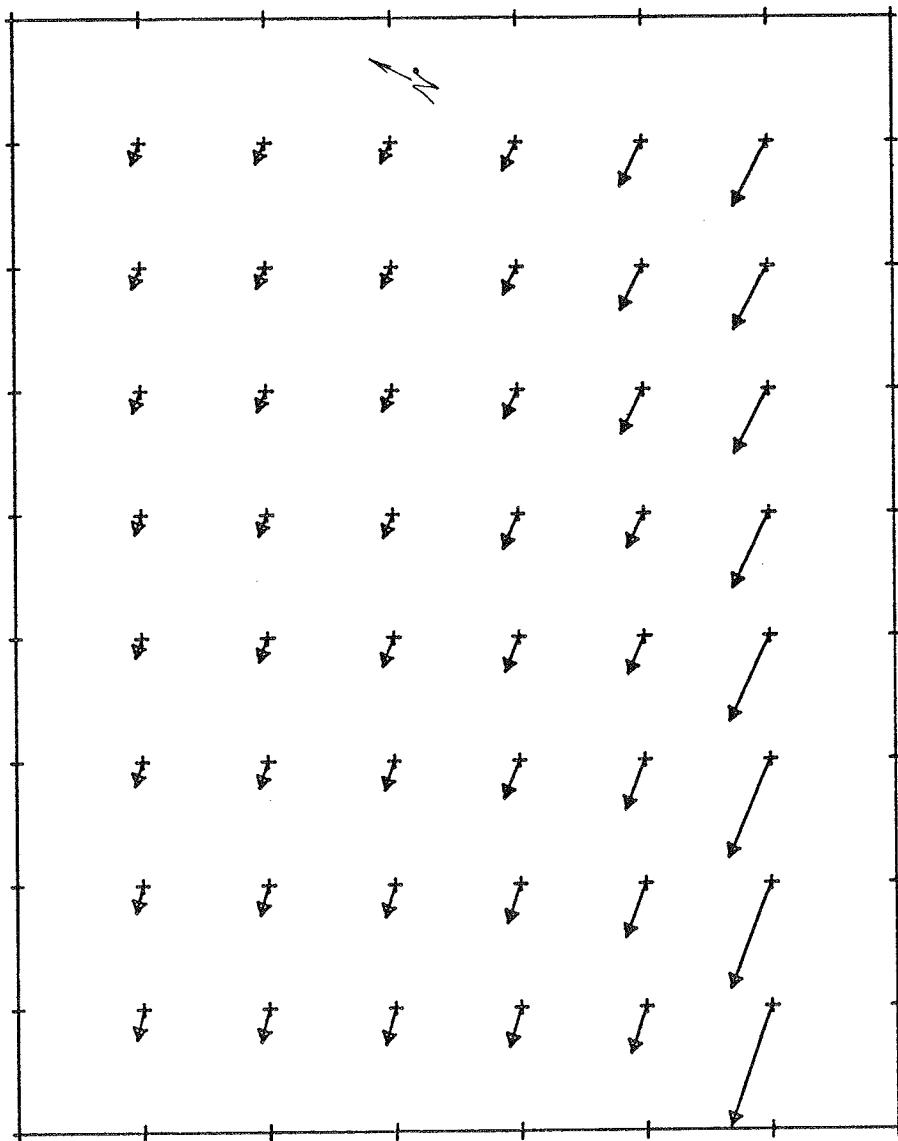


Figure 28. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 24 hours.

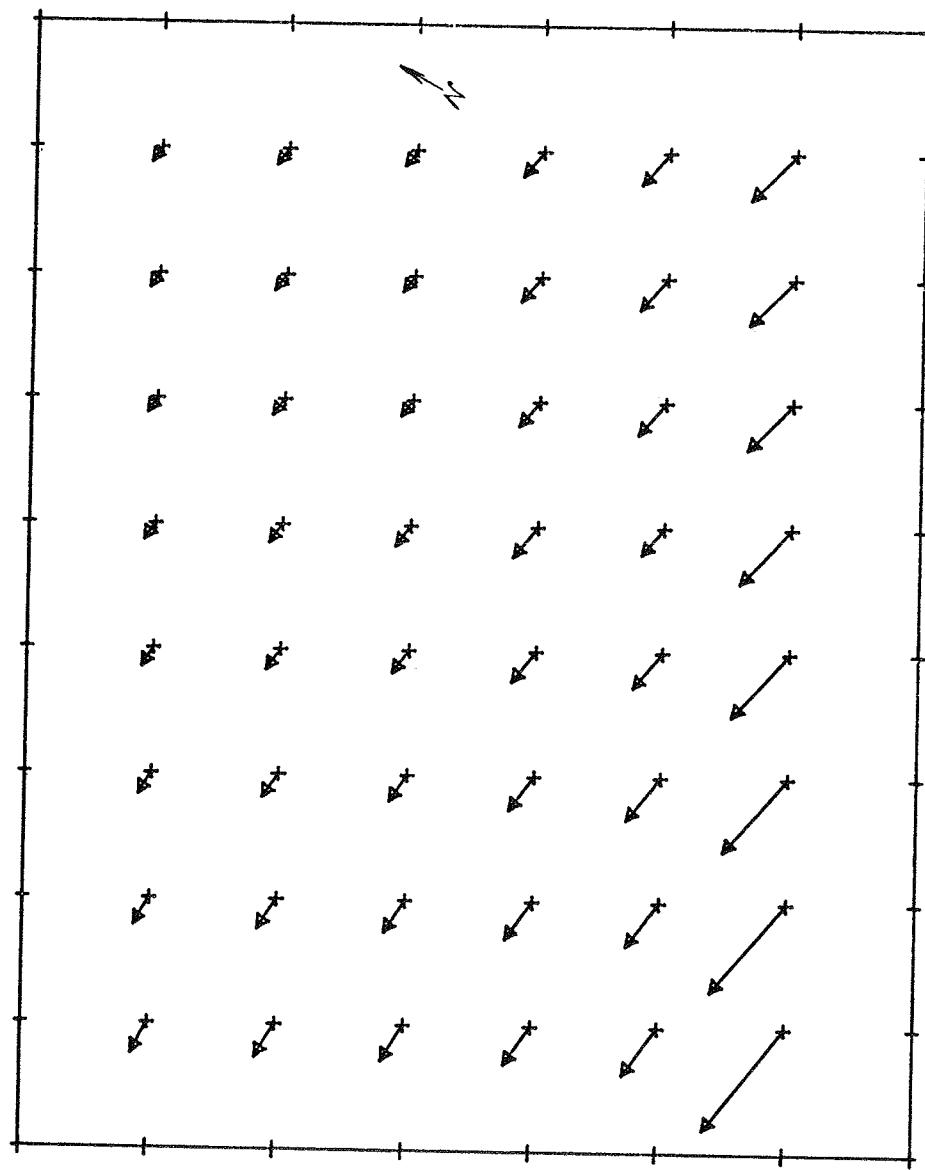


Figure 29. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 36 hours.

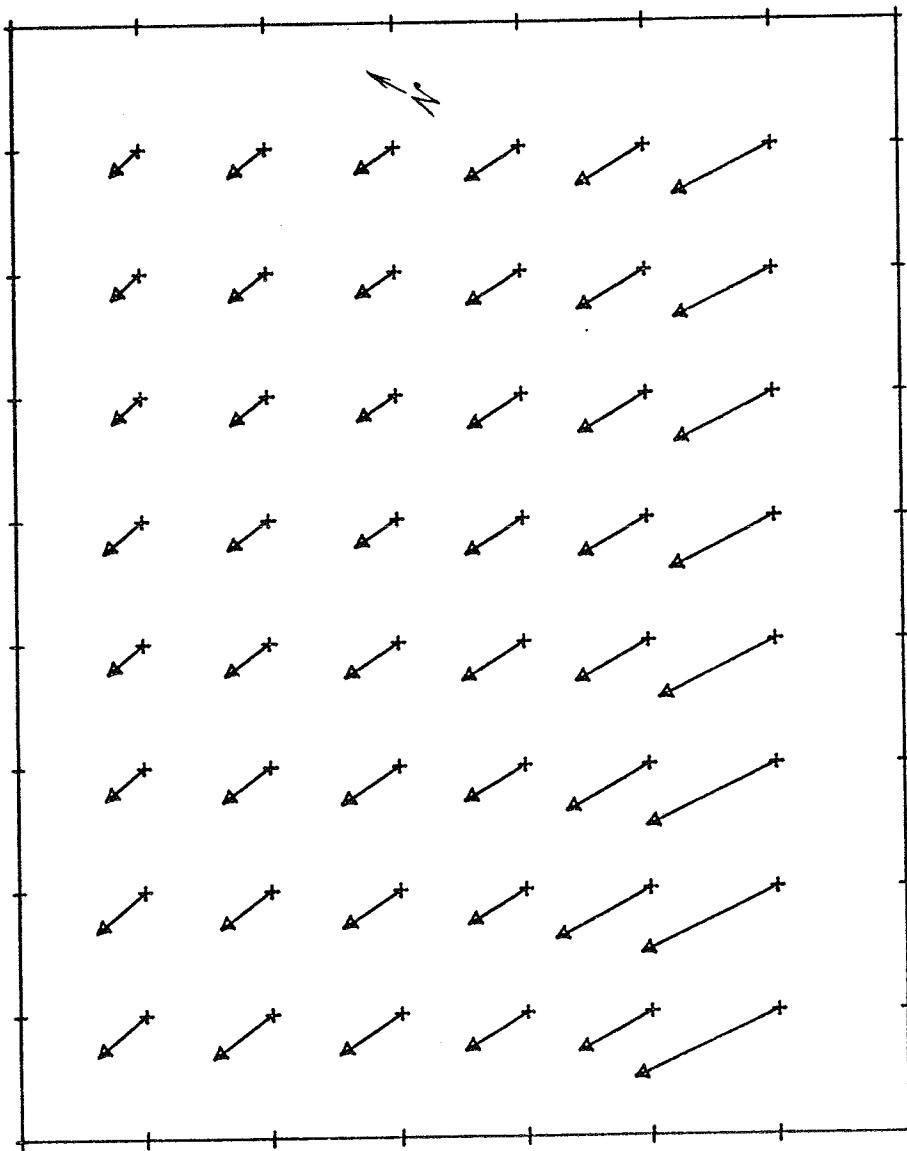


Figure 30. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 48 hours.

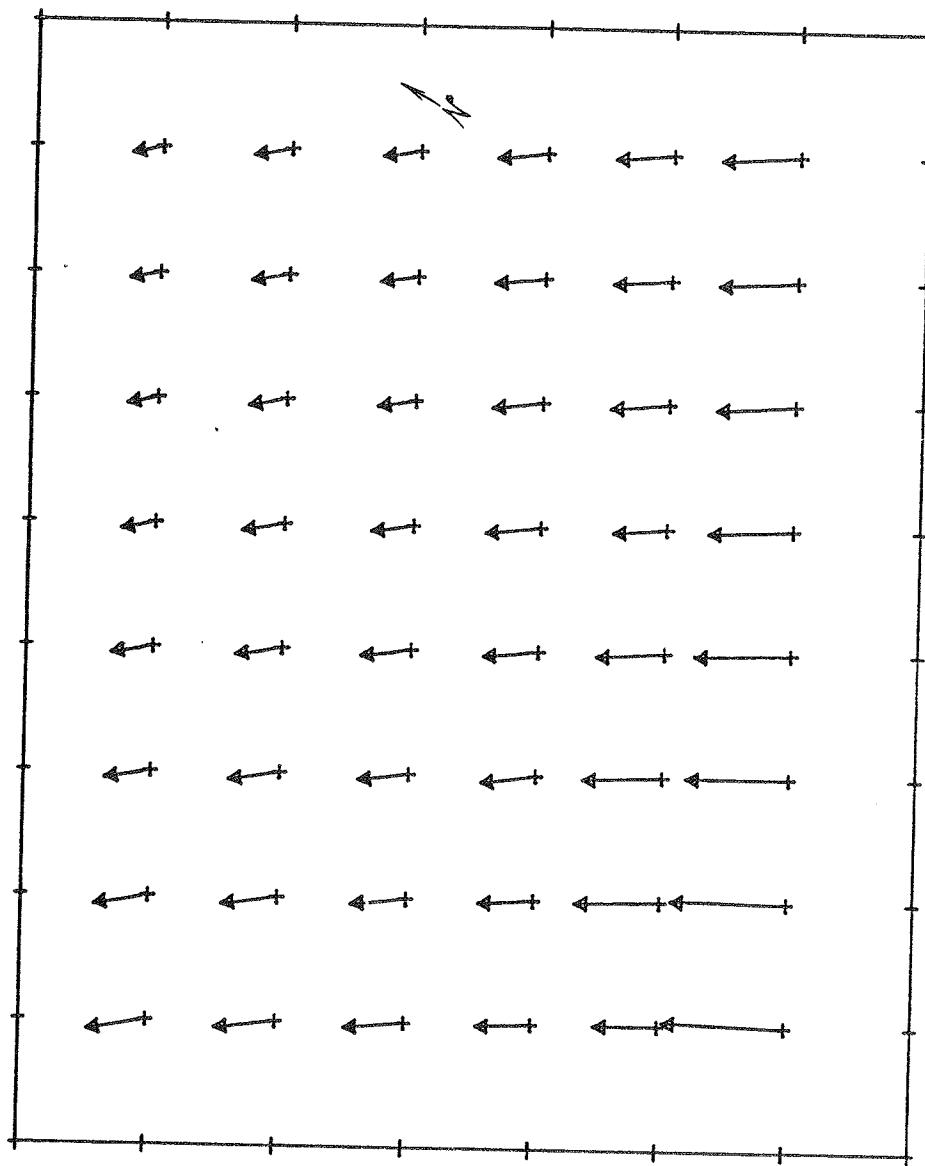


Figure 31. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 54 hours.

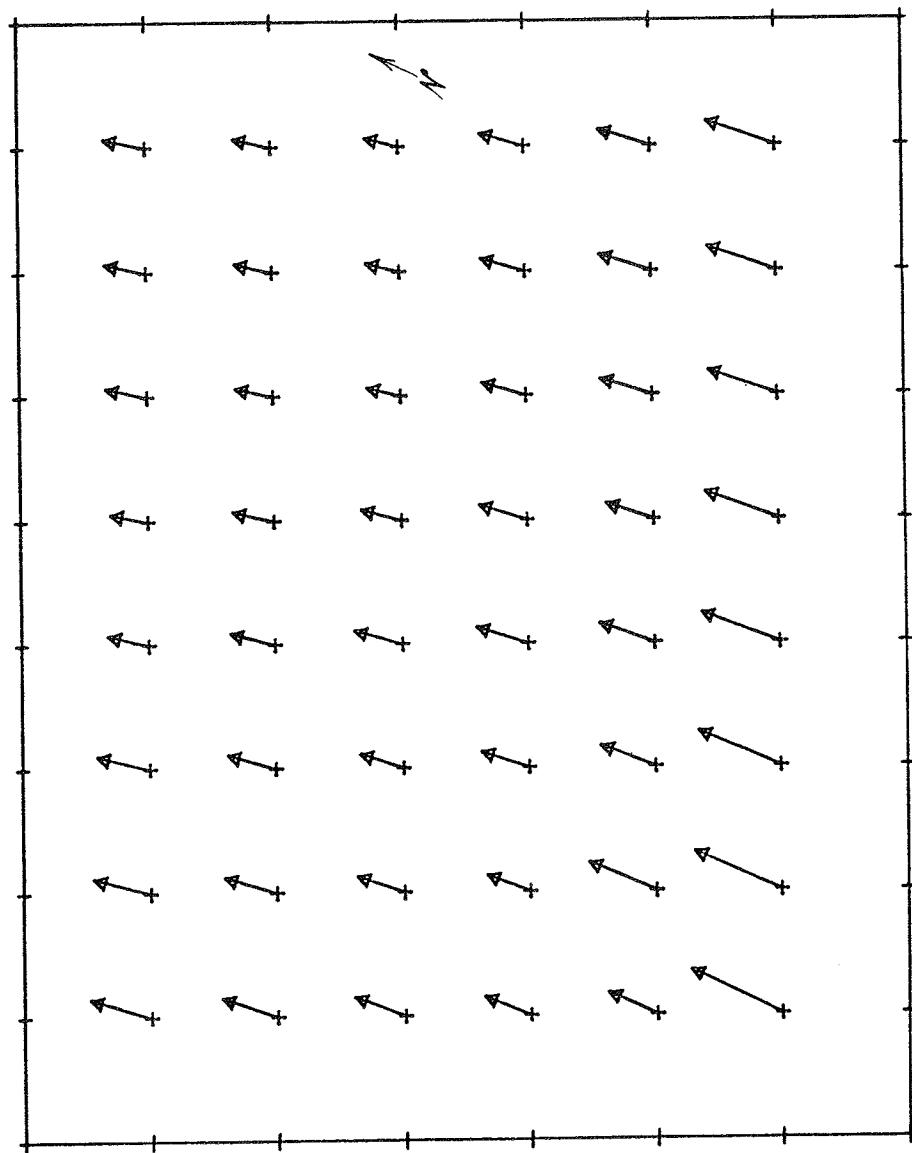


Figure 32. Wind-stress vectors for Hurricane Carla, over Sabine-Calcasieu region on an 8-nautical mile grid; time = 60 hours.

b. Results. The results of the Hurricane Carla simulation are given in Figures 33 to 41, and Appendix G. The input (observed) hydrograph for Sabine Pass is shown in Figure 33 for a 72-hour period starting at 0000 hours c.s.t., 10 September 1961. These results are based on a block friction factor of 0.0010.

The computed and observed values (where available) at gages 2 to 9 are shown in Figures 34 to 41. The principal discrepancy occurs at Beaumont where the computed peak surge exceeds the peak observed value by about 0.8 foot. It was found later that by increasing the block friction to 0.0025, this difference was reduced to 0.4 foot without materially changing the results at other key locations in the system.

The auxiliary sample output for the simulated Hurricane Carla run (App. G) gives, in addition to the serial listings of the above hydrographs and flow at the two main passes, sample listings of channel output at elapsed times of 30 and 60 hours.

## VII. STANDARD PROJECT HURRICANE (SPH)

### 1. LR-ST Storm Data.

The large radius, slow translation (LR-ST) storm was utilized as an atmospheric forcing function for the verified model of the Sabine-Calcasieu system. The storm parameters were extracted from the pertinent gulf coast section of the National Hurricane Research Project Report No. 33 (Graham and Nunn, 1959). Table 4 lists these values which were also used in conjunction with the analytic storm representation given by Jelesnianski (1965).

Table 4. Atmospheric parameters for the large radius, slow translation (ST) and medium translation (MT) storms.

Parameters	ST storm	MT storm
Radius to maximum winds	27 nmi	27 nmi
Maximum windspeed	100 mi/h	100 mi/h
Central pressure	27.55 in	27.55 in
Translation speed	4 kn	11 kn

Wind-stress vector plots have been prepared beginning at  $t = 30$  hours and at 10-hour increments to  $t = 80$  hours (Figs. 42 to 47). The storm track, which is taken normal to the general shoreline, has the Sabine-Calcasieu system on the right-hand side of the storm approaching the coastline. Landfall of the storm center is close to grid block 1,1. The orientation of these plots relative to the topography is similar to the wind fields shown for the Hurricane Carla verification. The gulf hydrographic input, provided by the Galveston District, was developed by an application of a one-dimensional bathystrophic model (Marinos and Woodward, 1968; Bodine, 1971). A tidal component has been added to this

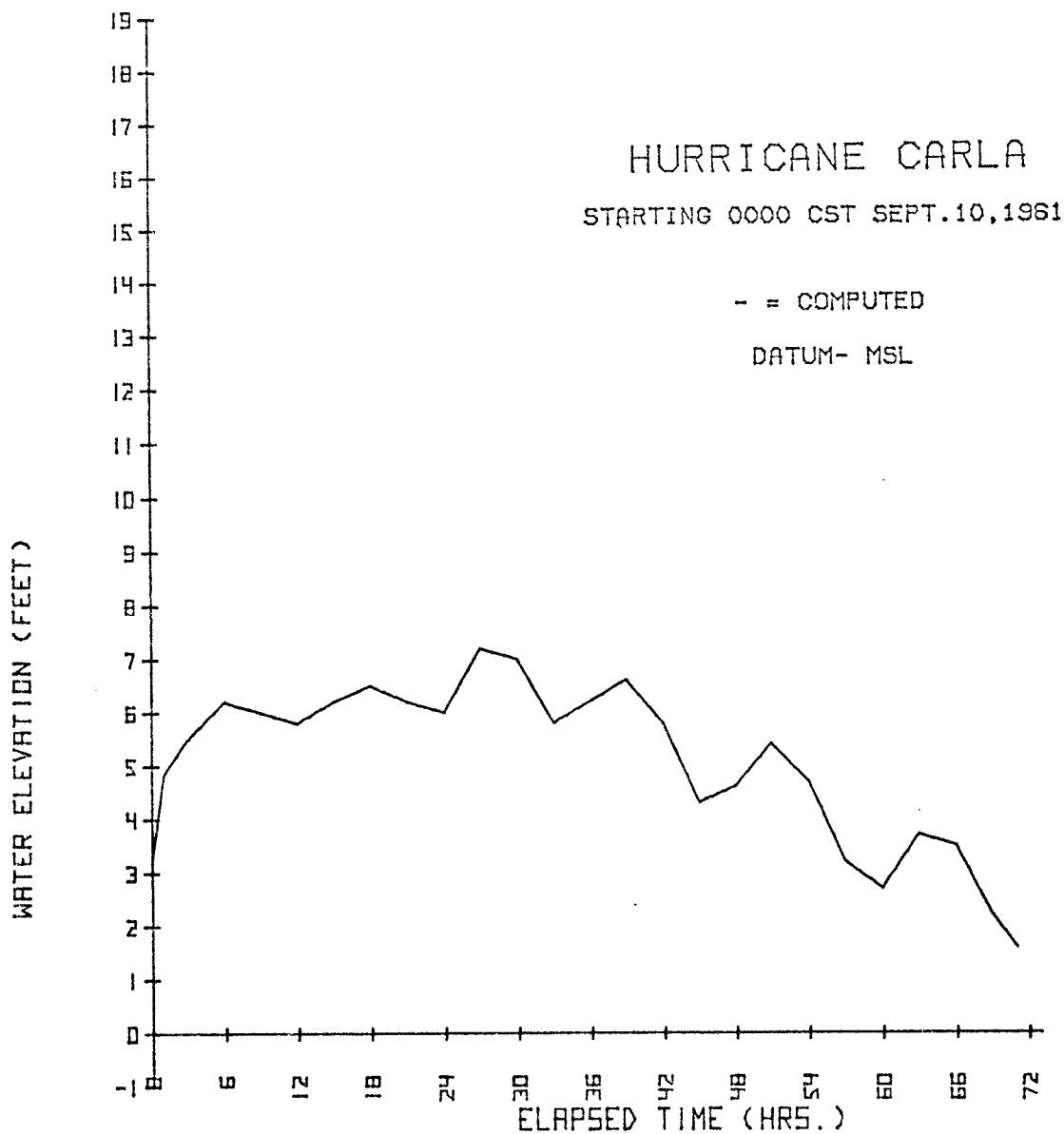


Figure 33. Hydrograph at Sabine Pass, southwest jetty for Hurricane Carla.

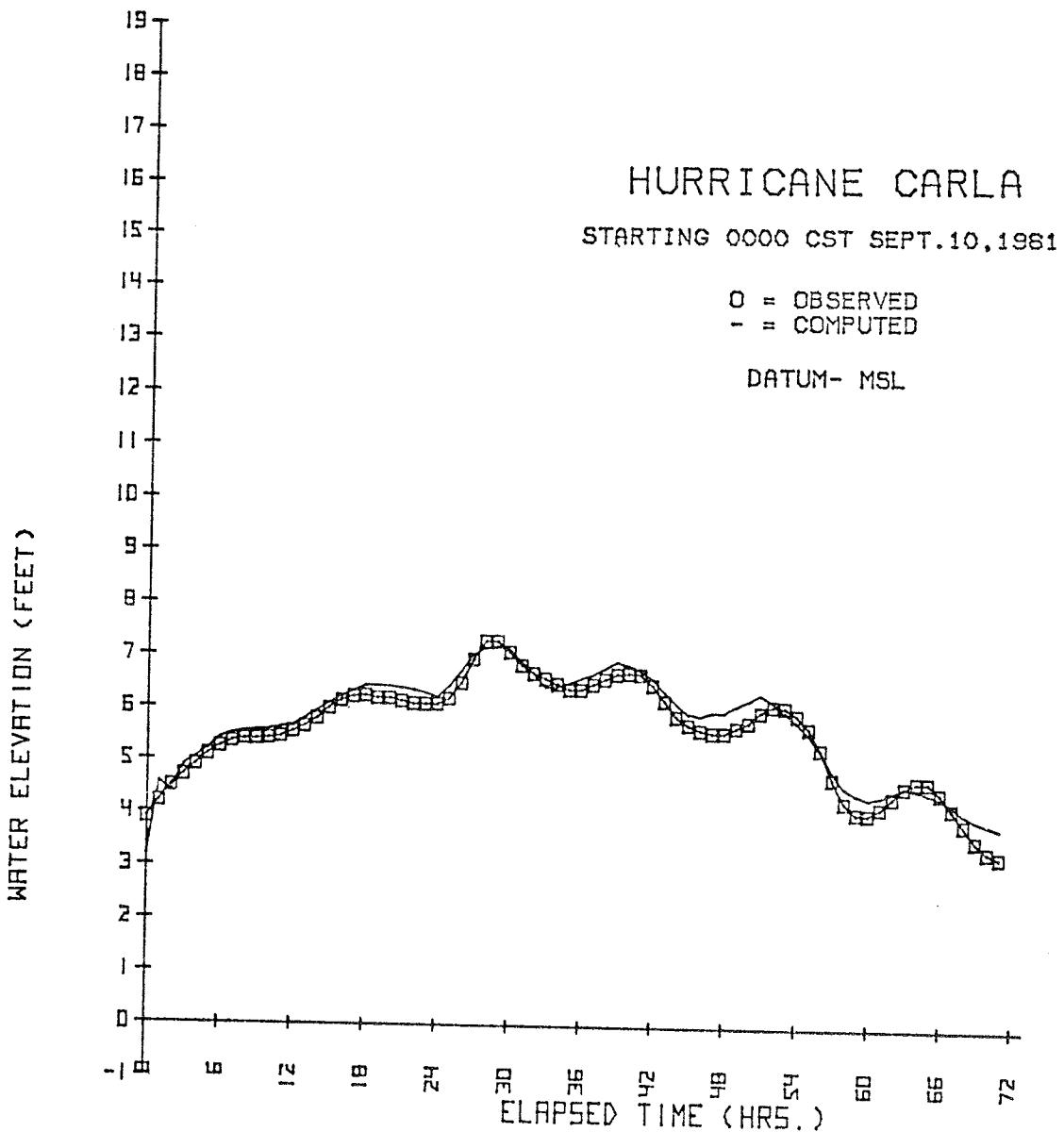


Figure 34. Hydrographs at Sabine Pass, U.S. Coast Guard Station for Hurricane Carla ( $FK = 0.0010$ ).

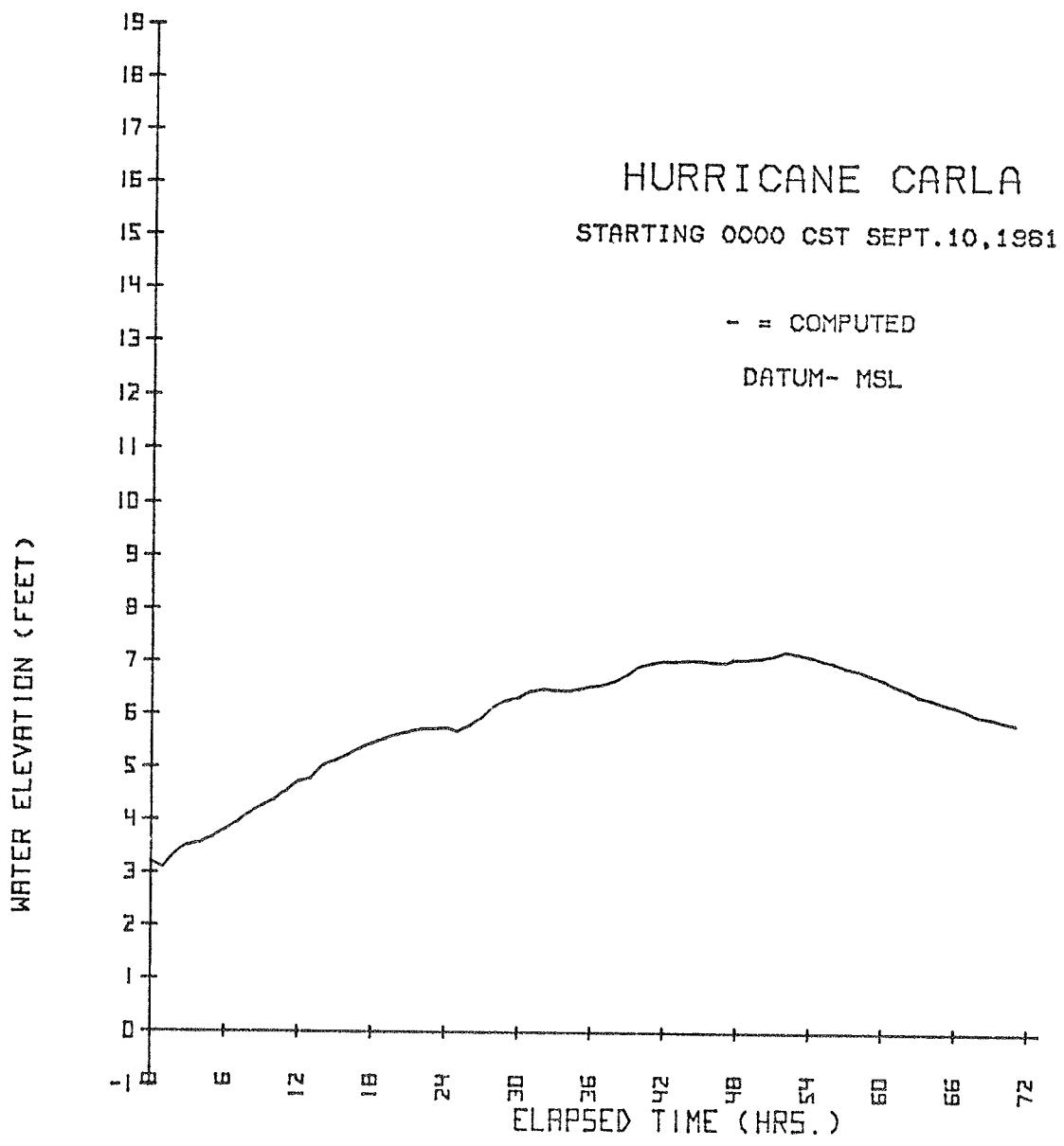


Figure 35. Hydrograph at Port Arthur for Hurricane Carla ( $FK = 0.0010$ ).

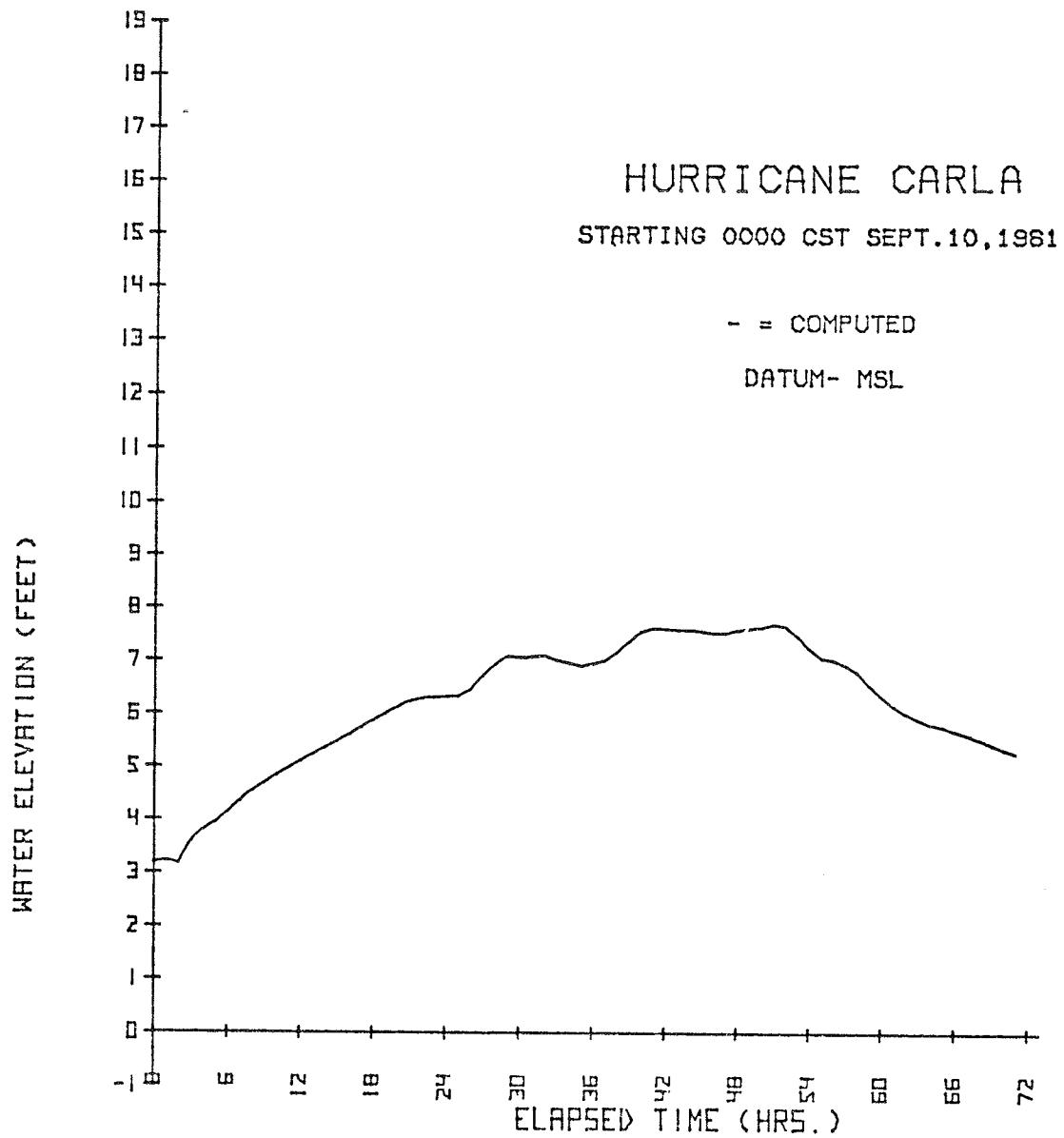


Figure 36. Hydrograph at north Sabine Lake for Hurricane Carla (FK = 0.0010).

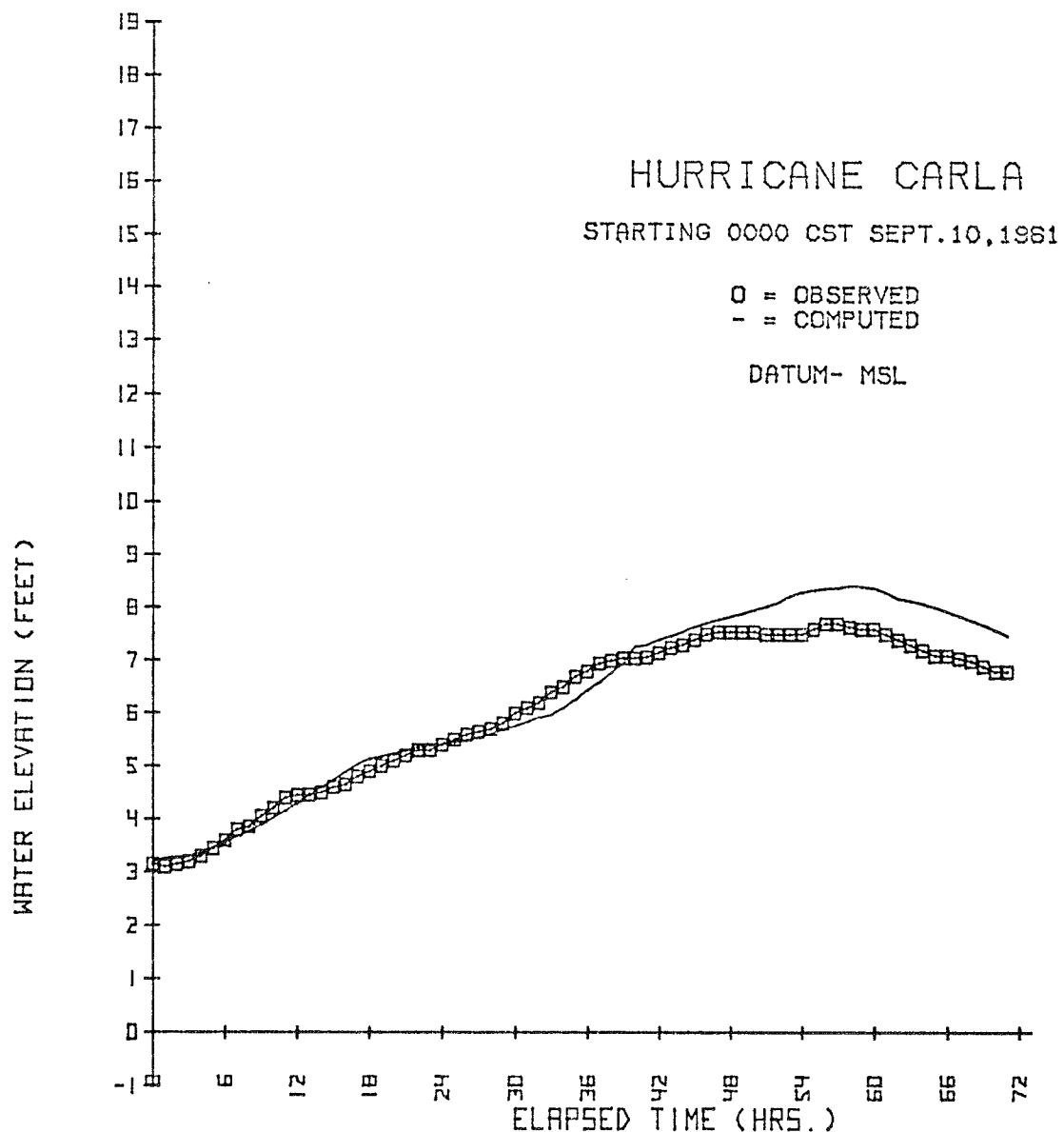


Figure 37. Hydrographs at Beaumont, Neches River, and Brakes Bayou for Hurricane Carla ( $FK = 0.0010$ ).

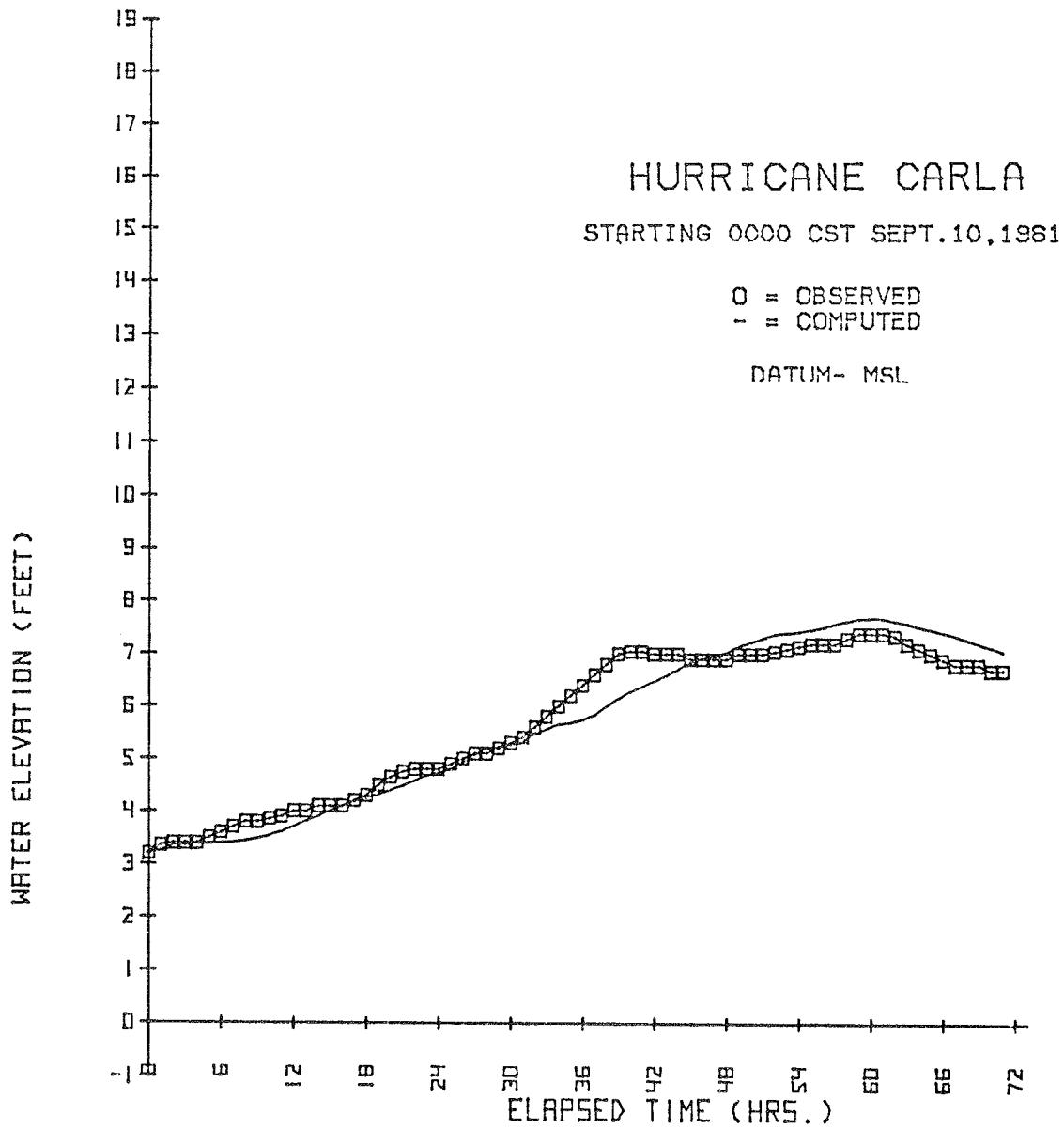


Figure 38. Hydrographs at Orange Naval Station, Sabine River for Hurricane Carla ( $FK = 0.0010$ ).

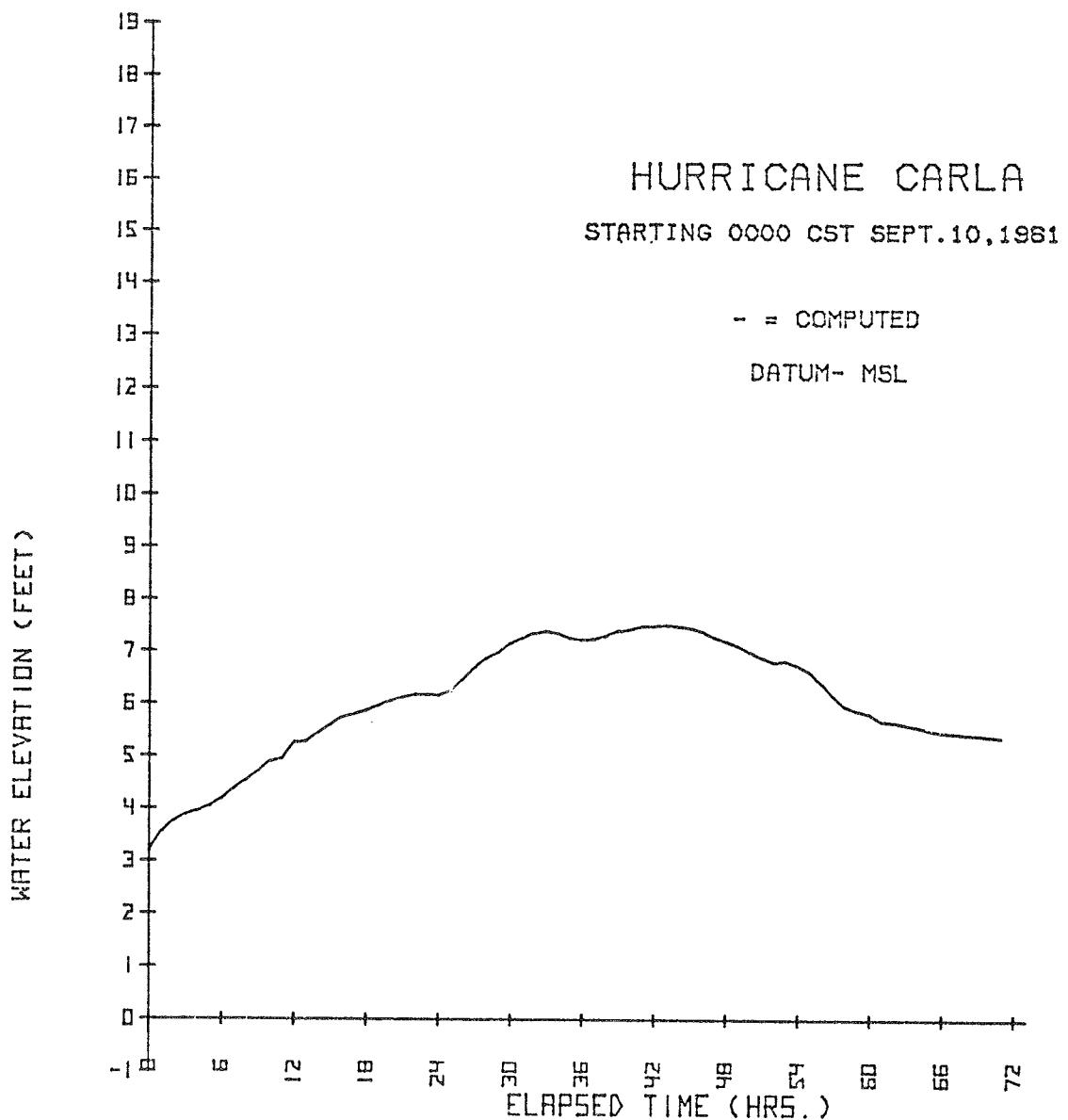


Figure 39. Hydrograph at west end of Intracoastal Waterway for Hurricane Carla ( $FK = 0.0010$ ).

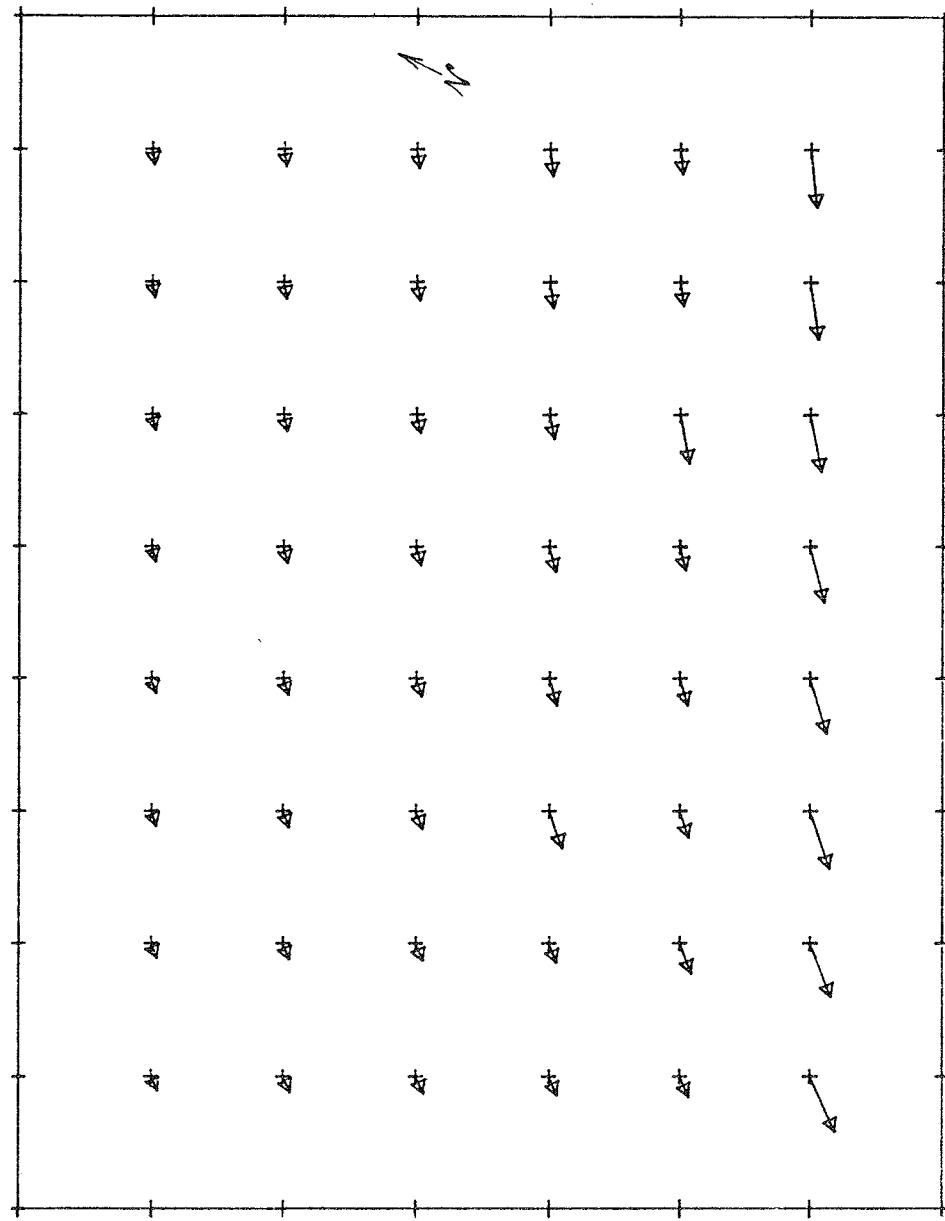


Figure 42. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 30 hours.

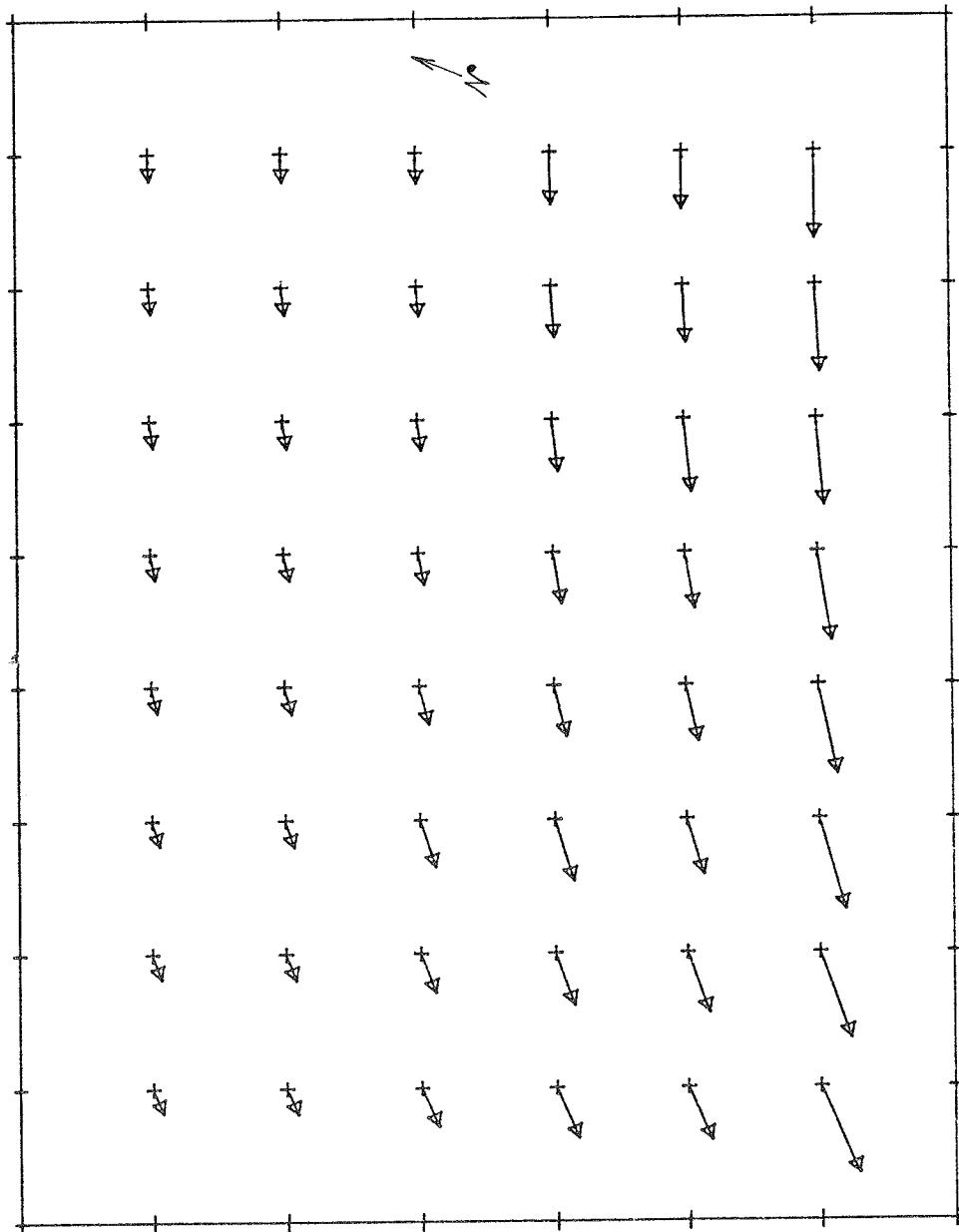


Figure 43. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 40 hours.

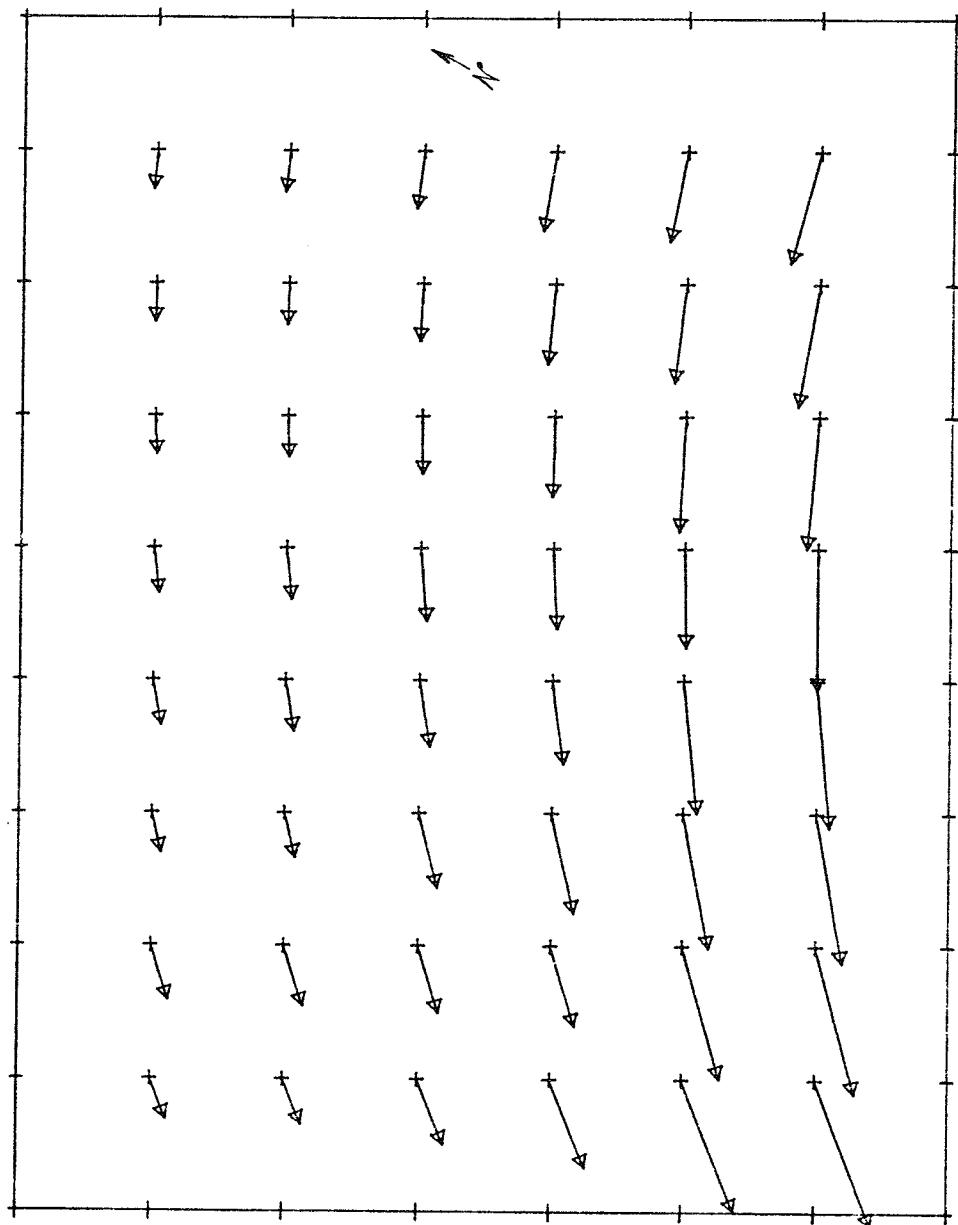


Figure 44. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 50 hours.

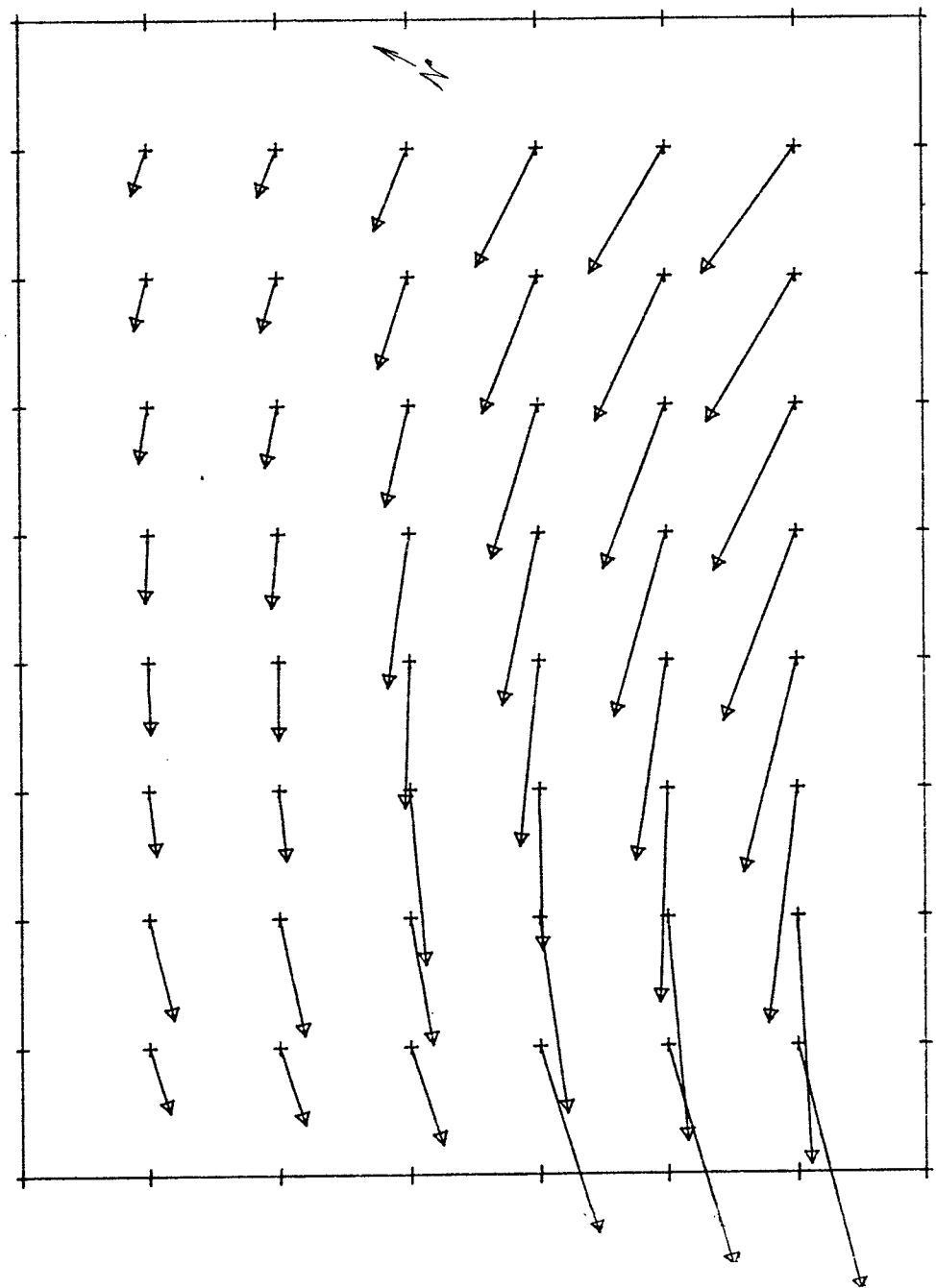


Figure 45. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 60 hours.

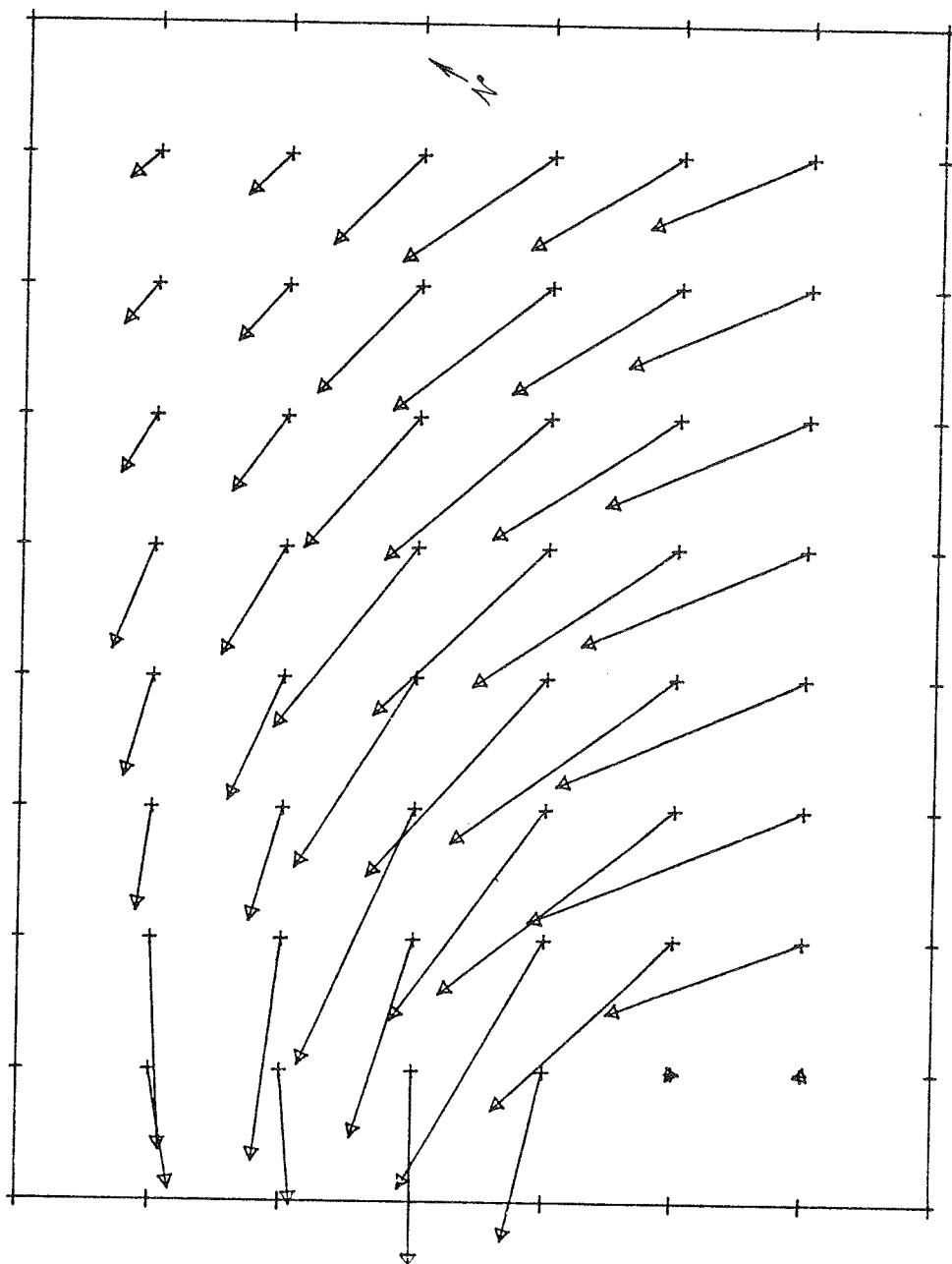


Figure 46. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 70 hours.

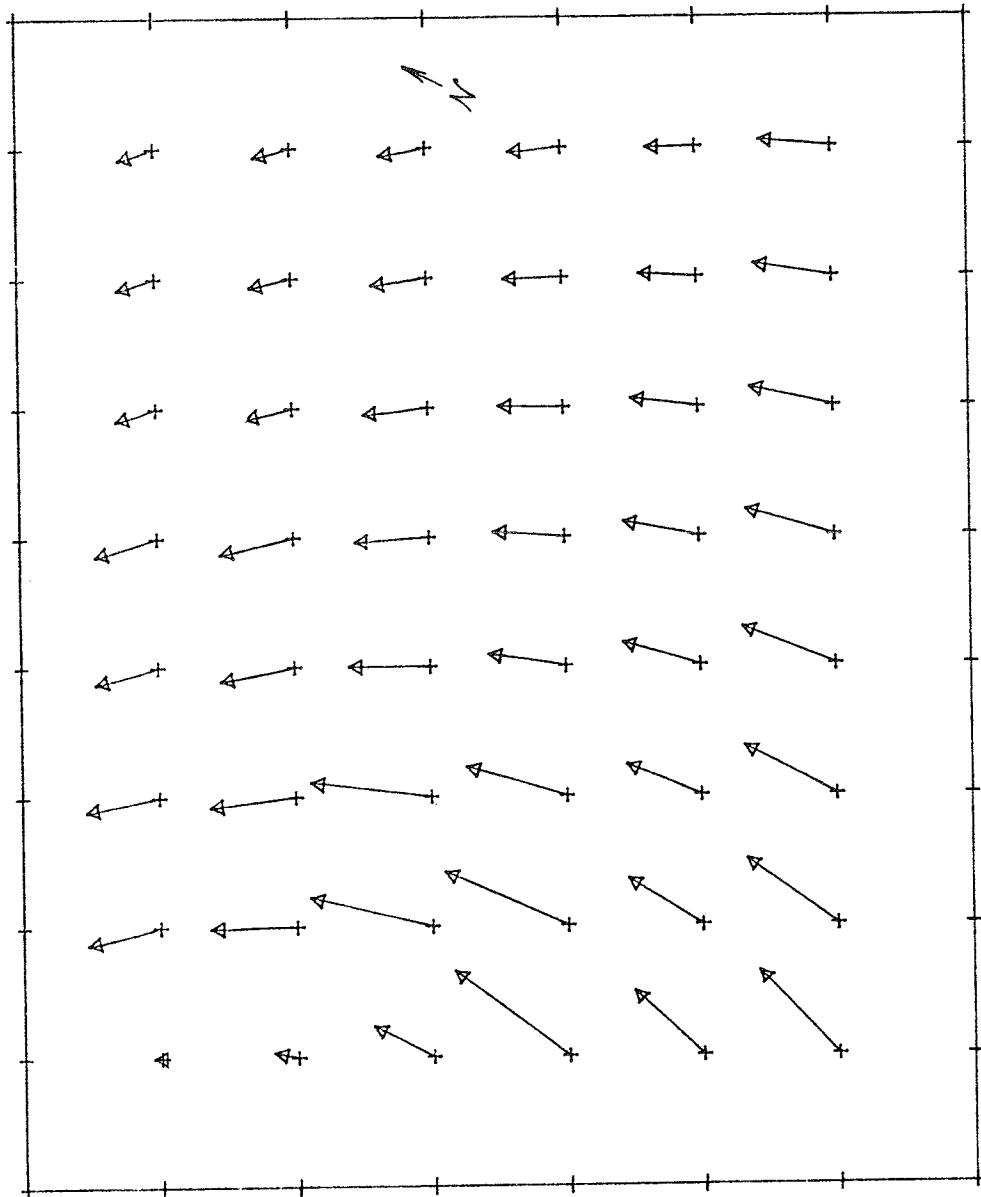


Figure 47. Wind-stress vectors for SPH large radius, slow translation (LR-ST) on an 8-nautical mile grid; time = 80 hours.

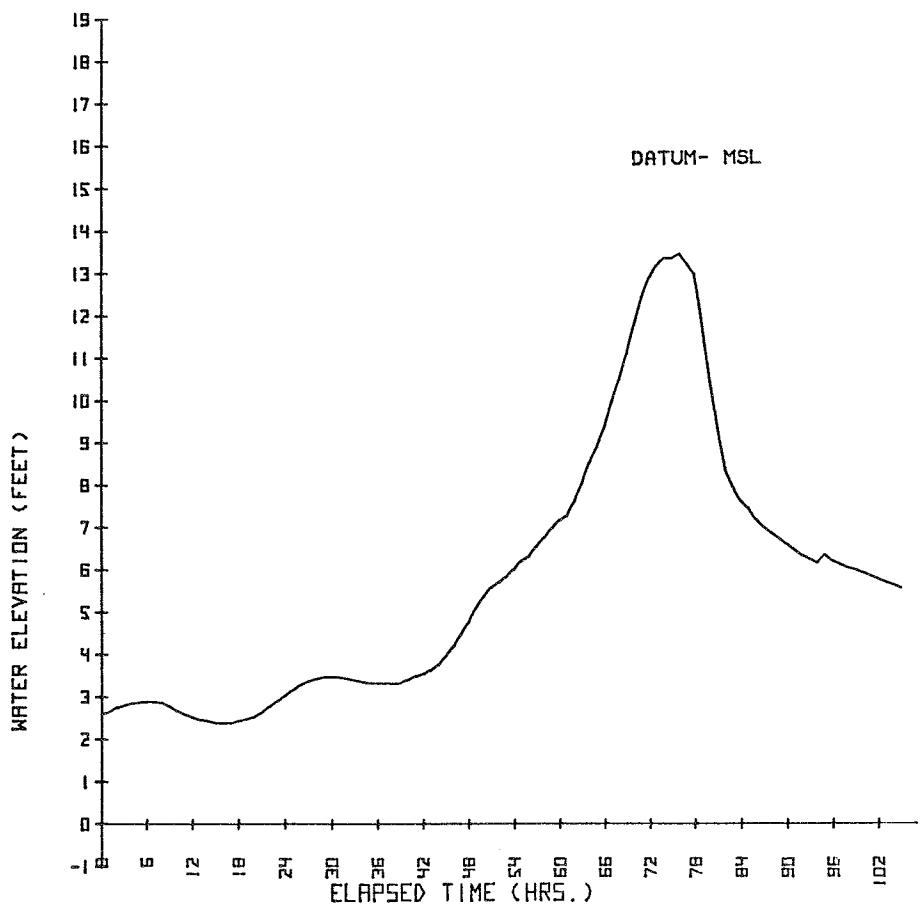


Figure 49. Hydrograph for SPH, LR-ST at Sabine Pass, U.S. Coast Guard Station (FK = 0.0010).

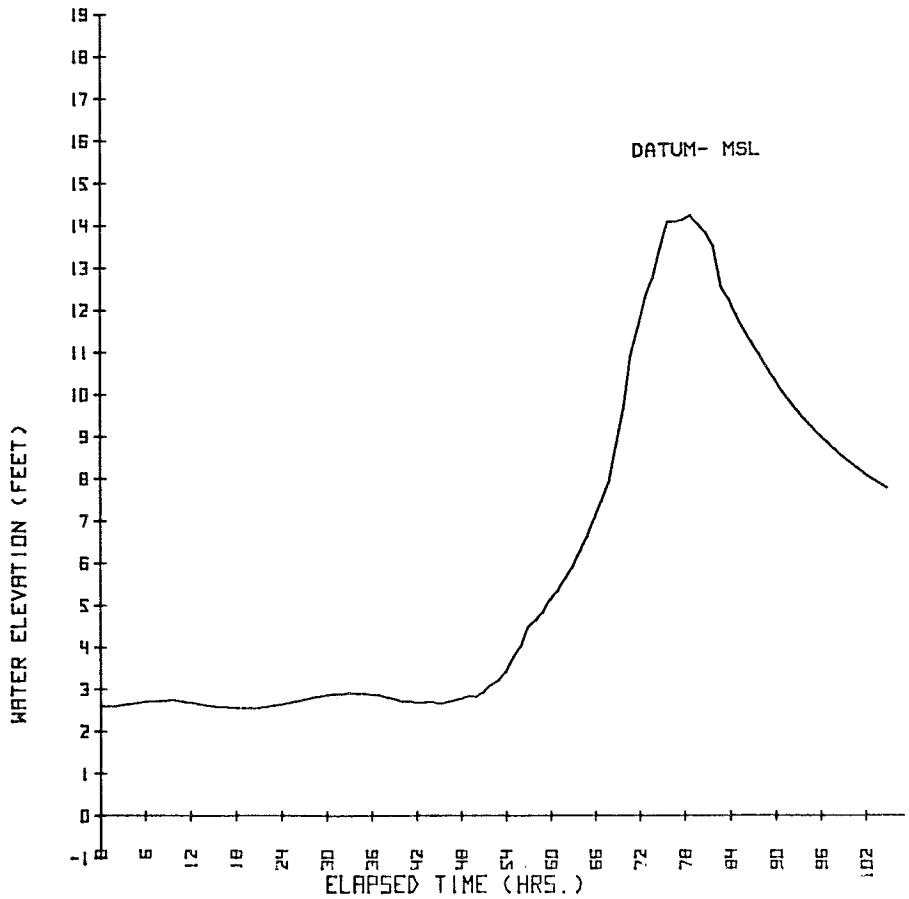


Figure 50. Hydrograph for SPH, LR-ST at Port Arthur (FK = 0.0010).

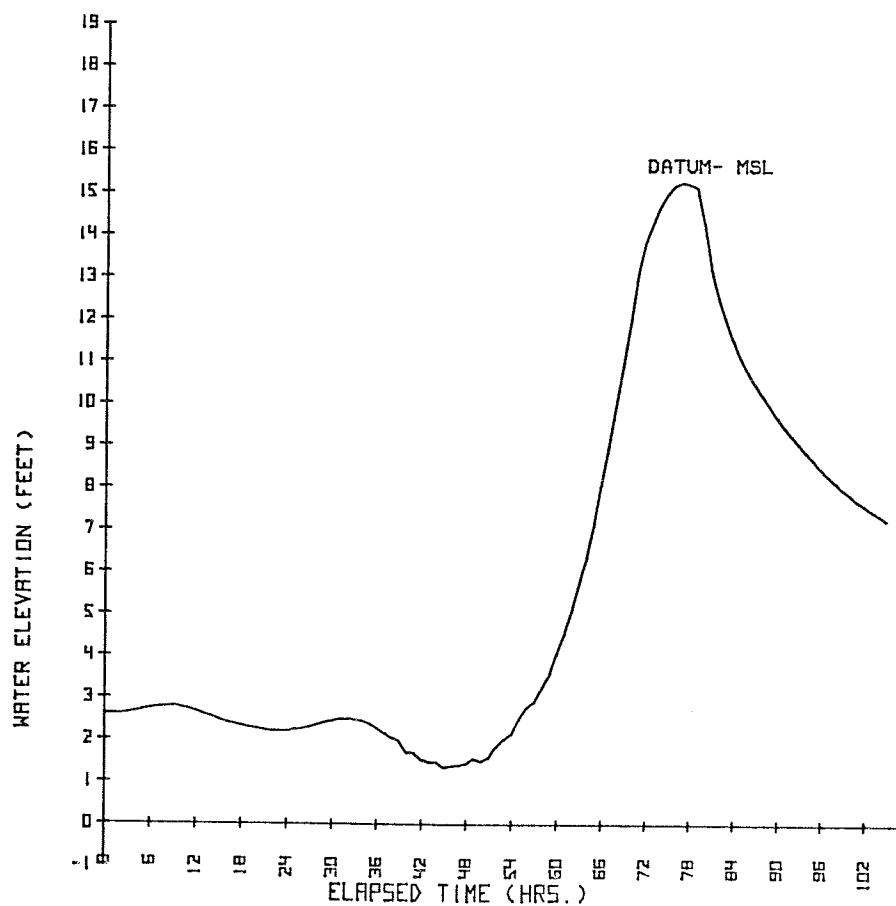


Figure 51. Hydrograph for SPH, LR-ST at north Sabine Lake  
(FK = 0.0010).

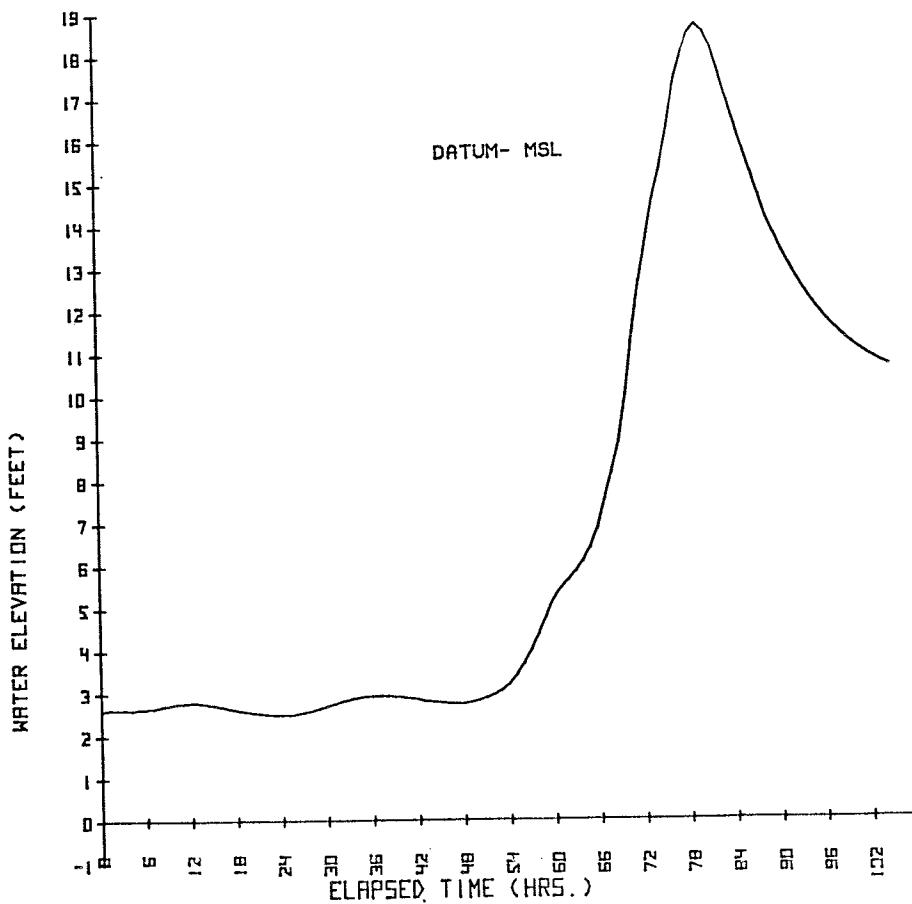


Figure 52. Hydrograph for SPH, LR-ST at Beaumont, Naches River, and Brakes Bayou.

SPH: ORANGE NAVAL STATION, SABINE RIVER

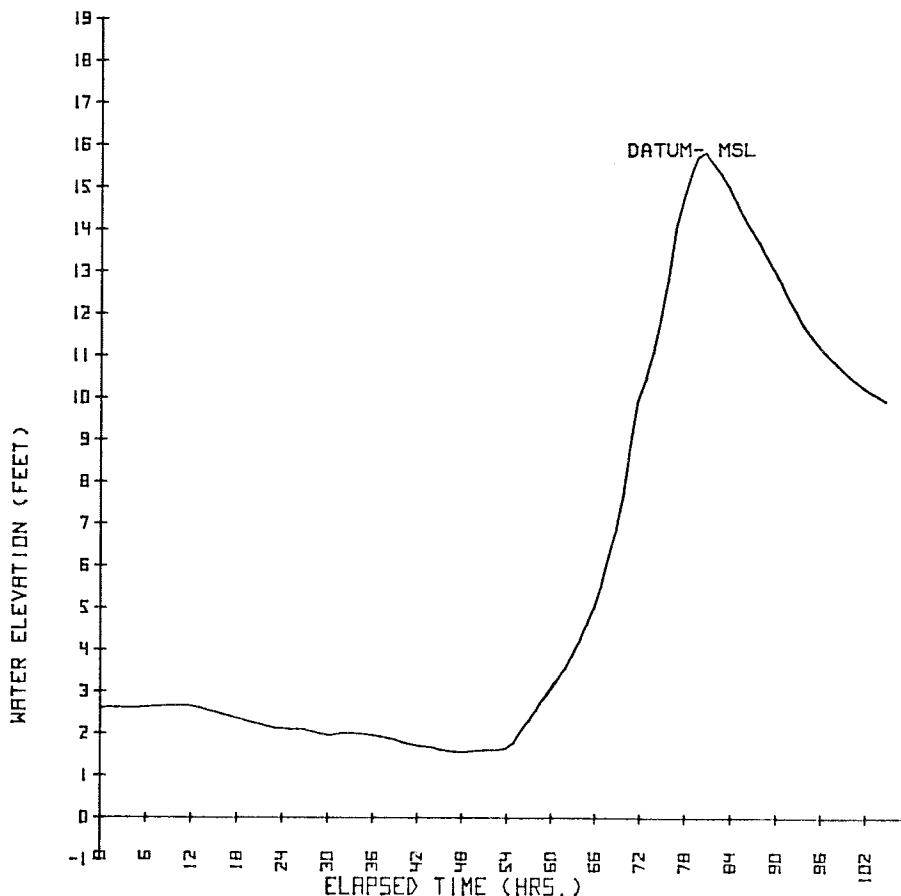


Figure 53. Hydrograph for SPH, LR-ST at Orange Naval Station, Sabine River (FK = 0.0010).

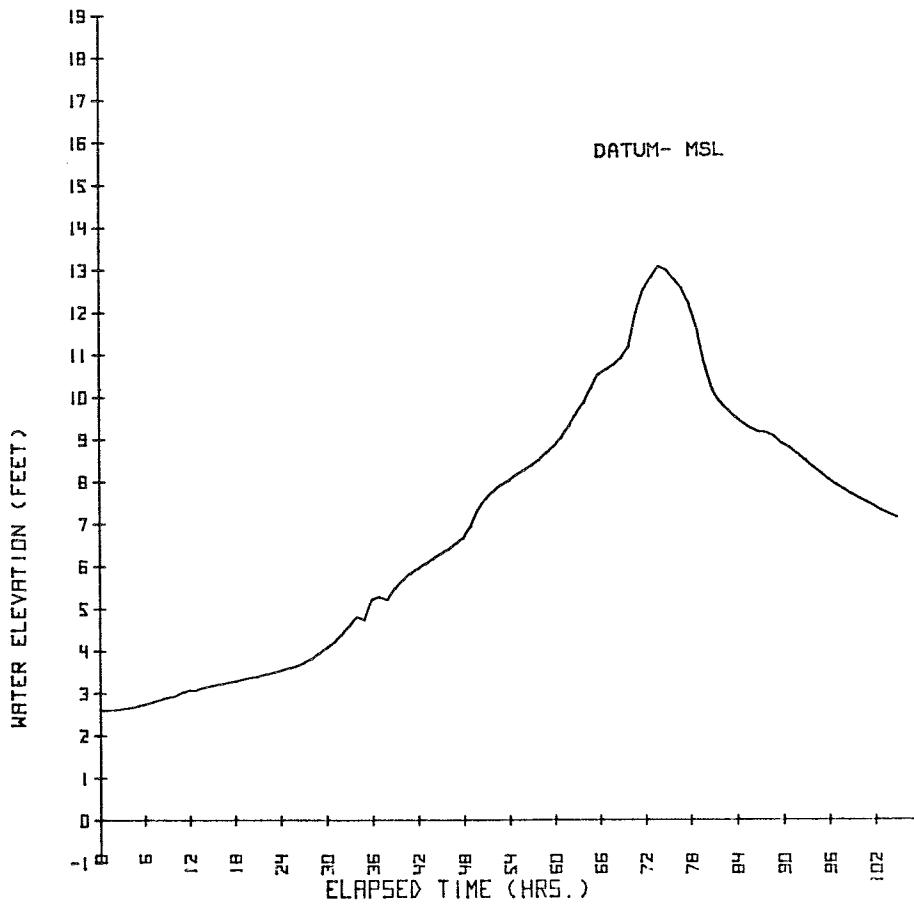


Figure 54. Hydrograph for SPH, LR-ST at west end of Intra-coastal Waterway ( $FK = 0.0010$ ).

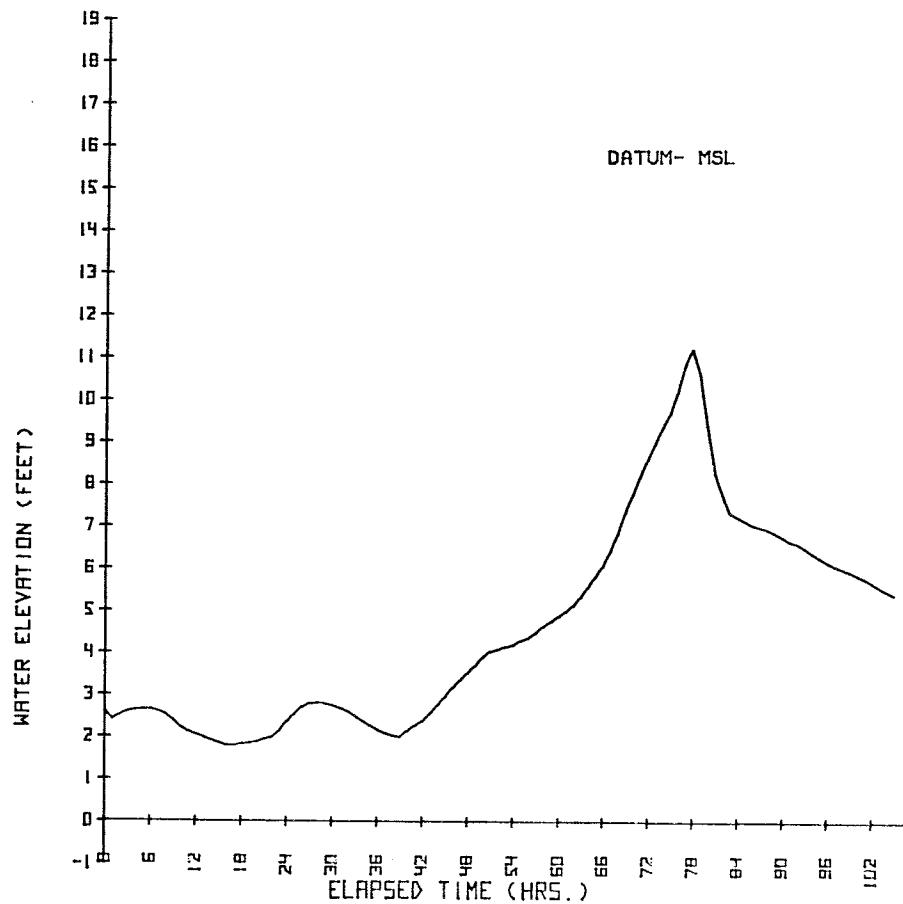


Figure 55. Hydrograph for SPH, LR-ST at Cameron, Calcasieu Pass ( $FK = 0.0010$ ).

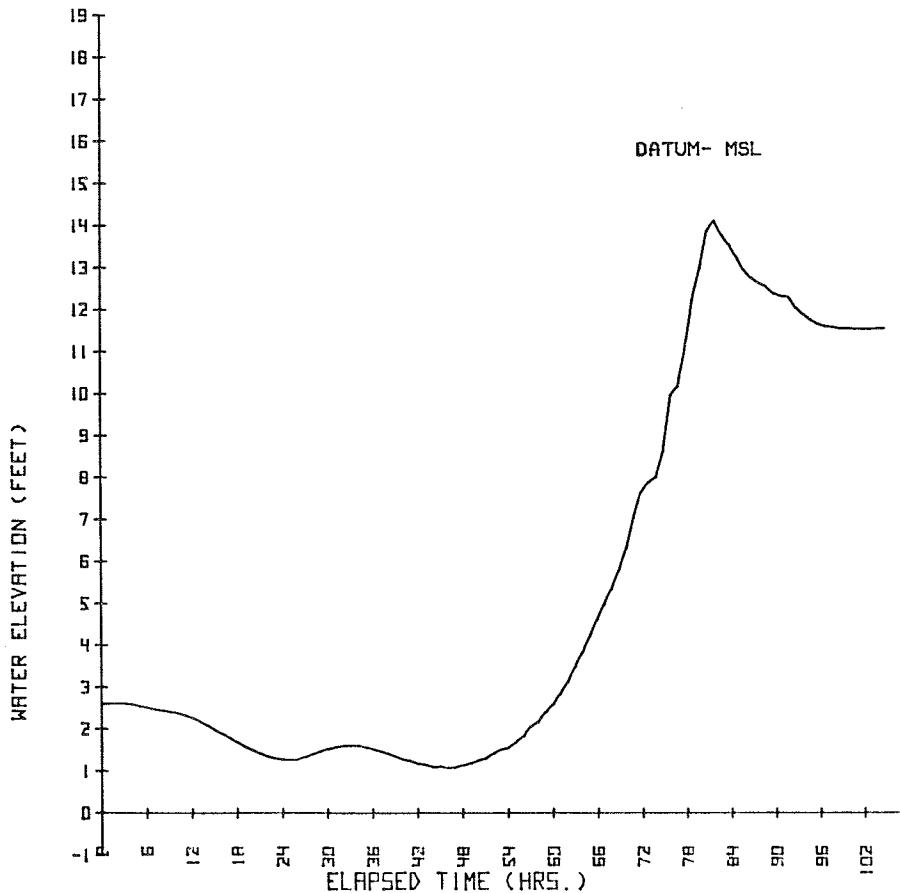


Figure 56. Hydrograph for SPH, LR-ST at Lake Charles,  
Calcasieu River ( $FK = 0.0010$ ).

### 3. LR-MT Storm Data.

The large radius, medium translation (LR-MT) storm has identical characteristics to the LR-ST storm with the exception of a higher translation speed of 11 knots. Wind vector plots from  $t = 15$  hours to  $t = 40$  hours are shown at 5-hour increments in Figures 57 to 62. The storm track is identical to that of the LR-ST storm. The gulf hydrographic input was derived by one-dimensional, bathystrophic analysis and provided by the Galveston District. Runs were made both with and without rainfall. Again, the results given graphically below are the tentative results based on  $FK = 0.0010$ .

### 4. LR-MT Storm Results.

The more rapid movement of the storm center across the Sabine-Calcasieu system yielded generally smaller water level excursions inside the bay system in comparison with the LR-ST storm. Hydrographs at the established prototype locations are shown in Figures 63 to 71 for the computer run with rainfall (16 inches) and without rainfall. Note that direct comparison between the LR-ST results and LR-MT results should be made on the basis of Figures 48 to 56 and 63 to 71, respectively. All of the SPH runs use an initial water level of about 2.5 feet in the bay system.

A summary of the peak values and relative times of water level at seven locations for the three different SPH runs is given in Table 6. Although the absolute values of the water levels depend on the value of  $FK$  (as discussed in previous sections), all results in Table 6 are based on the same  $FK$  and hence the difference between values is not too sensitive to  $FK$ .

Table 6. Comparison of peak surge and time of peak surge, showing effects of translational speed of storm and rainfall ( $FK = 0.0010$  for all three cases).

Location	Slow speed		Medium speed			
	With rainfall		With rainfall		Without rainfall	
	(ft above MSL)	(time <sup>1</sup> )	(ft above MSL)	(time)	(ft above MSL)	(time)
Sabine Pass entrance	13.0	0	14.9	0	14.9	0
Port Arthur	14.3	2	13.2	2	12.5	2
North Sabine Lake	15.3	1	15.3	1	14.7	1
Beaumont	18.7	4	15.1	5	11.5	6
Orange Naval Station	15.9	4	14.5	5	11.7	6
Cameron	11.3	1	11.0	1	10.8	1
Lake Charles	14.1	6	14.2	6	13.2	6

<sup>1</sup>Nearest hour after that of Sabine Pass entrance.

Comparison of the first and second sets of peak levels in Table 6 indicates a reduced response at nearly all stations within the Sabine-Calcasieu system with an increase in the translational speed of the storm, in spite of the increased surge at the shoreline (Sabine Pass entrance). A reduction in volume response within the system is expected

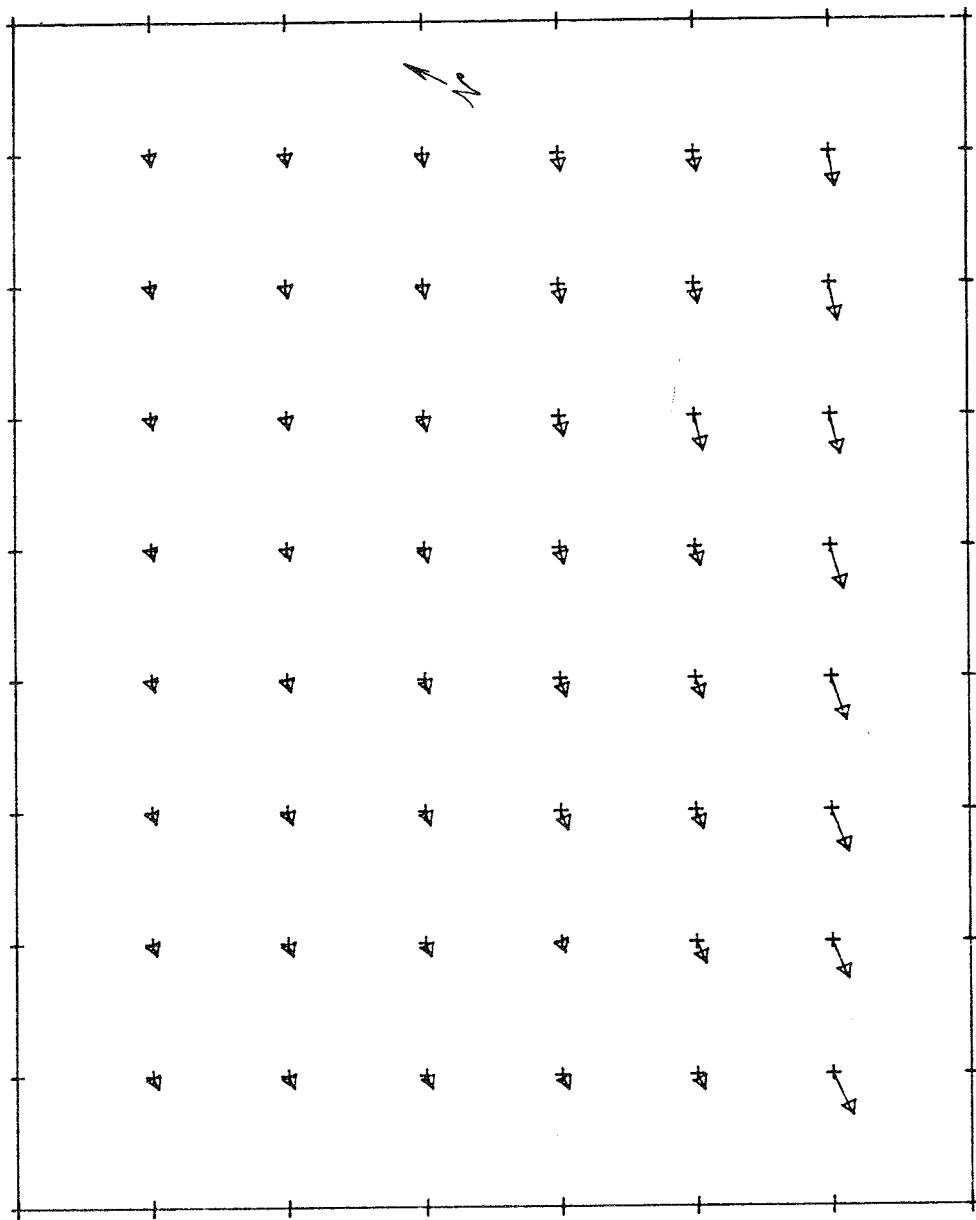


Figure 57. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 15 hours.

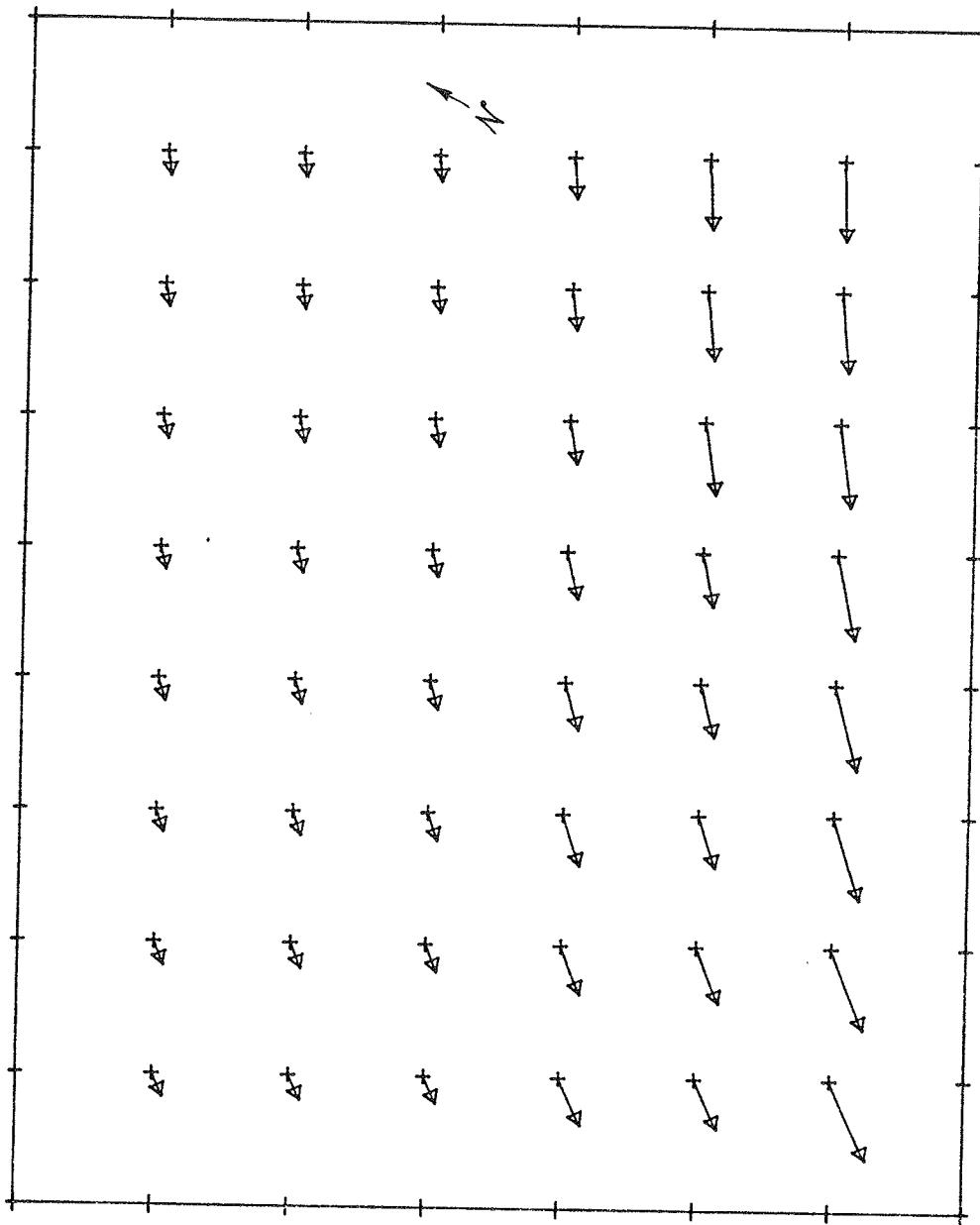


Figure 58. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 20 hours.

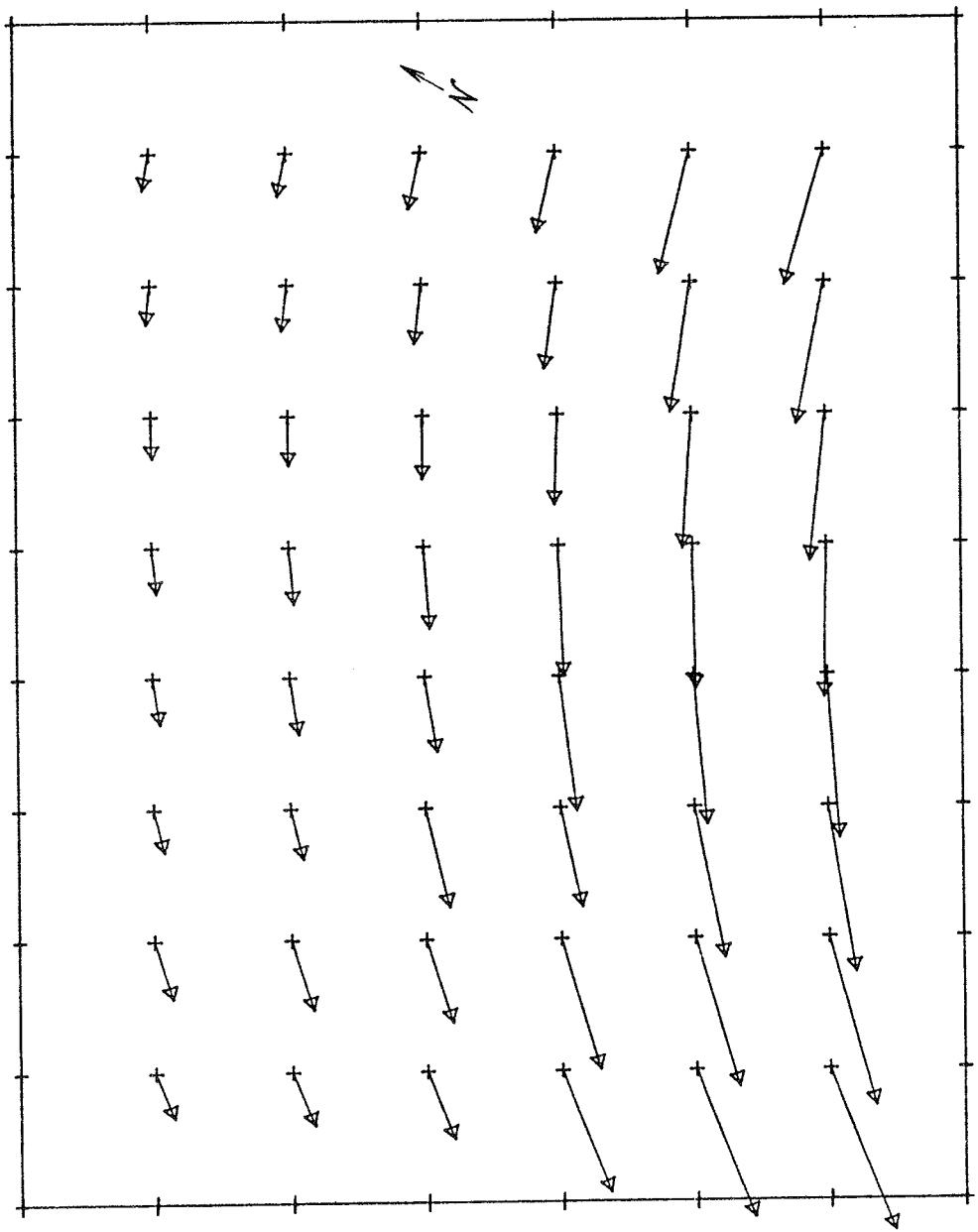


Figure 59. Wind-stress vectors for SPII large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 25 hours.

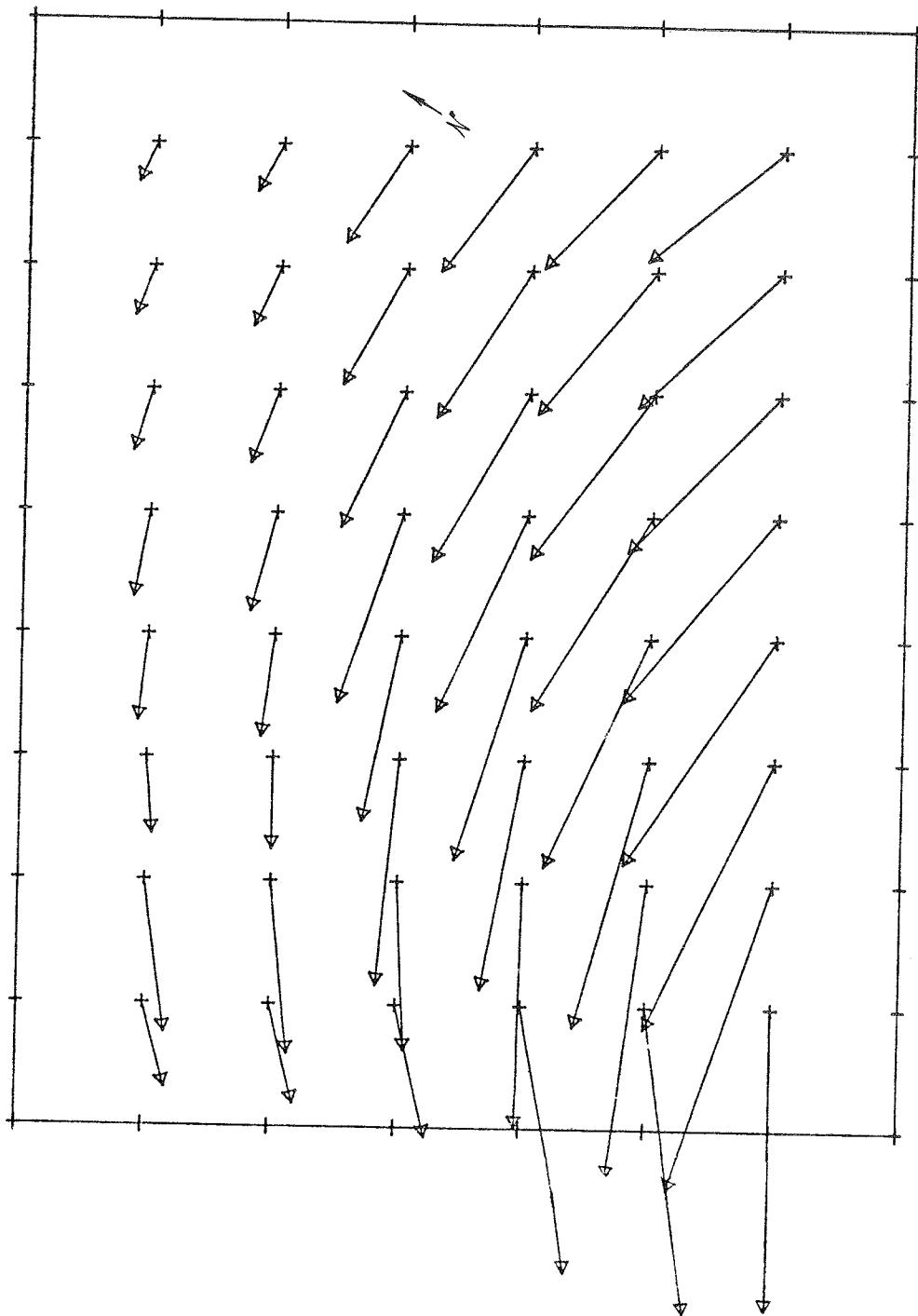


Figure 60. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 30 hours.

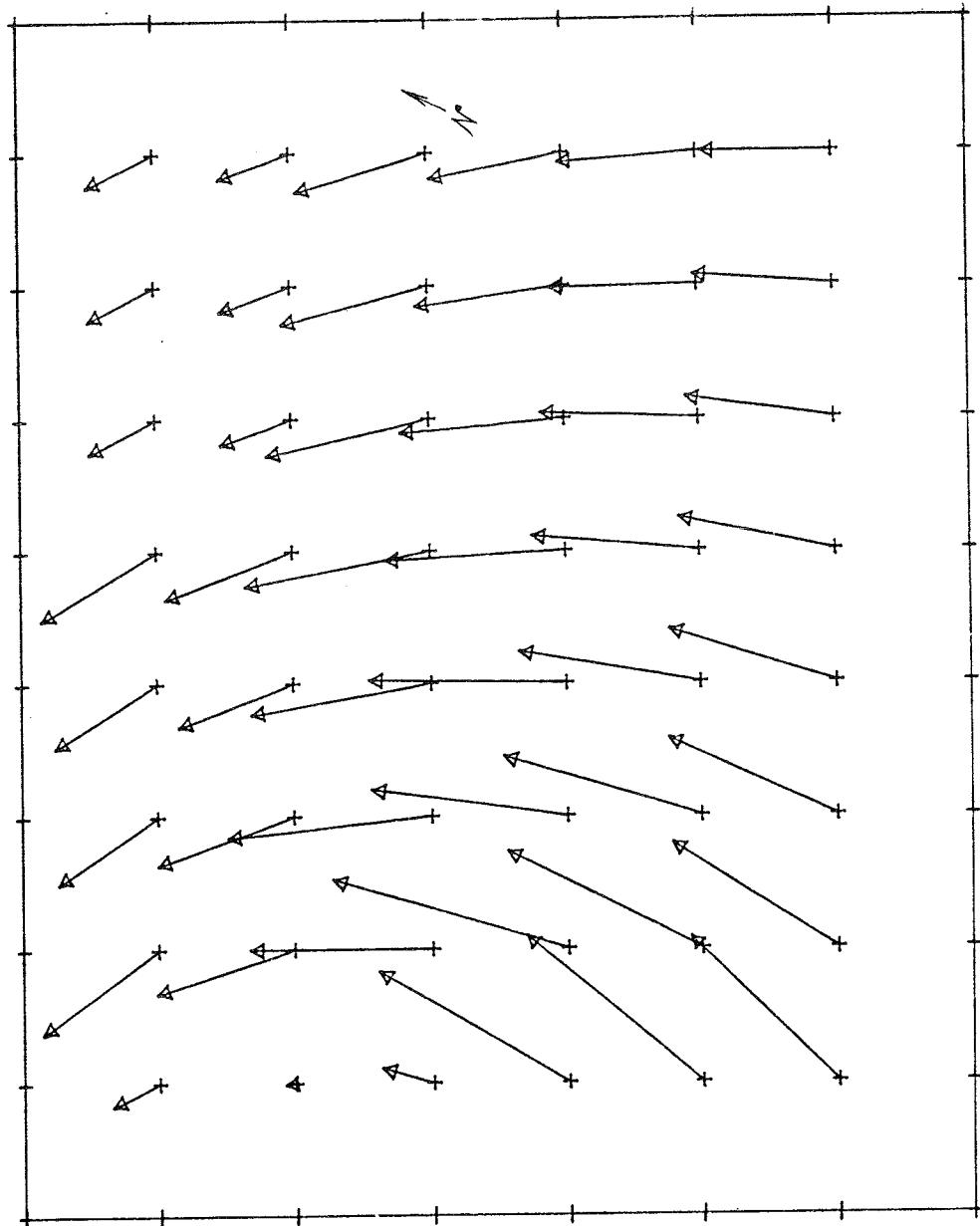


Figure 61. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 35 hours.

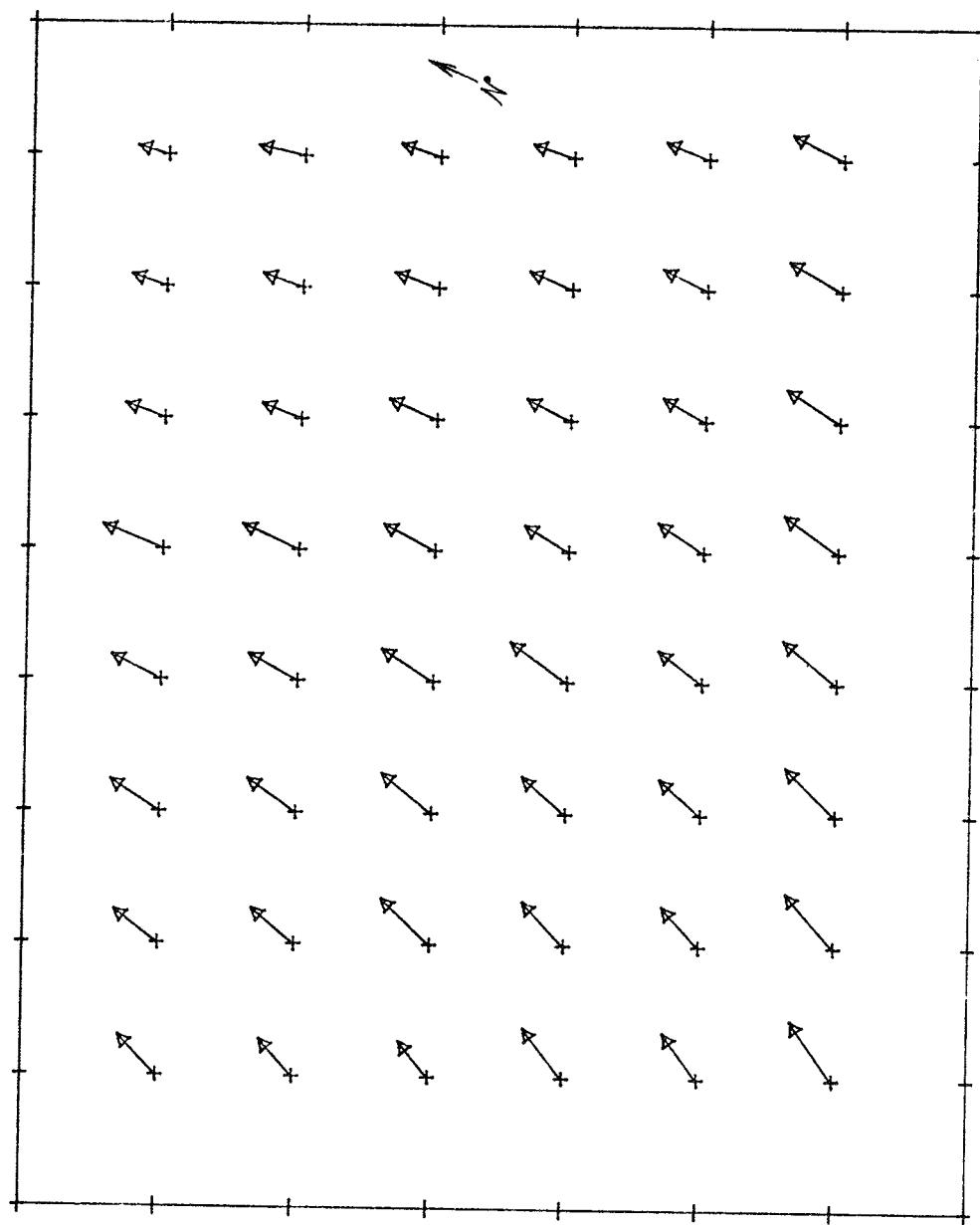


Figure 62. Wind-stress vectors for SPH large radius, medium translation (LR-MT) on an 8-nautical mile grid; time = 40 hours.

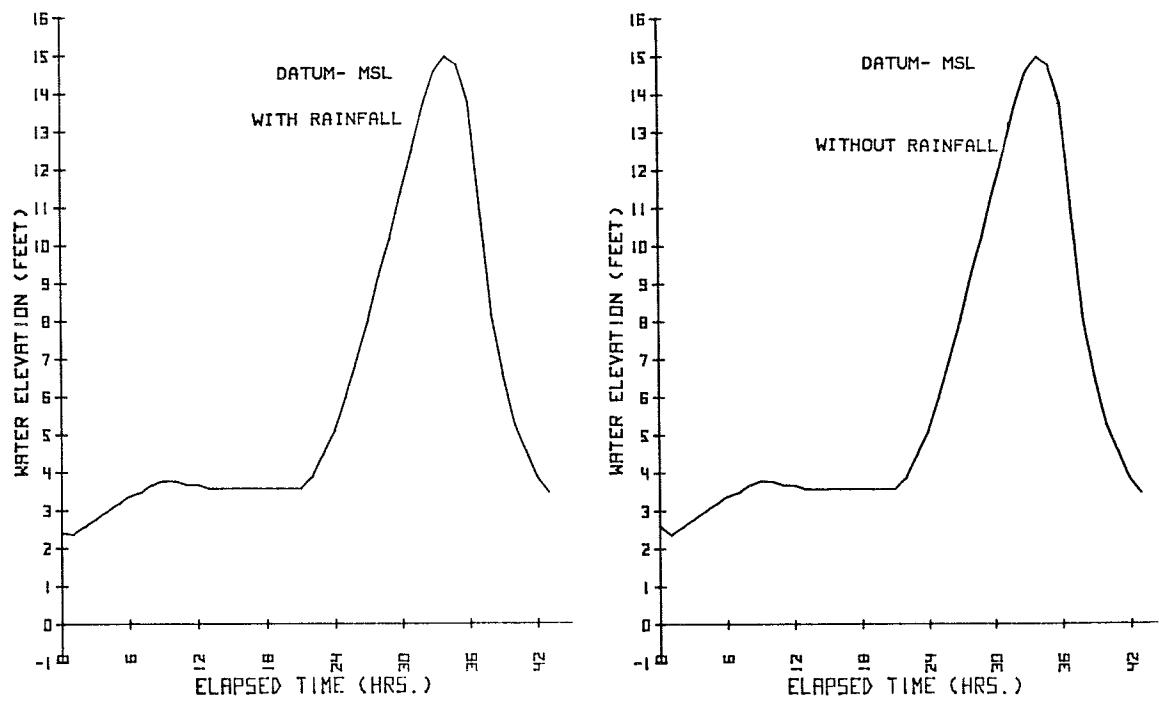


Figure 63. Hydrographs for SPH, LR-MT (with and without rainfall) at Sabine Pass, southwest jetty.

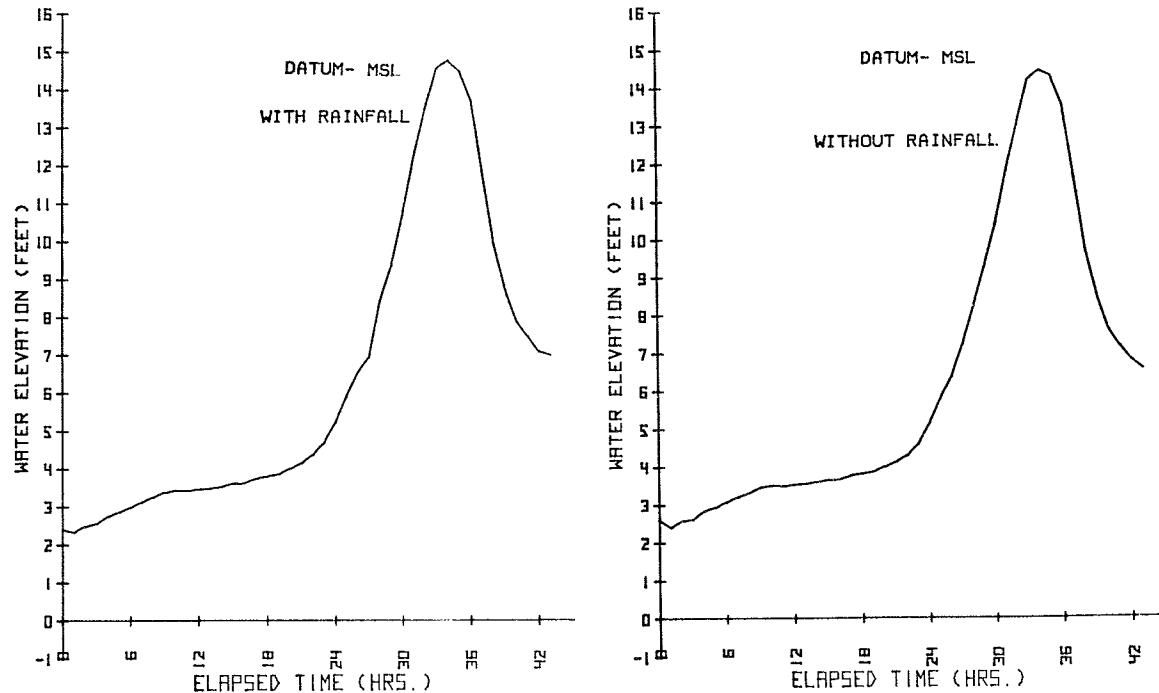


Figure 64. Hydrographs for SPH, LR-MT (with and without rainfall) at Sabine Pass, U.S. Coast Guard Station ( $FK = 0.0010$ ).

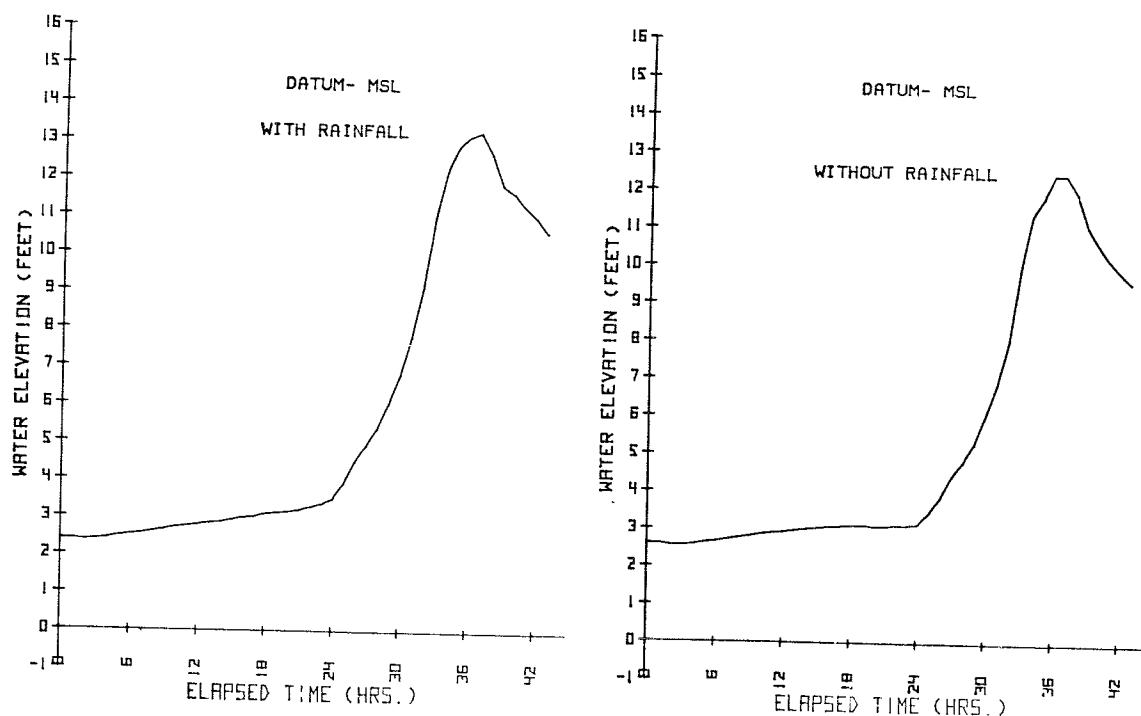


Figure 65. Hydrographs for SPH, LR-MT (with and without rainfall) at Port Arthur ( $FK = 0.0010$ ).

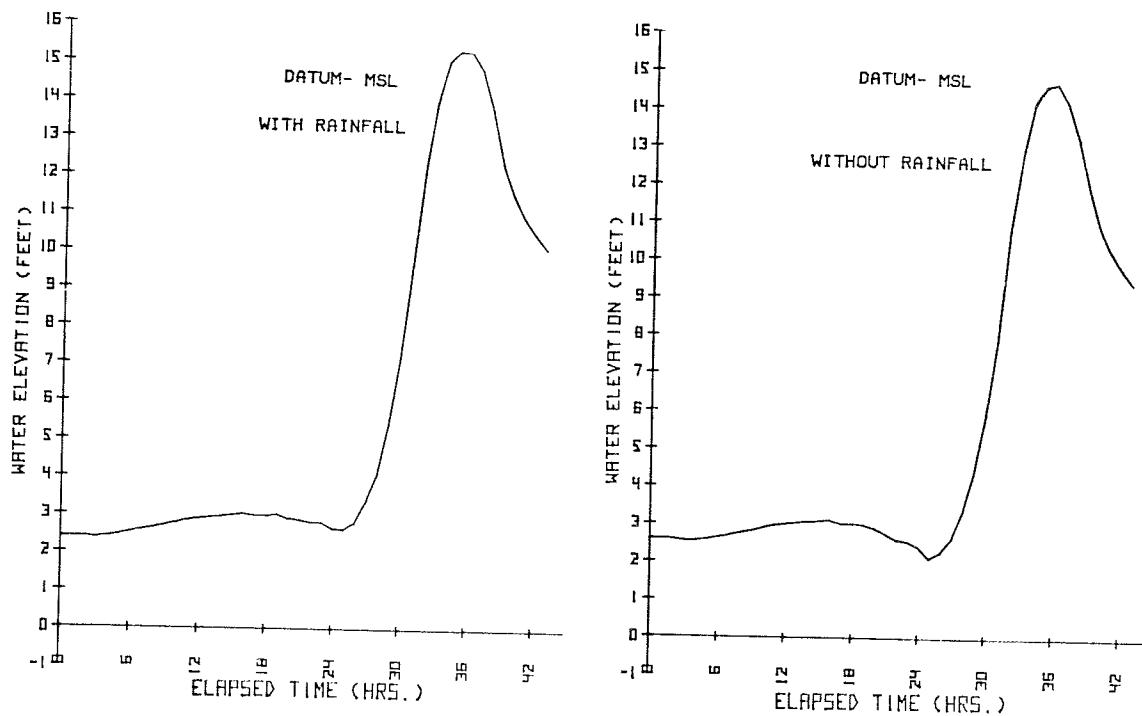


Figure 66. Hydrographs for SPH, LR-MT (with and without rainfall) at north Sabine Lake ( $FK = 0.0010$ ).

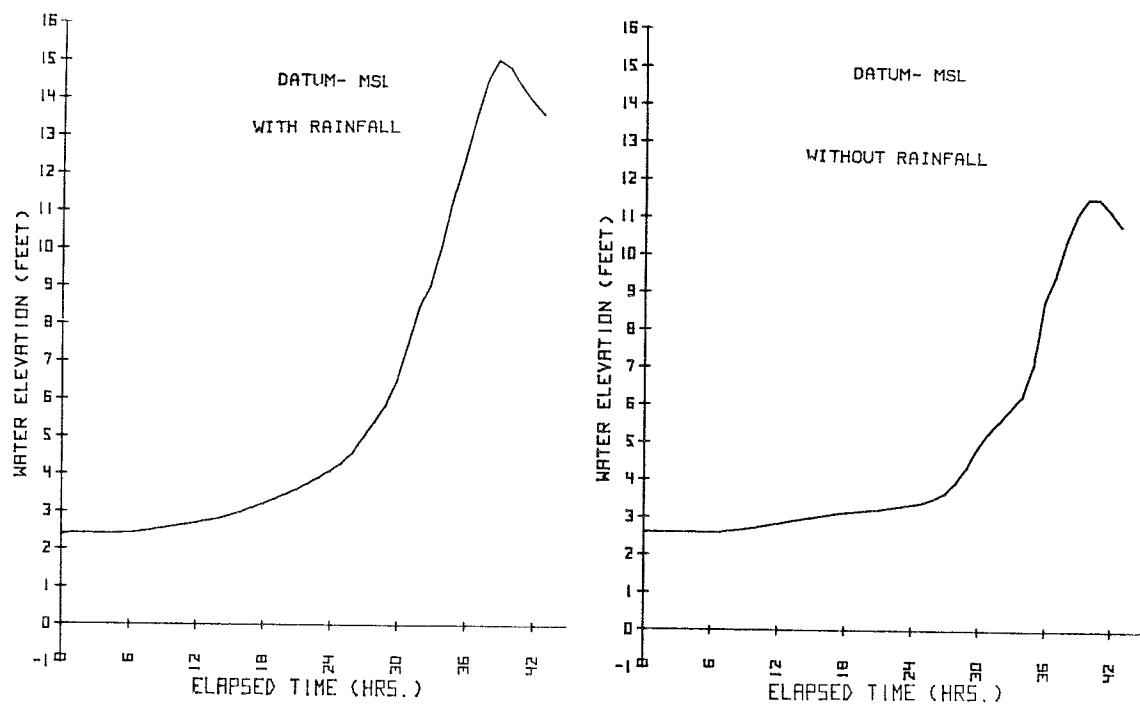


Figure 67. Hydrographs for SPH, LR-MT (with and without rainfall) at Beaumont, Neches River, and Brakes Bayou ( $FK = 0.0010$ ).

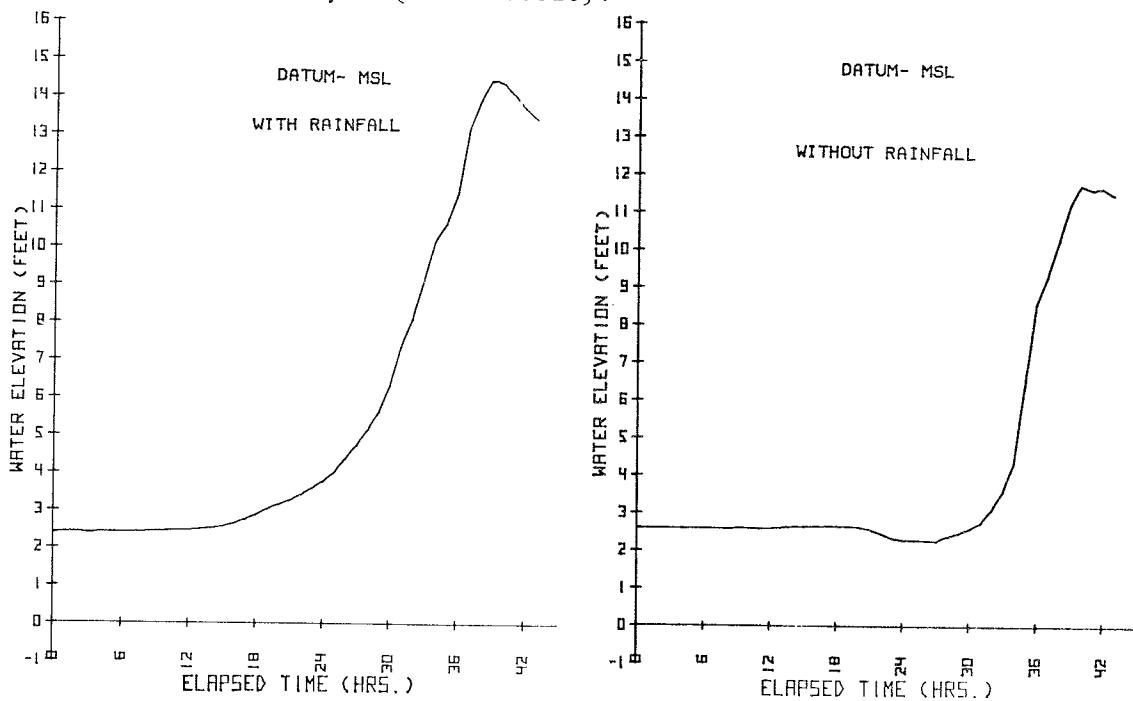


Figure 68. Hydrographs for SPH, LR-MT (with and without rainfall) at Orange Naval Station, Sabine River ( $FK = 0.0010$ ).

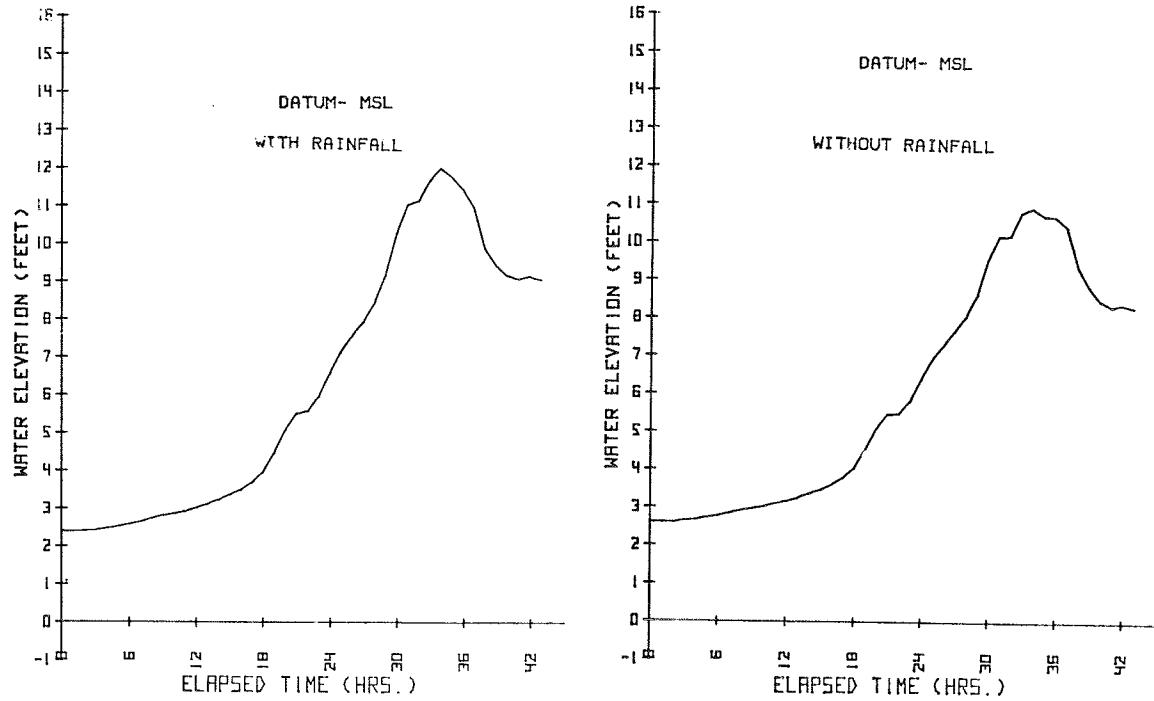


Figure 69. Hydrographs for SPH, LR-MT (with and without rainfall) at west end of Intracoastal Waterway ( $FK = 0.0010$ ).

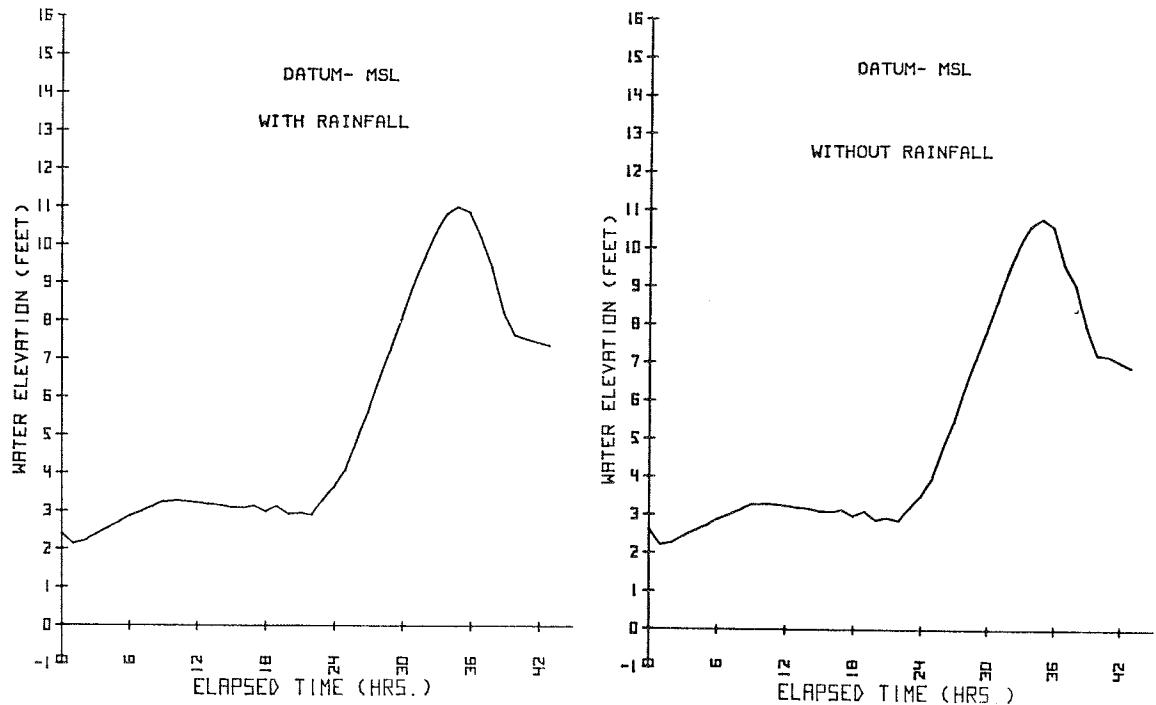


Figure 70. Hydrographs for SPH, LR-MT (with and without rainfall) at Cameron, Calcasieu Pass ( $FK = 0.0010$ ).

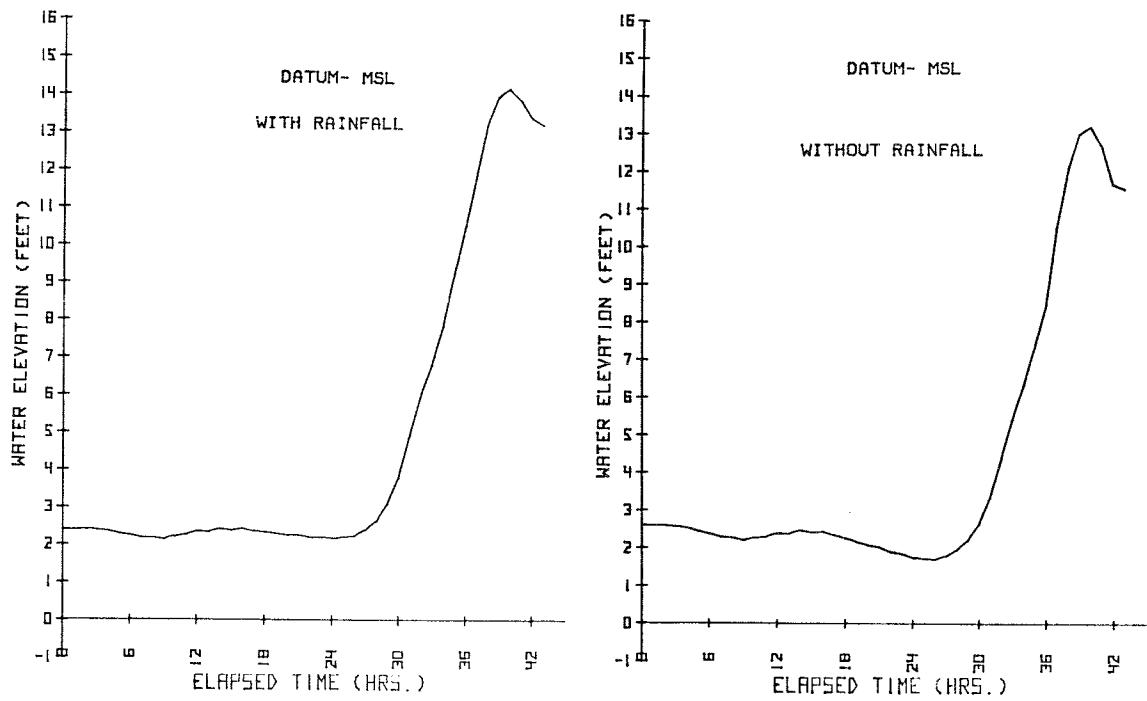


Figure 71. Hydrographs for SPH, LR-MT (with and without rainfall) at Lake Charles, Calcasieu River (FK = 0.0010).

for the greater speed (shorter duration) storm because of the constricted connection to the sea. Port Arthur shows a reduction of 1.1 feet for the MT storm relative to the ST storm; north Sabine Lake appears to show no change. An examination of the wind fields close to the time of the peak surges (Figs. 46 and 61) indicates that a greater wind-induced setup within the lake occurs between Port Arthur and the north Sabine Lake station for the medium speed storm, due to the favorable orientation of the winds near the time of peak surge at the lake entrance.

The response at Beaumont and Orange, both of which are well inland of the main lake area, shows a significant reduction (3.6 and 1.4 feet, respectively) as well as a greater timelag for the faster storm. Moreover, the peak elevations for both of these stations are somewhat less than that at north Sabine Lake for the MT storm in contrast to the situation for the ST storm. The limited access of water to these regions is apparently responsible for this sensitivity to storm duration.

The influence of rainfall and associated runoff from drainage areas well inland is shown very dramatically from a comparison of the second and third sets of peak levels in Table 6, particularly for Beaumont and Orange Naval Station, where runoff produces a differential flooding of 3.6 and 2.8 feet, respectively. A differential of about 0.6 foot due to runoff and rainfall occurs even within Lake Sabine. The effects within Lake Calcasieu and upstream to the northeast are less pronounced due to the smaller runoff.

#### VIII. CONCLUSION

The use of a modified program for inclusion of subgrid scale channels has been demonstrated to be essential for simulation of tides in the upper reaches of a system like the Sabine-Calcasieu region, where the primary connection to locations such as Beaumont, Orange, and Lake Charles is via river channels which would not otherwise be resolved by a grid scheme of the order of a 1-nautical mile scale. Even for conditions of extreme flooding, as occur during hurricanes, the incorporation of the subgrid scale channels provides a degree of freedom for return flow in the presence of water level gradient, which would otherwise not exist in models which exclude subgrid scale channels. The simulation of Hurricane Carla in particular is improved over that attainable with the SURGE I program which did not allow for the subgrid scale channel subroutine.

While programs such as SURGE I can, in principle, simulate the effects of channels, provided the grid scale is of the order of the channel width, the required computer time is usually prohibitive at least for explicit numerical models. Some advantage can be gained in respect to economy by the use of implicit numerical models such as that of Leendertse (1967); however, the accuracy of such schemes when used on a competitive basis, from the standpoint of economy (large time steps) can suffer relative to that which can be achieved with the subgrid scale channel routine. However, the best procedure for such numerical simulation remains to be determined.

#### LITERATURE CITED

- BODINE, B.R., "Storm Surge on the Open Coast: Fundamentals and Simplified Prediction," TM-35, U.S. Army, Corps of Engineers, Coastal Engineering Research Center, Washington, D.C., May 1971.
- DRONKERS, J.J., *Tidal Computations in Rivers and Coastal Waters*, North Holland, Amsterdam, and Wiley and Sons, New York, 1964.
- GRAHAM, H.E., and NUNN, D.E., "Meteorological Considerations Pertinent to Standard Project Hurricane, Atlantic and Gulf Coasts of the United States," Report 33, National Hurricane Research Project, U.S. Department of Commerce, Washington, D.C., 1959.
- JELESNIAKSKI, C.P., "A Numerical Calculation of Storm Tides Induced by A Tropical Storm Impinging on A Continental Shelf," *Monthly Weather Review*, Vol. 93, 1965, pp. 343-358.
- JELESNIAKSKI, C.P., "Numerical Computations of Storm Surges Without Bottom Stress," *Monthly Weather Review*, Vol. 94, 1966, pp. 379-394.
- JELESNIAKSKI, C.P., "Numerical Computations of Storm Surges with Bottom Stress," *Monthly Weather Review*, Vol. 95, 1967, pp. 740-756.
- LEENDERSTE, J.J., "Aspects of a Computational Model for Long-Period Water Wave Propagation," The Rand Corporation, Santa Monica, Calif., 1967.
- LOVE, R.W., "Tidal Response of a Bay with a Constricted Opening to the Sea," Thesis, Texas A&M University, College Station, Tex., 1959.
- MARINOS, C., and WOODWARD, J.W., "Estimation of Hurricane Surge Hydrographs," *Journal of the Waterways and Harbors Division*, Vol. XCIV, 1968, pp. 189-216.
- MASCH, F.D., et al., "A Numerical Model for the Simulation of Tidal Hydrodynamics in Shallow Irregular Estuaries," University of Texas, Austin, Tex., 1969.
- MIYAZAKI, M., "A Numerical Computation of the Storm Surge of Hurricane Carla 1961 in the Gulf of Mexico," Technical Report 10, Department of Geophysical Sciences, University of Chicago, Chicago, Ill., 1963.
- PLATZMAN, G.W., "A Numerical Computation of the Surge of 26 June 1954 on Lake Michigan," *Geophysica*, Vol. 6, 1958, pp. 407-438.
- REID, R.O., and BODINE, B.R., "Numerical Model for Storm Surges in Galveston Bay," *Journal of Waterways and Harbors Division*, No. WWI, 1968, pp. 33-57.
- STOKER, J.J., *Water Waves, the Mathematical Theory with Applications*, Interscience, New York, 1957.
- VAN DORN, W., "Wind Stress on an Artificial Pond," *Journal of Marine Research*, Vol. 2, 1953, pp. 249-276.



## APPENDIX A

### SURGE II PROGRAM

This appendix includes a complete listing of the SURGE II program. Except for SUBROUTINE CHANL, the program is much the same as that used in Reid and Bodine (1968). It should be emphasized that the coding of calculations of flow and water level for blocks does not include the effect of Coriolis force. Moreover, no attempt has been made to optimize the coding since the original version. The actual new part of the program is embodied in SUBROUTINE CHANL and the way in which the channel computations mesh with the block calculations. Thus, while many users may prefer their own version for calculations over the main grid, it should be possible to incorporate SUBROUTINE CHANL with their own program when applied to systems like the Sabine-Calcasieu region in which allowance for channels is essential.

PROGRAM SURGE

74/74 OPT#2

FTN 4.6+420

08/22/77 16.51.10

```

1      PROGRAM SURGE(INPUT,OUTPUT,TAPES=INPUT,TAPE&=OUTPUT)
C
C
C
5      ****
C
C      SURGE II PROGRAM
C      FOR SIMULATION OF TIDES AND WIND INDUCED SURGES IN BAYS WITH
C      ALLOWANCE FOR SUBGRID-SCALE CHANNELS AND HARRIERS
10     DEVELOPED FOR THE U. S. ARMY CORPS OF ENGINEERS, GALVESTON DISTRICT
C      BY R. D. REID, A. C. VASTAHO AND T. J. REID OF
C      COASTAL STUDIES, INC., P.O. BOX 9064, COLLEGE STATION, TX 77843
C      DECEMBER, 1975
C
15     ****
C
C
C
20     COMMON/PLK1/ I8(100),J8(100),IZX(100),IZY(100),ICDX(100)      MAIN0003
1,ICDY(100),ICDSX(100),ICDSY(100),LRCI(8),LRQJ(8),DIST(24)      MAIN0004
2,CHST(30),RD(R,30),HG(R,8),XR(8,6),YR(8,6),RR(8)                MAIN0005
C
25     COMMON/PLK2/ IZ(28,20),U(28,20),V(28,20),H(28,20),NTIME          MAIN0006
C
C      COMMON/PLK3/ NM,MMIN,MMAX,NFU,INFLD,IM,JW,KM,KMAX,LMAX,DELX,DELT  MAIN0007
1,CDO,FK,HG,IOUT,KI,LJ,KI,LJJ,JBL,JBR,XW,YW,LHM,RF,COSTS
2,IMRO,JMRD,KP,TRTR,IND,NOX,KIM,NORT,NTIME,INTIME,NO,I,D,GRAV   MAIN0008
3,KCMP,DFU,INTER
C
30     COMMON/PLK4/ HRD(8),CRO(8),KD(24),X1(28,21),Y1(28,21),X2(25,21)  MAIN0011
1,Y2(28,21),X(28),Y(28),HG1(28),HG2(28),H1(8),H2(9),VN(28)      MAIN0012
2,HG(28),HR2(R)
C
35     COMMON/PLK5/ JCG(130),JCX(130),JCY(130),IZCX(130)            MAIN0014
1,IZCY(130),QCXP(130),QCXN(130),QCYP(130),QCYR(130),HC(130),HP(130) MAIN0015
2,KCM,KCX(130),KCY(130),KCR(130),UCT(130),JCF(130),KRI(130),IBK
3,KEY(2,130),VCT(130),VCF(130),AGX(130),AGY(130),KCXP(130)      MAIN0016
4,KCYP(130),KL9(50),KLM,IFC(130),FC
C
40     COMMON/PLK6/ HGRM(8,25), HRR(8,25), XRM(8,6,25), YRM(8,6,25)  MAIN0019
C
45     COMMON/PLK7/ TEND,NF,IBL,NJ,ALPHA(40)                            MAIN0020
C
45     COMMON/PLK8/ HS(9,72), GS(6,72), TIME(72)                      MAIN0021
C
50     COMMON/PLK9/ KZ,LZ,NUMRO,C1,C2,C3,IMM,JMN,NT,AN,NEXT1,IT,MC,IFIRSTMAIN0022
1,JAIND,NEK1,XNOX,NE-3,XNRT, C4,KAIN,AJ,AII,LJK,KIK                 MAIN0023
C
50     COMMON/PLK10/ AGAGE,YFLOW,IGAGE(12),JGAGE(12),KFLOK(6),XMIN,XMAX  MAIN0024
C
C      CODE WORD (CARD 1) ON CARD ONE OF INPUT, CONTROLS INITIAL
C      COMPUTATIONS AND READ ACTIONS AS FOLLOWS
C

```

PROGRAM SURGE

74/74 OPT#2

FTN 4.6+420

08/22/77 10.51.06

```

      C      ICARD#0  THIS IS FOR STARTING A PROBLEM
55     C      CARD INPUT IS ICARD FOLLOWED BY THE FULL GALV
      C      DATA DECK LESS THE BLANK CARD AT THE END
      C      PLUS THE GALV LIST DECK
      C      PLUS THE CHANNEL DATA DECK (IF KCM.GT.ZERO)

      C      ICARD#1  THIS IS FOR CONTINUING A PROBLEM
60     C      CARD INPUT IS ICARD FOLLOWED BY THE CONTIN DECK
      C      THEN FOLLOWED BY THE FIRST FOUR DATA CARDS OF THE
      C      GALV DATA DECK. (THE CONTIN DECK IS OUTPUTTED FROM
      C      A PREVIOUS RUN)

      C      CODE WORD (IBL) IS THE STARTING COLUMN IN THE LISTING OF H
65     C      CODE WORD (KCM) IS THE NUMBER OF BLOCKS WITH CHANNELS
      C      CODE WORD (NOWIND) IS NEGATIVE FOR NO WIND FIELD (OMITS CARD INPUT)
      C      OF X,Y FIELDS AND USES FORMAT #10 FOR MG INPUT)
      C      INTER IS THE INTERVAL USED IN THE SAVE H OPERATION
70     C      NGAGE IS THE NUMBER OF HYDROGRAPHS TO BE SAVED
      C      NFLOW IS THE NUMBER OF FLOW GRAPHS TO BE SAVED
      C      IMIN AND IMAX ARE THE DESIRED LOWER AND UPPER LIMITS OF H FOR

      C      GRAPHICAL OUTPUT
      C      PRINT 10
      C      READ 1, ICARD
      C      10 FORMAT (1H1)
      C      READ 1, ICARD
      C      1 FORMAT(1,I3+10I4)
      C      1 IF(ICARD.EQ.0) GO TO 2
      C      READS PREVIOUS RESULTS FOR CONTINUATION OF PROBLEM
      C      CALL CONTIN(1)
      C      INTIME=NTIME
      C      2 CONTINUE
      C      READ 1, INTENT,IRL,KCM,NOWIND,INTER,NGAGE,NFLOW,IMIN,IMAX
      C      XMAX = IMAX
      C      XMIN = IMIN
      C
      C      50 FORMAT(3X,(ICARD=1,I2,( IBL=1,I3,( KCM=1,I4,( NOWIND=1,I3
      C      1,( INTER=1,I3,( NGAGE=1,I3,( NFLW=1,I3,( IMIN=1,I3,( IMAX=1,I3,/) MAIN0036
      C      51 FORMAT(3X,(INT=1,I2,( NTIME=1,I3,( NM=1,I4,( MM=1,I4
      C      1,( MMAX=1,I4,( NFU=1,I4,( INUT=1,I4,( INFU=1,I4) MAIN0037
      C      52 FORMAT(3X,(INT=1,I2,( IM=1,I3,( JH=1,I3,( KM=1,I3,( KMAX=1,I3
      C      1,( LMAX=1,I3 ) MAIN0038
      C      53 FORMAT(3X,(INT=1,I2,( DELX=1,F4.1,( N=41, DELT=1,F4.0
      C      1,( SEC=1,F6.3,( FK=1,F7.4,( FC=1,F7.4,( HG=1,F6.3,( FT() MAIN0039
      C      54 FORMAT(3X,(INT=1,I2,( KI=1,I3,( LJ=1,I3,( KI=1
      C      1,I3,( LJ=1,I3,( JH=1,I3,( JK=1,I3,/) MAIN0040
      C      55 FORMAT(3X,(K=1,I3,( IB=1,I3,( JR=1,I3,( ZX=1,I5
      C      1,( COOZ=1,I5,( COOY=1,I5,( COSX=1,I5,( COSY=1,I5) MAIN0041
      C      57 FORMAT(1,/3X,(INT=1,I2,( IMRG=1,I3,( JMRG=1,I3,( XE=1,I3
      C      1,( TSTR=1,I3,( IN=1,I3,( NO=1,I3,( KIN=1,T5,( NORT=1,I3) MAIN0042
      C      58 FORMAT(3X,(INT=1,I2,( RF=1,F6.3,( CONST=1,F7.4,( S=1,F8.5) MAIN0043
      C      75 FORMAT(3X,(INT=5,I3,(BARRIER DATA= Z VALUES IN TENTHS OF FEET, MAIN0051
      C      1 CC VALUES ARE TIMES 1000()
      C      76 FORMAT(1,/3X,(INT=6,( *3X,(BLOCK TOPOGRAPHY= Z VALUES IN FEET()MAIN0052
      C

```

SUBROUTINE PART2

74/74 OPT#2

FTN 4.6+420

08/22/77 16.51.06

```

1      C
2      C
3      C      SUBROUTINE PART 2          PT2=0001
4      C
5      COMMON/PLK1/  IR(100),JB(100),IZX(100),IZY(100),ICDX(100),
6      ICDDY(100),ICDSX(100),ICDSY(100),LRCI(5),LRCJ(8),DIST(24)    PT2=0004
7      CHST(30),RNF(8,30),HGR(8),XR(8,8),YR(8,8),DR(8)                PT2=0005
8      COMMON/PLK2/  TZ(28,20),U(28,20),V(28,20),H(28,20),NTIME          PT2=0007
9      COMMON/PLK3/  NM,MMIN,MMAX,NFU,INFLO,IM,JY,KY,KMAX,LMAX,DELX,DELT   PT2=0008
10     COO,FK,HGI,IOIT,KILJ,KIJ,LJJ,JHL,JHP,XYM,LUM,RF,CONST,S           PT2=0009
11     IMRD,JMRD,KR,ISTR,IND,NOM,KIM,NORT,NTIME,INTIME,NCAID,GRAV        PT2=0010
12     XCMP,DFU,INTER                                         PT2=0011
13     COMMON/PLK4/  HRO(8),CRO(6),KR(24),X1(28,21),Y1(28,21),X2(28,21)  PT2=0012
14     Y2(28,21),X(281,Y(28),HGI(28),HG2(28),M31(8),HR(9),VN(28)       PT2=0013
15     HG(28),HAP(8)                                         PT2=0014
16     COMMON/PLK5/  ICG(130),JCG(130),I,CX(130),I,CY(130),IZCX(130)      PT2=0015
17     IZCY(130),DCXP(130),DCXV(130),DCYP(130),DCVN(130),HC(130),HP(130) PT2=0015
18     KCN,CCX(130),CCY(130),KC(130),UCT(130),UCF(130),KPI(130),IM0K    PT2=0017
19     XEN(2,130),VCT(130),VCF(130),AGY(130),AGV(130),CX(130)           PT2=0018
20     KCYP(130),KLR(50),CLM,IFC(130),FC                                         PT2=0019
21     COMMON/PLK6/  HGP(8,25),HSP(8,25),XRM(8,6,25),YRM(8,6,25)          PT2=0020
22     COMMON/PLK7/  TEND,NF,IBL,NJ,ALPHA(40)                                PT2=0021
23     COMMON/PLK8/  HS(9,72),RS(6,72),TIME(72)                            PT2=0022
24     COMMON/PLK9/  KZ,LZ,NMRO,C1,C2,C3,IMM,JMM,NT,NN,EXT1,IT,KC,IFIRSTPT2=0023
25     JWIN,ME-1,XNO,NE-3,XNORT, CURAIN,AJ,AI,LJK,KIK                         PT2=0024
26     COMMON/PLK10/ NGAGE,NFLOR,IGAGE(12),JGAGE(12),XMIN,XMAX               PT2=0025
27
28     ABSF(X)=ARS(X)          PT2=0026
29     SQRTF(X)=SQRT(X)         PT2=0027
30
31     NMRO=IMRD
32     KZ=K1
33     LZ=LJ
34     GRAV=32.1456355
35     C1=FKADELT
36     C2=(GRAV*DELT)/(2.*DELX)
37     C3=DELT/DELX
38     IMM=IM-1
39     JMM=JM-1
40     NT=0
41     NN=0
42     NEXT1=1
43     IT=2
44     KC=1
45     IFIRST=-1
46     J=IM0=NFLOR+1
47     DO 130 I=1,IM
48     VN(I)=0.
49     DO 130 J=1,JK
50     H(I,J)=TZ(I,J)
51     Z=IZ(I,J)
52     IF(Z,LT,HGI) H(I,J)=HGI
53     IF(Z,LT,HGI) H(I,J)=HGI
54     U(I,J)=0.

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SUBROUTINE PART2 74/74 OPT#2 FTN 4.6+420 08/22/77 16.51.06

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      130 V(I,J)=0.
      MTIME=1 PT2=0051
  55      C
      C READ CHANNEL DATA AND ESTABLISH KEY ARRAYS
      IF(KCM.GT.0) CALL CHANL(1) PT2=0052
  C READ GAGE LOCATIONS FOR SAVING KEY H AND Q VALUES AS TIME SEQUENCE
  60      CALL SAVE(1) PT2=0053
  C READ LIST DATA AND PRINT PROBLEM IDENTIFICATION AND Z FIELD
  65      READ 15, TDENT,IEND,AF,IBEGIN,NJ,NCARD PT2=0054
      15 FORMAT(I1,I4,4I5) PT2=0055
      140 FORMAT( /1H ,15A2,15A2,10A2) PT2=0056
      220 FORMAT(1H1) PT2=0057
      230 FORMAT(15A2,15A2,10A2) PT2=0058
      PRINT 220 PT2=0059
      DO 250 J=1,NCARD PT2=0060
      READ 230, (ALPHA(I),I=1,40) PT2=0061
  70      PRINT 140, (ALPHA(I),I=1,40) PT2=0062
      PRINT 220 PT2=0063
      C
      C
  75      148 CONTINUE PT2=0064
      RF=(RF/12.)*CONST PT2=0065
      NE1=NOW PT2=0066
      XNOW=XNOW PT2=0067
      NE3=NORT PT2=0068
      XNORT=XNORT PT2=0069
  80      ISTR=ISTR*NFU PT2=0070
      IND=IND*NFU PT2=0071
      AJSLZ PT2=0072
      AIEKZ PT2=0073
      JMK=JMK+1 PT2=0074
      LJK=LJK+1 PT2=0075
      KIK=KIK+1 PT2=0076
      C
  85      ENTRY PART 2B PT2=0077
      NU = (NM-TNTIME)/INTER PT2=0078
  90      CALL PLOT PT2=0079
  C PLOT CHANNELS AND BARRIERS
      CALL SAVE(2) PT2=0080
      IF(KCM.GT.0) CALL CHANL(4) PT2=0081
  C START OF TIME INCREMENTING LOOP
  95      200 CONTINUE PT2=0082
      IF(NOWIND.LT.0) GO TO 430 PT2=0083
      300 IF (NE1>NOW) 330,310,310 PT2=0084
      310 C=CHST(NEXT1+1)-CHST(NEXT1))/XNOW PT2=0085
      BIND=CHST(NEXT1) PT2=0086
      NE1=1 PT2=0087
      DO 320 NE2=1,TMRO PT2=0088
      320 CRO(NEW2)=(PO(NEW2,NEXT1+1)-RD(NEW2,NEXT1))/XNOW PT2=0089
      NEXT1=NEXT1+1 PT2=0090
      GO TO 340 PT2=0091
  100      330 NE1=NEW1 PT2=0092
      340 AN1=NEW1 PT2=0093
      PT2=0094
      PT2=0095
  
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SUBROUTINE PART2

74/74 CPT2

FTN 4.6+420

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      CHSTR=BTNP+(AN1-1.)*C
      YC=CHSTR
      DO 350 KA=1,IMRO
      NX=NEXTI
      350 HRO(KA)=RN(KA, NX=1 )+(AN1-1.)*CRO(KA)
      IHOUR=TIME/FU*NFU
      360 IF(INTIME-IHOUR)430,380,370
      370 IF(INTIME-IHOUR)380,410,430
      380 IF(NEX3-NORT)400,340,390
      390 NEX4=(DIST(KC+1)-DIST(KC))/XNORT
           KC=KC+1
           NE=3#1
           GO TO 420
      400 NE=3#NE3+1
           GO TO 420
      410 NE=4#0
           KC=KC+1
           420 CONTINUE
           AN3=NE-3
           AN4=NE-4
           R=(RF*(DIST(KC+1)+(AN3-1.)*AN4-1))/XNORT
           RAIN=0
           GO TO 440
      430 RAIN=0.0
      440 CONTINUE
      C
      C   END OF RAIN AND RO VALUES
      C
      C   START OF WIND COMPUTATIONS
      500 IF ((JWIND=NFU)800,800,510
      510 CONTINUE
      560 IF (IFIRST)600,570,570
      570 DO 580 I=1,I"
           HG1(I)=HG2(I)
           DO 580 J=1,J"
           X1(I,J)=XP(I,J)
      580 Y1(I,J)=YP(I,J)
           DO 590 IR1=2,A
           590 HB1(IR1)=HE2(IR1)
      600 MTIME=TIME+1
           IT=MTIME+F+1
           DO 610 I=1,KMAX
           HGR(I)=HGR4(I,IT)
           IF(MODIND,LT,0) GO TO 610
           DO 620 J=1,LMAX
           XR(I,J)=XR4(I,J,IT)
      620 YR(I,J)=YR4(I,J,IT)
      610 CONTINUE
           DO 630 J=2,B
           630 HBR(J)=HBR4(J,IT)
           640 J=IND#1
           IS#1
           DO 710 L=1,LMAX
           JC#1+(LT*(L-1))

```

PT2-0096  
PT2-0097  
PT2-0098  
PT2-0099  
PT2-0100  
PT2-0101  
PT2-0102  
PT2-0103  
PT2-0104  
PT2-0105  
PT2-0106  
PT2-0107  
PT2-0108  
PT2-0109  
PT2-0110  
PT2-0111  
PT2-0112  
PT2-0113  
PT2-0114  
PT2-0115  
PT2-0116  
PT2-0117  
PT2-0118  
PT2-0119  
PT2-0120

PT2-0121  
PT2-0122  
PT2-0123  
PT2-0124  
PT2-0125  
PT2-0126  
PT2-0127  
PT2-0128  
PT2-0129  
PT2-0130  
PT2-0131  
PT2-0132  
PT2-0133  
PT2-0134  
PT2-0135  
PT2-0136  
PT2-0137  
PT2-0138  
PT2-0139  
PT2-0140  
PT2-0141  
PT2-0142  
PT2-0143  
PT2-0144  
PT2-0145

SUBROUTINE PART2      74/74      OPT#2      F7N 4.6+420      08/22/77 16.51.06

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160      DO 680 K=1,KMM          PT2=0146
        I1=1+(K2*(K-1))          PT2=0147
        I2=I1+KTK              PT2=0148
        DXR=(XR(K+1,L)-XR(K,L))/AI    PT2=0149
        DYR=(YR(K+1,L)-YR(K,L))/AJ    PT2=0150
        GO TO (650,660),IS          PT2=0151
165      650 DMK=(HGR(K+1)-HGR(K))/AI    PT2=0152
        DO 680 IC=I1,I2          PT2=0153
        DFU=IC-1                PT2=0154
        Y2(IC,JC)=YR(K,L)+(DYR*(DFU+.5))  PT2=0155
        X2(IC,JC)=XR(K,L)+(DXR*DFU)       PT2=0156
        GO TO (670,680),IS          PT2=0157
170      670 HG2(IC)=HGR(K)+DHR*(DFU+.5)  PT2=0158
        680 CONTINUE              PT2=0159
        DO 690 IRT=2,8            PT2=0160
175      690 H82(IRT)=H8R(IRT)          PT2=0161
        GO TO (700,710),IS          PT2=0162
        700 IS=2                  PT2=0163
        710 CONTINUE              PT2=0164
        DO 740 I=1,IM              PT2=0165
        DO 730 L=1,LMM              PT2=0166
        J1=1+(L2*(L-1))          PT2=0167
        J2=J1+LJK              PT2=0168
        JIKI= J1+KZ              PT2=0169
        JILJ=J1+LZ              PT2=0170
        DXR=(X2(I,JIKI)-X2(I,J1))/AI    PT2=0171
        DYR=(Y2(I,J1,LJ)-Y2(I,J1))/AJ    PT2=0172
        DO 720 J=J1,J2              PT2=0173
        DFU=J-J1                PT2=0174
        X2(I,J)=X2(I,J1)+DXR*(DFU+.5)  PT2=0175
        Y2(I,J)=Y2(I,J1)+DYR*DFU      PT2=0176
        720 CONTINUE              PT2=0177
        730 CONTINUE              PT2=0178
        740 CONTINUE              PT2=0179
        IF (IFIRST)750,800,800      PT2=0180
195      750 IFIRST=1              PT2=0181
        GO TO 570              PT2=0182
        800 CONTINUE              PT2=0183
        810 ANUP=NFI              PT2=0184
        WIND=J+IND              PT2=0185
        DFU=(WIND-1.)/ANUP        PT2=0186
        DFUM=DFU*(1./ANUP)        PT2=0187
        DO 820 K=2,8              PT2=0188
        820 H8(K)=H81(K)+DFUM*(H82(K)-H81(K))  PT2=0189
        HG(IM)= HG1(IM) + DFUM*(HG2(IM)-HG1(IM))  PT2=0190
205      C   SWEET WHOLE FIELD FOR FLOW FROM BLOCKS
        C
        C   830 DO 2010 J=1,JMM          PT2=0191
        C   THIS BRANCH SKIPS THE INVESTIGATION OF POSSIBLE BARRIERS FOR THE
        C   ROW J=1. FOR J=1 THE INDICATOR LG3 IS SET. IF J IS GREATER THAN
        C   1 A SEARCH FOR BARRIERS IN THE ROW WILL TAKE PLACE.
        C   840 KJ = 0                  PT2=0192

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SUBROUTINE PART2 74/74 OPTER FTN 4.6+420 08/22/77 16.51.06

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L=0 PT2=0193
215 C A NORMAL COMPUTATION SEQUENCE WILL OCCUR. THE FIRST X=DIR FLUX
C TEMPORARY STORAGE IS SET AS THAT OF THE FIRST COLUMN.
C THE NIMREP AND LOCATIONS OF THE BARRIERS PRESENT IN THE ROW ARE
C FOUND AND PLACED IN TEMPORARY STORAGE. IF NO BARRIERS ARE PRESENT
C THE INDICATOR KJ REMAINS ZERO.
IF (KM,EQ.0) GO TO 870 PT2=0194
220 LD 860 KE1,KM PT2=0195
IF (J=JA(K))860+850+860 PT2=0196
850 KJ = KJ + 1 PT2=0197
L=L+1 PT2=0198
KB(L)=K PT2=0199
225 C
840 CONTINUE PT2=0200
C BASED ON KJ, THE INDEX LJ IS SET TO INDICATE THE BARRIER SITUATION
C IN THE MATR COMPUTATION. LJ=1 FOR NO BARRIERS.
IF(KJ)870,870,880 PT2=0201
230 870 LJ=1 PT2=0202
GO TO 890 PT2=0203
880 LJ=2 PT2=0204
C
235 C THIS IS THE PRIMARY LOOP FOR STEPPING THRU THE IM GRID COLUMNS.
890 DO 2000 IE1,IMM PT2=0205
C BEGIN THE EXAMINATION OF THE BASIC TRIAD OF GRID SQUARES. THE
C DUMMY VARIABLES H1,D1,H2,D2 AND Q ARE USED TO ALLOW ONE ROUTINE TO
C BE EMPLOYED FOR BOTH SETS OF SQUARES. SQUARES ONE AND TWO ARE
C TAKEN FIRST.
240 900 IF (J=1)910,910,920 PT2=0206
910 HG(I)=HG1(I)+DFUM*(HG2(I)-HG1(I)) PT2=0207
920 X(I)=FS*(X1(I,J)+DFU*(X2(I,J)-X1(I,J))) PT2=0208
Y(I)=SY*(Y1(I,J)+DFU*(Y2(I,J)-Y1(I,J))) PT2=0209
H1 = H(J,J) PT2=0210
Z = IZ(I,J) PT2=0211
D1 = H1-Z PT2=0212
C THIS BRANCH WILL SET UP A SEARCH FOR A BARRIER IN THE SQUARES
C BEING CONSIDERED IF LJ=2. IF LJ=1 OR THE BARRIER EXISTS BETWEEN
C THE OTHER PAIR OF SQUARES AN INDEX IS SET, LI=1, FOR A BARRIER.
250 C
LJ=2.
1000 GO TO (1040+1010)*LJ PT2=0213
C THIS X LOOP SEARCHES FOR A BARRIER IN THE PAIR OF SQUARES.
1010 DO 1030,K=1,KJ PT2=0214
K1= KB(K) PT2=0215
255 IF(I=IR(K1))1030+1020+1030 PT2=0216
1020 LI=2 PT2=0217
GO TO 1050 PT2=0218
C
260 1030 CONTINUE PT2=0219
1040 LI=1 PT2=0220
C THE DUMMY VARIABLES H2 AND D2 ARE SET FOR THE SQUARE ONE AND TWO
C CALCULATION. THIS IS INDICATED BY LQ=1.
1050 CONTINUE PT2=0221
1053 H2 = H(I+1,J) PT2=0222
1054 Z = IZ(I+1,J) PT2=0223

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SUBROUTINE PART2

74/74 OPT=2

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D2 = H2 -Z          PT2-0224
LQ = 1              PT2-0225

C
270   C THE INVESTIGATION OF THE RELATION BETWEEN DATUMS OF BOTH PAIRS OF
      C SQUARES BEGINS HERE. THIS BRANCH TESTS LI FOR A BARRIER.
      C GO TU (1110,1070)+LI          PT2-0226
      C A BARRIER EXISTS AND ON THE BASIS OF LQ THE DATUM IS ASSIGNED THE
      C PROPER BARRIER WEIGHT.
      C GO TU (1080,1090)+LQ          PT2-0227
275   1080 ZB = IZX(XT)          PT2-0228
      CDDI = ICDOX(KI)          PT2-0229
      CDSI = ICDSX(KI)          PT2-0230
      GO TO 1100          PT2-0231
      1090 ZB = IZY(KI)          PT2-0232
      CDDI = ICDOY(KI)          PT2-0233
      CDSI = ICDSY(KI)          PT2-0234
      1100 ZB = ZB *.1          PT2-0235
      CDDI = CDDI *.001          PT2-0236
      CDSI = CDSI *.001          PT2-0237
      GO TO 1140          PT2-0238

280   C NO BARRIER EXISTS. THE RELATIVE DATUM HEIGHTS OF THE SQUARES ARE
      C TESTED AND THE HIGHER DATUM SET EQUAL TO ZB.
      CDDI = CDD          PT2-0239
      IF (H1-D1-H2+D2)1120+1130          PT2-0240
290   1120 ZB=D2          PT2-0241
      GO TU 1140          PT2-0242
      1130 ZB = H1 - D1          PT2-0243
      C THE INVESTIGATION OF THE DEPTH SIGNATURES BEGINS AT THIS POINT.
      C THE PROPER ASSIGNMENT IS MADE FOR THE FLUX CALCULATION.
      C
      1140 IF(D1)1150,1160,1190          PT2-0244
      1150 LM#1          PT2-0245
      GO TO 1170          PT2-0246

300   C
      1160 LM#2          PT2-0247
      1170 IF(D2)1360,1360,1180          PT2-0248
      1180 IF(H2-ZB)1360+1360,1260          PT2-0249
      1190 IF(D2)1200,1210,1230          PT2-0250
      1200 LM#1          PT2-0251
      GO TO 1220          PT2-0252
      1210 LM#2          PT2-0253
      1220 IF(H1-ZB)1360+1360,1270          PT2-0254
      1230 IF(H1-ZB)1160+1160,1240          PT2-0255
      1240 IF(H2-ZB)1250+1250,1280          PT2-0256
      1250 LM#2          PT2-0257
      GO TO 1270          PT2-0258
      1260 DH=ZB-H2          PT2-0259
      DP=ABS(DH)
      TAD = 4.* D2          PT2-0260
      GO TO (1290+1350)+LM          PT2-0261
      PT2-0262

315   C
      1270 DH=H1-ZB          PT2-0263
      DP=ABS(DH)          PT2-0264

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SUBROUTINE PART2

78/74 OPT=2

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320      TAD = 4.* D1          PT2=0265
        GO TO ((1300+1350)+LM
1280     GO TU ((1460+1330)+LT
1290     H(I,J)=H1=01          PT2=0267
        GO TU 1350
1300     GO TU ((1310+1320)+LQ
1310     H(I+1,J) = H2 = D2    PT2=0268
        GO TU 1350
1320     H(I,J+1) = H2 = D2    PT2=0269
        GO TU 1350
1330     IF (ZB=(H1=01)) 1460,1460+1340    PT2=0270
1340     IF(ZB=(H2=D2))1460,1460+1370    PT2=0271
1350     IF(CP.LT.0.000001) GU TO 1360    PT2=0272
        DREW = (CPOI *DH)*(CDOI*DH)
        GO TO 1380
1360     Q=0.                  PT2=0273
        GO TO 1570
1370     DH=H1=H2              PT2=0274
        TAD = D1+D2
        D8 =((H1+H2)/2.) -ZB) * COSI
        DREW = DR*D8
1380     GO TO ((1390+1400)+LQ
1390     Q = U(I+1,J)          PT2=0275
        PUSH = X(I) * DELT
        GO TO 1450
1400     Q = V(I,J+1)          PT2=0276
1440     PUSH = Y(I) * DELT    PT2=0277
        C
        SPECIAL CALCULATION OF Q FOR BARRIERS
1450     GDS=GRAV*DREW          PT2=0278
        RG=GDS/(C2*TAD)
350      FORCE=RG*(Q+PUSH)+GDS*DH          PT2=0279
        HRG=RG/2.
        Q = SQRT((ABS(FORCE)+HRG**2)) = HRG
        IF(FORCE.LT.0.) Q = -Q
        GO TO 1570
355      C
        1460     GO TO ((1470+1490)+LQ
        1470     Q = U(I+1,J)          PT2=0280
        1480     B1 = V(I,J) + V(I+1,J) + V(I+J+1) + V(I+1+J+1)    PT2=0281
        PUSH = X(I)*DELT
        GO TO 1510
        1490     Q = V(I,J+1)          PT2=0282
        1500     B1 = U(I,J) + U(I+1,J) + U(I+J+1) + U(I+1+J+1)    PT2=0283
        1505     PUSH=Y(I)*DELT
        1510     A1 = 4.* Q
        1520     R = SQRTF ((A1 * A1) + (B1 * B1))
        1530     G = 1. + ((C1 * R) / ((D1+D2)*(D1+D2)))          PT2=0284
        TAD = D1+D2
        DH = H1=H2
        1540     IF(PUSH)1541,1542+1542          PT2=0285
        1541     IF(D2=0.0)1560,1560+1545          PT2=0286
        1545     IF(D2=0.1)1544,1544+15e0          PT2=0287
        PT2=0288
        PT2=0289
        PT2=0290
        PT2=0291
        PT2=0292
        PT2=0293
        PT2=0294
        PT2=0295
        PT2=0296
        PT2=0297
        PT2=0298
        PT2=0299
        PT2=0300
        PT2=0301
        PT2=0302
        PT2=0303
        PT2=0304
        PT2=0305
        PT2=0306
        PT2=0307
        PT2=0308
        PT2=0309
        PT2=0310
        PT2=0311
        PT2=0312
        PT2=0313
        PT2=0314

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SUBROUTINE PART2      74/74      OPT#2      FTM 4.6+420      08/22/77 16.51.06

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1542 IF(D1=0.0)1560,1560+1543      PT2-0315
1543 IF(D1=0.111544,1544+1560      PT2-0316
1544 PUSH#0.0      PT2-0317
375      G=(G+1.)*.07+1.      PT2-0318
C
C      STANDARD CALCULATION OF G FOR BLOCKS
1560 G =(1.0/G)*( G +(C2 * TAD * UN)+ PUSH)      PT2-0319
C
380      C      THE H AND D CALCULATIONS ARE MADE ON THE BASIS OF THE INDEX LQ. IF
C      LQ=1 THE CALCULATIONS ARE POSTPONED AND A RETURN TO THE POINT OF
C      INVESTIGATION OF THE DATUM RELATIONSHIPS IS MADE (STATEMENT 21)
C      AFTER THE DUMMY VARIABLES H2 AND D2 ARE SET UP FOR THE ONE-THREE
C      SQUARES.
385      1570 GD1=D1/C3      PT2-0320
      GD2=D2/C3      PT2-0321
      IF(ABS(D).LT.1.E-10) G=0.0      PT2-0322
      GO TO (1571,1681),LQ      PT2-0323
C
390      1571 IF(G)1572,1577,1573      PT2-0324
      1577 M[LW]=1      PT2-0325
      GO TU 1580      PT2-0326
      1578 ML=80      PT2-0327
      IF(H2=ZR)1576,1575+1575      PT2-0328
395      1576 G=0.0      PT2-0329
      GO TO 1580      PT2-0330
      1575 IF(302+0)1574,1574+1580      PT2-0331
      1574 G=999      PT2-0332
      GO TO 1580      PT2-0333
400      1573 ML=81      PT2-0334
      IF(H1=ZR)1576,1580,1580      PT2-0335
      1580 IF(I=1)1590,1590+1630      PT2-0336
      1590 IF(J=JBL)1610,1610+1620      PT2-0337
C      LEFT HAND SEAWARD BOUNDARY CONDITION
405      1610 H(1,J)=HG(1)      PT2-0338
      1620 UN=0.      PT2-0339
      GO TO 1670      PT2-0340
C
410      1630 IF(J=JBR)1640,1640+1670      PT2-0341
      1640 IF(I=IMM)1670,1650,1670      PT2-0342
C      RIGHT HAND SEAWARD BOUNDARY CONDITION
415      1650 H(IMM,J) = HG(IMM)      PT2-0343
      GO TO 1680      PT2-0344
      1670 UN=0      PT2-0345
420      1680 H2 = H(I,J+1)      PT2-0346
      Z= IZ(I,J+1)      PT2-0347
      D2 =+2 -Z      PT2-0348
      LQ = ?      PT2-0349
      2682 GO TU 1^60      PT2-0350
C
      1681 IF(D)1671,1674,1673      PT2-0351
      1674 IF(MLW)1690,1688,1689      PT2-0352
      1671 IF(H2=ZR)1672,1682,1682      PT2-0353
      1672 G=0.0      PT2-0354

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SUBROUTINE PART2 74/74 \* OPT=2 FTN 4.6+420 08/22/77 16.51.06

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425      1682 IF(MLW)1684,1684,1685 PT2=0355
          1685 IF(GD1-UN1)1686,1686,1684 PT2=0356
          1686 UN1=GD1 PT2=0357
          1687 IF(GD2*Q)1687,1687,1690 PT2=0358
          1688 Q=GD2 PT2=0359
        GO TO 1690 PT2=0360
430      1689 IF(H1-ZR)1672,1683,1683 PT2=0361
          1690 IF(MLW)1688,1689,1689 PT2=0362
          1691 IF(GD1-R)1692,1692,1690 PT2=0363
          1692 R=GD1 PT2=0364
        GO TO 1690 PT2=0365
          1693 IF(R+UN1-GD1)1690+1690,1691 PT2=0366
          1694 ADDQ = R+UN1 + 0.00001 PT2=0367
          Q = (Q/(ADDQ))*GD1 PT2=0368
          UN1 = (UN1/(ADDQ))*GD1 PT2=0369
440      1695 UN1 = 0 PT2=0370
          U(I,J)=UN PT2=0371
          UN = UN1 PT2=0372
          V(I,J) = VN(I) PT2=0373
          VN(I) = VN1 PT2=0374
445      2000 CONTINUE PT2=0375
          U(I,M,J)=UN1 PT2=0376
        2010 CONTINUE PT2=0377
          IF(KCM.GT.0) CALL CHANL(2) PT2=0378
C
450      C SWEEP WHOLE FIELD FOR H ON BLOCKS
C
          SUM=0, PT2=0379
          COUNT=0, PT2=0380
        DO 2020 J=1,JMN PT2=0381
        DO 1790 I=1,IMM PT2=0382
          Z=IZ(I,J) PT2=0383
          D1=H(I,J)=Z PT2=0384
          IF(J=1)1700,1700,1710 PT2=0385
          1700 H(I,I) = HG(I) PT2=0386
          1710 IF(D1)1740,1720,1720 PT2=0387
          1720 IF(J=1)1790,1790,1721 PT2=0388
          1721 IF(I=1)1722,1722,1723 PT2=0389
          1722 IF(J=JRL)1790,1790,1729 PT2=0390
          1723 IF(I=TMM)1729,1724,1724 PT2=0391
          1724 IF(J=JBR)1790,1790,1729 PT2=0392
          1729 SETUP=C3*(U(I,J)-U(I+1,J)+V(I,J)-V(I,J+1)) PT2=0393
          H(I,J)= H(I,J) + SETUP + RAIN PT2=0394
          SUM=SUM+ABSF(H(I,J)) PT2=0395
          COUNT=COUNT+1, PT2=0396
        470      IF ( D1 + SETUP + RAIN ) 1740,1740,1750 PT2=0397
          1740 H(I,J) = TZ(I,J) PT2=0398
C
          1750 IF(KCM.GT.0) GO TO 1790 PT2=0399
C
475      C ENTER RUNOFF VALUES ON ENTRY BLOCKS ONLY IF CHANNELS NOT PROVIDED
          DO 1770 IJK=1,NUMRO PT2=0400
          IF ( LROJ(TJK)=J)1770,1760,1770 PT2=0401
  
```

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1760 IF(LR0(IJK)=T)1770+1780+1770 PT2=0402
1770 CONTINUE PT2=0403
480      GO TO 1790 PT2=0404
1780 H(I,J)= H(I,J) + MRO(IJK)*DELT/(DELX**2) PT2=0405
1790 CONTINUE PT2=0406
2020 CONTINUE PT2=0407
IF(KCM,GT,0) CALL CHANL(3) PT2=0408

485      C
C      THE TIME INDICES ARE STEPPED TO THE NEW LEVEL. PT2=0409
NT = NT +1 PT2=0410
NTIME = NTIME + 1 PT2=0411
JWIND=JWIND+1 PT2=0412
490      IF(SUM/COUNT>100.) 2075,2140+2140
C      TEST THE STABILITY OF THE COMPUTATIONS VIA AVE ABS(H). PT2=0413
C      COMPUTATIONS ARE STABLE. CALCULATIONS CONTINUE, PT2=0414
C      COMPUTATIONS ARE UNSTABLE. AN ON-LINE MESSAGE IS PRINTED PT2=0415
C
495      C      TEST NT FOR THE OUTPUT OF U,V,H,D,X,Y FIELDS. PT2=0416
2075 TIM=NTIME-INTIME PT2=0417
ITIM=TIM PT2=0418
HINT=INTEP PT2=0419
IF((TIM/HINT)=ITIM/INTER) 2055,2055+2090 PT2=0420
500      2055 CALL SAVE(2) PT2=0421
2090 IF(NT=IOUT) 2110,2195,2100 PT2=0422
C      OUTPUT U,V,H,D,X,Y FIELDS. RESET NT=0 AND STEP NN. PT2=0423
2105 IF(INFL,EG,0) GO TO 2110 PT2=0424
CALL CHANL(4) PT2=0425
505      GO TO 211^ PT2=0426
2100 NT = 0 PT2=0427
NN = NN + 1 PT2=0428
HOUR = NTIME/NF PT2=0429
CALL CHANL(4)
510      2110 CONTINUE PT2=0430
C
2130 IF (NN= NTIME) 2160,2160,200 PT2=0431
C      STORM COMPLETED. FINAL OUTPUT ON TAPES. PT2=0432
2140 PRINT 2150,NN
2150 FORMAT (21H STOP BS AT NTIME =   ,I4)
STOP
C
515      2160 CALL SAVE(3) PT2=0433
CALL CONTIN(2) PT2=0434
RETURN PT2=0440
END PT2=0441

```

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```

1      C
2      C
3      C      SUBROUTINE CHANL(N)          CHNL0001
4      C
5      COMMON/RLK1/ IR(100),JB(100),IZX(100),IZY(100),ICDOX(100)
6      1,ICDOY(100),ICDSX(100),ICDSY(100),LR0I(R),LR0J(S),DIST(24)   CHNL0004
7      2,CHST(30),R0(R,30),HGR(8),XR(8,6),YR(8,6),HAR(8)           CHNL0005
8      COMMON/RLK2/ IZ(28,20),U(28,20),V(28,20),H(28,20),NTIME       CHNL0006
9      COMMON/RLK3/ KM,MMIN,MMAX,NFU,INFLD,IM,JM,KM,KMAX,LMAX,DELT   CHNL0007
10     1,CDO,FK,HGI,IOUT,KI,LJ,KII,LJJ,JdL,JBR,KMM,LKM,RF,CONST,S CHNL0008
11     2,IHRG,JHRD,KR,ISTR,IND,NOW,KIM,NORT,NTIME,INTIME,NOND,GRAV CHNL0009
12     3,KCHP,DFU,INTER           CHNL0010
13     COMMON/RLK4/ H00(8),CRO(8),KB(24),X1(28,21),Y1(28,21),X2(28,21) CHNL0011
14     1,Y2(28,21),X(28),Y(28),HG1(28),HG2(28),H81(R),H82(Q),VN(28) CHNL0012
15     2,HG(28),HR2(8)           CHNL0013
16     COMMON/RLK5/ ICG(130),JCG(130),IACX(130),IACY(130),IZCX(130) CHNL0014
17     1,IZCY(130),RCXP(130),CCXN(130),RCYP(130),QCYN(130),HC(130),HP(130) CHNL0015
18     2,KC,KCY(130),KCY(130),KCB(130),UCT(130),UCF(130),XRI(130),IRON CHNL0016
19     3,KEN(2,130),VCT(130),VCF(130),ACGX(130),AGY(130),KCXP(130) CHNL0017
20     4,KCYP(130),KLB(50),KLM,IFCC(130),FC           CHNL0018
21     COMMON/RLK7/ IEND,NF,IBL,~NJ,R(40)           CHNL0019
22     COMMON/RLK9/ KZ,LZ,NHRD,C1,C2,C3,IHM,JHM,NT,NF,EXT1,IT,KC,IFIRST CHNL0020
23     1,JIND,NEW1,XNOW,NEW3,XNORT, C4,RAIN,AJ,AI,LJK,KIK           CHNL0021
24     EQUIVALENCE (DA,DAC)           CHNL0022
25     C
26     ABSF(X) = ABS(X)           CHNL0023
27     SQRTF(X) = SQRT(X)         CHNL0024
28     C
29     GO TO (1000,2000,3000,4000) +N          CHNL0025
30     C
31     CHANNEL CODE 1 IS FOR READING CHANNEL DATA AND ESTABLISHING KEY ARRAYS
32     CHANNEL CODE 2 IS FOR FLOW AND HEIGHT CALCULATIONS IN CHANNELS
33     CHANNEL CODE 3 IS FOR CALCULATION OF H ON BLOCKS CONTAINING CHANNELS
34     CHANNEL CODE 4 IS FOR LISTING OF CHANNEL OUTPUT
35     C
36     ENTRY POINT 1 FOR READING CHANNEL DATA, INITIALIZATION AND FOR
37     C ESTABLISHING KEY ARRAYS FOR ROUTINE CALCULATIONS
38     C
39     1000 PRINT 500           CHNL0027
40     PRINT 500           CHNL0028
41     500 FORMAT(I1 THE FOLLOWING ARE SUBGRID CHANNEL DATA- Z VALUES IN FECHL0029
42     1ET(+/)
43     CH=(DFLY**2)/DELT           CHNL0030
44     C
45     C A NEGATIVE I-CX OR IACY IDENTIFIES THOSE CHANNELS WITH BARRIERS OF
46     C EQUAL ELEVATION ON BOTH SIDES SUCH AS A JETTY SYSTEM
47     C FOR SINGLE RAPIERS, THE LATTER IS TAKEN ON THE INNER SIDE OF THE
48     C CHANNEL BLOCK IF IZC IS NEGATIVE, WHILE ON THE OUTER SIDE IF IZC IS
49     C POSITIVE
50     DO 50 K=1,XCN           CHNL0032
51     READ 501, IDENT, ICG(K),JCG(K),IACX(K),IZCX(K),IACY(K),IZCY(K) CHNL0033
52     1,IFC(K)           CHNL0034
53     1F(IFC(K).EQ.,0) IFC(K)= FC*10000           CHNL0035

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501 FORMAT(I1,2X,I5,9(3X,I5))
55   IF(IOENT.NE.8) GO TO 510
      CONTINUE
      DO 100 K=1,KCM
      KEN(1,K)=0
      KEN(2,K)=0
      KRI(K)=0
      KCX(K)=0
      KCY(K)=0
      KCXP(K)=0
      KCYP(K)=0
      I=ICG(K)
      J=JCG(K)
      DO 80 L=1,KCM
      IF(ICG(L).EQ.(I+1).AND.JCG(L).EQ.J) KCYP(K)=L
      IF(ICG(L).EQ.I.AND.JCG(L).EQ.(J+1)) KCXP(K)=L
      IF(ICG(L).EQ.(I-1).AND.JCG(L).EQ.J) KCY(K)=L
      IF(ICG(L).EQ.I.AND.JCG(L).EQ.(J-1)) KCX(K)=L
      80 CONTINUE
      KCB(K)=0
      IF(KM.EQ.0) GO TO 91
      DO 90 L=1,KM
      IF(Ib(L).EQ.I.AND.Jb(L).EQ.J) KCB(K)=L
      90 CONTINUE
      91 CONTINUE
      UCT(K)=0.0
      UCF(K)=0.0
      VCT(K)=0.0
      VCF(K)=0.0
      HP(K)=H(I,J)
      PRINT 502, K,ICG(K),JCG(K),I+CX(K),IZCX(K)+I+CY(K),IZCY(K),IFC(K)
502 FORMAT(I,K=1,I3,( ICG=1,I3,( I+CX=1,I5,( IZCX=1,I4
      1,( I+CY=1,I5,( IZCY=1,I4,( IFC=1,I4)
      100 CONTINUE
      C
      C     ARRAY KLB IDENTIFIES BARRIER BLOCKS WHICH ARE NOT COMMON
      C     WITH CHANNEL BLOCKS
      C     LC=0
      C     DO 105 K=1,KM
      C     I=IB(K)
      C     J=JB(K)
      C     DO 102 L=1,KCM
      C     IF(ICG(L).EQ.I.AND.JCG(L).EQ.J) GO TO 105
      C     102 CONTINUE
      C     LC=LC+1
      C     KLB(LC)=K
      C     105 CONTINUE
      C     KLM=LC
      C
      C     THE FOLLOWING CREATES A SPECIAL INDEX FOR CHANNEL STARTING AND END
      C     ANY BLOCK WITH NEGATIVE ICG OR JCG IDENTIFIES A CHANNEL END POINT
      C     ARRAY KEN IDENTIFIES WHAT TYPE OF END POINT EXISTS ACCORDING TO THE
      C     1 KCX H      5 KCA Q

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SUBROUTINE CHANL

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C	2 KCY H	6 KCY Q	
C	3 KCXP H	7 KCXP Q	
C	4 KCYP H	8 KCYP Q	
110 C	I80=0 DO 200 K=1,KCM I=ICG(K) J=JCG(K)		CHNL0081 CHNL0082 CHNL0083 CHNL0084
115 C	IF(KCX(K),NE,0) GO TO 110 IF(I=CY(K),EQ,0) GO TO 110 I80=I80+1 KS=KCM+I80 KCX(K)=KS ICG(K)=I KEN(1,K)=1 IF(J,EQ,1) GO TO 110 Z=IZ(I,J+1) IF((H(I,J+1)=Z),LE,0) KEN(1,K)=5		CHNL0085 CHNL0086 CHNL0087 CHNL0088 CHNL0089 CHNL0090 CHNL0091 CHNL0092 CHNL0093 CHNL0094
120 C	110 IF(KCY(K),NE,0) GO TO 120 IF(I=CY(K),EQ,0) GO TO 120 I80=I80+1 KS=KCM+I80 KCY(K)=KS IF(ICG(K),LT,0) JCG(K)=J ICG(K)=I L=1 IF(JCG(K),LT,0) L=2 KEN(L,K)=2 IF(I,EQ,1,AND,J,LE,JBL) GO TO 120 KEN(L,K)=6 IF(I,FE,1) GO TO 120 Z=IZ(I-1,J) IF((H(I-1,J)=Z),GT,0) KEN(L,K)=2		CHNL0095 CHNL0096 CHNL0097 CHNL0098 CHNL0099 CHNL0100 CHNL0101 CHNL0102 CHNL0103 CHNL0104 CHNL0105 CHNL0106 CHNL0107 CHNL0108 CHNL0109
130 C	120 KX=KCXP(K) KY=KCYP(K) IF(I=CY(K),NE,0) GO TO 130 IF(KY,EQ,0) GO TO 121 IF(I=CY(KY),NE,0) GO TO 130 121 IF(KX,EQ,0) GO TO 125 IF(I=CY(KY),NE,0) GO TO 130 I80=I80+1 KCXP(K)=KCM+I80 125 IF(ICG(K),LT,0) JCG(K)=J ICG(K)=I L=1 IF(JCG(K),LT,0) L=2 KEN(L,K)=7 IF(J,EQ,JMM) GO TO 130 Z=IZ(I,J+1) IF((H(I,J+1)=Z),GT,0) KEN(L,K)=5		CHNL0110 CHNL0111 CHNL0112 CHNL0113 CHNL0114 CHNL0115 CHNL0116 CHNL0117 CHNL0118 CHNL0119 CHNL0120 CHNL0121 CHNL0122 CHNL0123 CHNL0124 CHNL0125 CHNL0126

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160      C
        130 IF(I+CX(K).NE.0) GO TO 200
        IF(KX.EQ.0) GO TO 131
        IF(I+CX(KX).NE.0) GO TO 200
        131 IF(KY.EQ.0) GO TO 135
        IF(I+CY(KY).NE.0) GO TO 200
        IBO=IBO+1
        KCYP(K)=KCM+IBO
        135 IF(ICG(K).LT.0) JCG(K)=-J
        ICG(K)=1
        L=1
        IF(JCG(K).LT.0) L=2
        KEN(L,K)=R
        IF(I.GE.IHM.AND.J.GT.JBR) GO TO 200
        KEN(L,K)=4
        175 IF(I.GE.IHM) GO TO 200
        Z=IZ(I+1,J)
        IF(H(I+1,J)=Z).LE.0 KEN(L,K)=8
        200 CONTINUE
        C
        180 C IBOM IS THE TOTAL NUMBER OF CHANNEL END POINTS OF ANY KIND
        IBOM = IBO
        KCMR=KCM+IBOM+1
        DO 210 K=1,KCMR
        HC(K)=HGI
        GCXP(K)=0.
        QCYP(K)=0.
        GCYN(K)=0.
        QCYR(K)=0.
        AOGX(K)=0.
        AOGY(K)=0.
        210 CONTINUE
        C
        C
        C ARRAY KRI IDENTIFIES THE LOCATIONS OF RIVER INPUT FOR Q TYPE END POINTS
        195 LRM=0
        LQM=0
        DO 300 K=1,KCM
        I= ICG(K)
        I= IAHS(I)
        J= JCG(K)
        J= JAHS(J)
        ITOP=KCMR
        IF(KCX(K).EQ.0) KCX(K)=ITOP
        IF(KCY(K).EQ.0) KCY(K)=ITOP
        205 IF(KCXP(K).EQ.0) KCXP(K)=ITOP
        IF(KCYR(K).EQ.0) KCYR(K)=ITOP
        C
        KRI(K)=0
        IF(IRM0.EQ.0) GO TO 480
        LS=1
        IF(ICG(K).GT.0) GO TO 460
        410 KJ=KEN(LS,K)
  
```

SUBROUTINE CHANL

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```
IF(KJ.LE.4) GO TO 460          CHNL0173
L0M=L0M+1                      CHNL0174
215
LR=0                            CHNL0175
DO 450 L=1,140                   CHNL0176
IF(LROI(L),EQ,I,AND,LROJ(L),EQ,J) L=EL
450 CONTINUE                     CHNL0177
KRI(K)=LR                      CHNL0178
220
IF(LR.GT.0) LRM=LRM+1           CHNL0179
460 IF(JCG(K).GT.0) GO TO 300   CHNL0180
IF(LS.EQ.2) GO TO 300          CHNL0181
LS=2                            CHNL0182
GO TO 410                       CHNL0183
225
300 CONTINUE                     CHNL0184
IF(LRM.EQ.I*140) GO TO 480    CHNL0185
C
PRINT 470, LRM,TMRO             CHNL0186
470 FORMAT(1x,'//*****ARV1.G***** ONLY I,I3,
1 CHANNEL END POINTS(I,I3) MATCH THE I,I3 IN RTVER INPUT POSITIONS(
2 //*****'*)                                         CHNL0187
CHNL0188
CHNL0189
C
480 CONTINUE                     CHNL0190
PRINT 549                      CHNL0191
549 FORMAT(I,I)
DO 600 K=1,KCM                 CHNL0192
PRINT 550,K,KCX(K),KCY(K),KCXP(K),KCYP(K),KCB(K),ICG(K),JCG(K)
1,KEN(1,K),KEN(2,K),KRI(K)      CHNL0193
550 FORMAT(I,K,I,KCX=I,I3,I,KCY=I,I3,I,KCXP=I,I3,I,KCYP=I,I3,
1,I,KCB=I,I3,I,ICG=I,I3,I,KEN=I,I3,I,KEN2=I,I3
2,I,KRI=I,I3)                  CHNL0194
CHNL0195
CHNL0196
CHNL0197
CHNL0198
CHNL0199
600 CONTINUE                     CHNL0200
PRINT 551, KCMP                 CHNL0201
551 FORMAT(1x,10X,(KCM=I,15,//)) CHNL0202
C
RETURN                         CHNL0203
510 PRINT 503                   CHNL0204
503 FORMAT(1x,STOP BECAUSE CARDS WITH IDENT = 8 EXPECTED(//))
PRINT 504, IDENT                CHNL0205
504 FORMAT(3X,(IDENT=I4)
STOP                           CHNL0206
CHNL0207
CHNL0208
C ENTRY POINT 2 FOR FLOW AND HEIGHT CALCULATIONS IN CHANNELS
C
255
2000 SDT = S*Delt              CHNL0209
DO 2500 K=1,KCM                CHNL0210
I= ICG(K)                      CHNL0211
I= IAIS(I)                      CHNL0212
J= JCG(K)                      CHNL0213
J= IAIS(J)                      CHNL0214
PUSHU = SDT*(X1(I,J)+DFU*(X2(I,J)-X1(I,J)))  CHNL0215
PUSHV = SDT*(Y1(I,J)+DFU*(Y2(I,J)-Y1(I,J)))  CHNL0216
H1 = H(I,J)                      CHNL0217
Z1 = IZ(I,J)                    CHNL0218
HCI = HC(K)                      CHNL0219
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    D1 = H1-Z1          CHNL0220
    CF= IFC(K)          CHNL0221
    CF= CF*DELT/10000.  CHNL0222
    KK = KCA(K)          CHNL0223
    KC=KCX(K)           CHNL0224
    KC=ABS(KC)           CHNL0225
    IF(KC.EQ.0.) GO TO 2250 CHNL0226
    LS = 1               CHNL0227

270
    C
    Z2 = IZ(I+1,J)      CHNL0228
    H2 = H(I+1,J)      CHNL0229
    D2 = H2-Z2           CHNL0230
    QH = QCXN(K)         CHNL0231
    QP = QCXP(K)         CHNL0232
    GT = UCT(K)          CHNL0233
    GF = UCF(K)          CHNL0234
    PUT = PUSHU          CHNL0235
    PUC = PUSHV*WC       CHNL0236
    KA=KCX(K)           CHNL0237
    ZCS=IZCX(K)          CHNL0238
    ZC=ARS(ZCS)          CHNL0239
    IF(KK.EQ.0.) GO TO 2010 CHNL0240
    ZBC=IZX(KK)          CHNL0241
    ZBC=ZBC/10.           CHNL0242
    CDOI= ICDOX(KK)      CHNL0243
    CDOI= CDOI/1000.      CHNL0244
    CDSI= ICDOX(KK)      CHNL0245
    CDSI= CDSI/1000.      CHNL0246
    GO TO 2020            CHNL0247

275
    C
    ****CUTER RF-ENTRY POINT (X AND Y CHANNELS)
    2010 CDOI = CDO          CHNL0248
    CDSI = CDO          CHNL0249

280
    C
    2020 HN = HC(KA)        CHNL0250
    HAC = (HCI+HN)/2.0     CHNL0251
    DAC = HAC-ZC          CHNL0252
    IF(DAC,GT, 0.0) GO TO 20205 CHNL0253

285
    C
    PRINT 20206, DAC, K     CHNL0254
    20206 FORMAT(1 DAC=1.17.2,1 AT CHANNEL BLOCK(,I4,///) CHNL0255
    GO TO 4000             CHNL0256

290
    C
    20205 CEL = SQRT(GRAY*DAC) CHNL0257
    ALP= C3*CEL           CHNL0258
    CALP = 1.0 - ALP       CHNL0259
    HA = ALP*HN + CALP*ICI CHNL0260
    HE = CALP*HN + ALP*HCI CHNL0261
    GA = ALP*GN + CALP*GP CHNL0262
    GB = CALP*GN + ALP*GP CHNL0263
    LF= 1                 CHNL0264
    LO=0                 CHNL0265

300
    C
  
```

SUBROUTINE CHANNEL	74/74 OPT#2	FTN 4.6+420	08/22/77 16.51.06
320	DI = DI DII = DAC HI = H1 HII = HAC QI = OT		CHNL0266 CHNL0267 CHNL0268 CHNL0269 CHNL0270
325	WI = DELX -WC WII = wC IF(WK.GT.0) GO TO 2022 ZB=Z1 GO TO 2021		CHNL0271 CHNL0272 CHNL0273 CHNL0274 CHNL0275
330	2022 ZH=ZD IF(WS.LT.0.) GO TO 2021 IF(ZCS.GT.0.) ZB=Z1 2021 LQ=1 IF(Z1.GT.ZB) ZB=Z1 ZB=ZB		CHNL0276 CHNL0277 CHNL0278 CHNL0279 CHNL0280
335	C *****INNER RF-ENTRY POINT (SIDES 1 AND 2 OF CHANNEL) 2025 IF(WI-ZB) 2030,2030,2040 2030 IF (HI-ZB) 2060,2060,2070 2040 IF(HI-ZB) 2075,2075,2080 2060 QOUT = 0. GO TO 2100 C OVERFLOW FROM REGION I TO REGION II 2070 DH=HI-ZB GO TO 2090		CHNL0281 CHNL0282 CHNL0283 CHNL0284 CHNL0285
340	C OVERFLOW FROM REGION II TO REGION I 2075 DH=ZB-HI GO TO 2090 C SUBMERGED BARRIERS 2080 GO TO (2081+2082)+ LF		CHNL0286 CHNL0287 CHNL0288
345	2081 QOUT= WI*WII*(HI-HII)/((WI+HII)*DELT) LF= 2 GO TO (2110+2120)+ LQ 2082 GO TO (2083+2084)+ LS 2083 QOUT= U(I+1,J) GO TO 2085		CHNL0289 CHNL0290
350	2084 QOUT= V(I,J+1) 2085 WT = QOUT + ((H1-2.*HAC+H2)*wC-(H1-HAC)*(wC**2)/DELX)/(2.*DELT) HDQ= (WT-QOUT)/2. QTS= QOUT+HDQ		CHNL0291 CHNL0292 CHNL0293 CHNL0294 CHNL0295 CHNL0296 CHNL0297 CHNL0298
355	360 QOUT= QOUT-HDQ GO TO 2120 C 2090 QOUT = C001*DH*SQRT(GRAV*ABS(DH)) LQ=LQ		CHNL0299 CHNL0302
365	2100 GO TO (2110+2120)+LQ C 2110 CI = DAC DII = D2 HI = HAC HII = H2 QI = QF		CHNL0303 CHNL0304 CHNL0305 CHNL0306 CHNL0307 CHNL0308 CHNL0309 CHNL0310

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      WI = WC
      WI = DELX
      QT = QOUT
      IF(KK.GT.0) GO TO 2112
      ZB=Z2
      GO TO 2111
  2112 ZB=ZAC
      IF(ZCS.LT.0.) GO TO 2111
      IF(ZCS.LT.0.) ZB=Z2
  2111 LO=2
      IF(Z2.GT.ZB) ZB= Z2
      ZB2=ZB
      GO TO 2025
  385 C*****END OF INNER RE-ENTRY
  C
  2120 QF=QOUT
  C
  C THE FOLLOWING TESTS CONSTRAIN THE CHANNEL OVERFLOW (QT AND/OR QF) SUCH
  C THAT (QF-QT) CANNOT PRODUCE AN IMPOSSIBLE CHANGE IN HC IN ONE TIME STEP
  C (IE, HC SHOULD NOT FALL BELOW SILL DEPTH NOR RISE ABOVE THE HIGHER
  C OF THE ADJOINING BLOCK H DUE TO OVERFLOW ALONE).
  C
  395 IF(LG.EQ.0) GO TO 2190
      IF(LF.EQ.2) GO TO 2160
      IF((WF-QT).LE.0.0) GU TO 2140
  C NET OUTFLOW= BARRIERS OVERTOPPING
      ZMIN=ZB1
      IF(ZB2.LT.ZMIN) ZMIN=ZB2
      QNET = (HAC-ZMIN)*AC/DELT
      IF((WF-QT).LE.QNET) GO TO 2190
      IF((WF-QT).GT.0.0) GU TO 2130
      QFS=WF**2
      QTS=QT**2
      BUM=QNET/(QFS+QTS)
      WF=BUM*QFS
      QTS=BUM*QTS
      GO TO 2190
  400 2130 IF(QF.LT.0.0) GO TO 2135
      2134 WF=QF-QT
      GO TO 2190
      2135 QT = -(QNET-QF)
      GO TO 2190
  C NET INFLOW= BARRIERS OVERTOPPING
  415 2140 HMAX = H1
      IF (H2,GT,HMAX) HMAX = H2
      QNET = (HMAX-HAC)*AC/DELT
      IF ((QT-QF).LE.QNET) GO TO 2190
      IF ((QF-QT).GT.0.0) GO TO 2150
      QFS = WF**2
      QTS = QT**2
      BUM = QNET/(QFS+QTS)
      QF = -BUM*QFS
      QT = BUM*QTS
      CHNL0311
      CHNL0312
      CHNL0313
      CHNL0314
      CHNL0315
      CHNL0316
      CHNL0317
      CHNL0318
      CHNL0319
      CHNL0320
      CHNL0321
      CHNL0322
      CHNL0323
      CHNL0324
      CHNL0325
      CHNL0326
      CHNL0327
      CHNL0328
      CHNL0329
      CHNL0330
      CHNL0331
      CHNL0332
      CHNL0333
      CHNL0334
      CHNL0335
      CHNL0336
      CHNL0337
      CHNL0338
      CHNL0339
      CHNL0340
      CHNL0341
      CHNL0342
      CHNL0343
      CHNL0344
      CHNL0345
      CHNL0346
      CHNL0347
      CHNL0348
      CHNL0349
      CHNL0350
      CHNL0351
      CHNL0352
      CHNL0353
  420

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SUBROUTINE CHANL

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425      GO TO 2190
2150 IF(GF.GT.0.0) GO TO 2155          CHNL0354
2154 GT = QNET + GF                  CHNL0355
2155 GO TO 2190                      CHNL0356
2155 GF = GT - QNET                  CHNL0357
430      GO TO 2190                      CHNL0358
2160 GO TU (2170+2180)* LD          CHNL0359
2170 IF(OT.GT.0.) GO TO 2175          CHNL0360
C      BARRIER 1 OVERTOPPING OUTWARDS= OTHER SIDE SUBMERGED
QNET = (HAC-ZR1)*C/DELT           CHNL0361
IF((GF=GT).LE.QNET) GO TO 2190
GO = GF-QNET
IF(QO.GT.0.) QO = 0.
GO TO 2179
C      BARRIER 1 OVERTOPPING INWARDS= OTHER SIDE SUBMERGED
2175 QNET = (H1-HAC)*C/DELT         CHNL0362
IF((GT-GF).LE.QNET) GO TO 2190
GO = GF-QNET
IF(QO.LT.0.) QO = 0.
2179 GT = GO                      CHNL0363
GO TO 2190                      CHNL0364
445      GO TU (2180+2185)* LD          CHNL0365
C      BARRIER 2 OVERTOPPING OUTWARDS= OTHER SIDE SUBMERGED
QNET = (HAC-ZR2)*C/DELT           CHNL0366
IF((GF=GT).LE.QNET) GO TO 2190
GO = GF-QNET
IF(QO.LT.0.) QO = 0.
GO TO 2189
C      BARRIER 2 OVERTOPPING INWARDS= OTHER SIDE SUBMERGED
2185 QNET = (H2-HAC)*C/DELT         CHNL0367
IF((GT-GF).LE.QNET) GO TO 2190
GO = GF-QNET
IF(QO.GT.0.) QO = 0.
2189 GF = GO                      CHNL0368
2190 END OF ADJUSTMENT OF OT AND/OR GF          CHNL0369
460      CONTINUE                      CHNL0370
C
C      CHANNEL COMPUTATIONS
AA = WC*CFL                         CHNL0371
GAM = 1.0+CF*SQRT((GN**2+QP**2)/2.)/(WC*DAC**2)    CHNL0372
AOG = AA/GAM                         CHNL0373
B00M = CEI*(DFLT*(GT-GF) + WC*RAIN)           CHNL0374
B0P = (GA+AA*HA+PI*(C+B00M))/GAM            CHNL0375
BN = (GR-AA*HE+PI*(C+B00M))/GAM            CHNL0376
GE TU (2200+2300)*LS                  CHNL0377
470      C
2200 UCT(K) = OT                      CHNL0378
UCF(K) = GF                          CHNL0379
U(I+1,J) = GF                        CHNL0380
QCXP(K) = AP                          CHNL0381
QCXN(K) = AN                          CHNL0382
AOGK(K) = AOG                         CHNL0383
C

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2250  CSEI=CY(X)
      +C=ABS(VCY)
      IF( <C,EO,n.) GO TO 2500
      LS = 2
      ZR = TZ(I,J+1)
      H2 = H(I,J+1)
      D2 = H2-ZR
      485   GR = QCYN(K)
      GP = QCYP(K)
      GT = VCT(K)
      GF = VCF(K)
      PUT = PUSHV
      PUC = PUSHU*C
      KA = KCY(K)
      ZCS=IZCY(K)
      ZCR=ABS(ZCS)
      IF( KK,EO,n.) GO TO 2010
      495   ZRC=IZY(KK)
      ZRC=ZRC/10.
      CDO1=ICD0Y(KK)
      CDO1=CDO1/1000.
      COS1=ICD0Y(KK)
      COS1=COS1/1000.
      500   GO TO 2020
      C*****END OF OUTER RE-ENTRY
      C
      505   2300 VCT(K) = GT
              VCF(K) = GF
              V(I,J+1) = GF
              QCYP(K) = GP
              QCYN(K) = GR
              AOGY(K) = AOG
      510   2500 CONTINUE
      C
      DO 2700 K=1,KCM
      I=ICG(K)
      J=IARS(I)
      J=JCG(K)
      J=IARS(J)
      XK=I-CX(K)
      XK=ABSF(XX)
      YY=I-CY(K)
      YY=ABSF(YY)
      KX = KCXP(YY)
      KY = KCYP(K)
      RA = QCXP(K)
      RB = QCYP(K)
      AGA = ANGX(K)
      AGB = ANGY(K)
      RC=QCX(I,XX)
      AGC=ANGX(KX)
      BD=QCYN(KY)
      AGD=ANGY(KY)
      CHNL0398
      CHNL0399
      CHNL0400
      CHNL0401
      CHNL0402
      CHNL0403
      CHNL0404
      CHNL0405
      CHNL0406
      CHNL0407
      CHNL0408
      CHNL0409
      CHNL0410
      CHNL0411
      CHNL0412
      CHNL0413
      CHNL0414
      CHNL0415
      CHNL0416
      CHNL0417
      CHNL0418
      CHNL0419
      CHNL0420
      CHNL0421
      CHNL0422
      CHNL0423
      CHNL0424
      CHNL0425
      CHNL0426
      CHNL0427
      CHNL0428
      CHNL0429
      CHNL0430
      CHNL0431
      CHNL0432
      CHNL0433
      CHNL0434
      CHNL0435
      CHNL0436
      CHNL0437
      CHNL0438
      CHNL0439
      CHNL0440
      CHNL0441
      CHNL0442
      CHNL0443
      CHNL0444
      CHNL0445
      CHNL0446
      CHNL0447

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C
      HCM = (RA+RB+RC-BD)/(AGA+AGR+AGC+AGD)          CHNL0448
      QAERA=AGA*HCM                                     CHNL0449
      QB=BR*AGB*HCM                                     CHNL0450
      GCXN(KX) = BC*AGC*HCM                           CHNL0451
      QCYN(KY) = BD*AGD*HCM                           CHNL0452
      HC(K)= HCM                                      CHNL0453
      IF(JCG(K).LT.0) GO TO 2600                      CHNL0454
      GO TO 2695                                         CHNL0455

  535   C
      BOUNDARY CONDITIONS FOR Q END POINTS           CHNL0456
      2600 L=1                                         CHNL0457
      2605 KEY=KEN(L,K)                                CHNL0458
      GO TO(2690,2690+2630+2640+2650+2660+2670+2680)+ KEY
  545   2630 QA=QCXP(K)                                CHNL0459
      GO TO 2690                                       CHNL0460
      2640 QB=QCYP(K)                                CHNL0461
      GO TO 2690                                       CHNL0462

  550   C
      THE FOLLOWING ASSUMES Q=0 AT END IF NO DISCHARGE DATA EXISTS
      2650 BA=QCXM(K)                                CHNL0463
      KS=KCX(K)                                     CHNL0464
      KT=KRI(K)                                     CHNL0465
      QCXM(K)= 0.                                    CHNL0466
      IF(KT.GT.0) QCXM(K)= HRO(KT)                  CHNL0467
      HC(KS)=(QCXM(K)-BA)/AOGX(K)                  CHNL0468
      GO TO 2690                                       CHNL0469
      2660 BA=QCYN(K)                                CHNL0470
      KS=KCY(K)                                     CHNL0471
      KT=KRI(K)                                     CHNL0472
      QCYN(K)= 0.                                    CHNL0473
      IF(KT.GT.0) QCYN(K)= HRO(KT)                  CHNL0474
      HC(KS)=(QCYN(K)-BA)/AOGY(K)                  CHNL0475
      GO TO 2690                                       CHNL0476
  560   2670 BA=QCXP(K)                                CHNL0477
      KT=KRI(K)                                     CHNL0478
      QA= 0.                                         CHNL0479
      IF(KT.GT.0) QA= -HRO(KT)                     CHNL0480
      HC(K)=(QA-QA)/AOGX(K)                       CHNL0481
      GO TO 2690                                       CHNL0482
  570   2680 BA=QCYP(K)                                CHNL0483
      KT=KRI(K)                                     CHNL0484
      QB= 0.                                         CHNL0485
      IF(KT.GT.0) QB= -HRO(KT)                     CHNL0486
      HC(K)=(QA-QB)/AOGY(K)                       CHNL0487
      C
      2690 IF(JCG(K).GT.0) GO TO 2695               CHNL0488
      IF(L.EQ.2) GO TO 2695                         CHNL0489
      L=2                                           CHNL0490
      GO TO 2695                                     CHNL0491

  580   C
      2695 QCXP(K)=QA                            CHNL0492
      QCYP(K)=QB                            CHNL0493
  
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SUBROUTINE CHANL 74/74 OPT=2 FTN 4.64420 08/22/77 16.51.06

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      2700 CONTINUE          CHNL0494
  585      C                   CHNL0495
      RETURN
      C
      C ENTRY POINT 3 FOR HEIGHT CALCULATIONS ON BLOCKS WITH CHANNELS
      C
  590      3000 DO 3050 K=1,KCM          CHNL0497
      I= JCG(K)          CHNL0498
      I= IABS(I)          CHNL0499
      J= JCG(K)          CHNL0500
      J= IABS(J)          CHNL0501
      IF(I.EQ.0,OR,J.EQ.JM) GO TO 3050          CHNL0502
      Z=IZ(I,J)          CHNL0503
      H(I,J)=HG(I)          CHNL0504
      IF(J.EQ.1) GO TO 3050          CHNL0505
      UT=UCT(K)
      VT=VCT(K)
      WX= ICX(K)
      WX= ABSF(WX)
      WY= ICY(K)
      WY= ABSF(WY)
      IF(WX.EQ.0.) UT=U(I+1,J)
      IF(WY.EQ.0.) VT=V(I+J+1)
      SETUP=DELT*((U(I,J)-UT)/(DELX-WX)+(V(I,J)-VT)/(DELX-WY))
      H(I,J)=HP(K)+SETUP+RAIN          CHNL0512
      IF(H(I,J).LE.Z) H(I,J)=Z          CHNL0514
  605      3050 HP(K)= H(I,J)          CHNL0515
      DO 3500 K=1,KCM          CHNL0516
      I= JCG(K)
      I= IABS(I)
      J= JCG(K)
      J= IABS(J)
      Z= IZ(I,J)
      IF(ICG(K).LT.0) GO TO 3100          CHNL0517
      GO TO 3500          CHNL0518
      C
      C BOUNDARY CONDITIONS FOR H END POINTS
      C IN THESE CALCULATIONS HC EQUALS THE H OF THE ADJOINING WATER BLOCK
      C HC AND D ARE SOLVED FROM SIMULTANEOUS EQUATIONS WHICH ALLOA FOR THE
      C VOLUME TRANSPORT TO OR FROM THE ADJOINING BLOCK VIA CHANNEL FLOW Q
      C
  620      3100 L=1          CHNL0519
      TCF=2.*C4          CHNL0520
      3105 KEY=KEY(L,K)          CHNL0521
      GO TO (3100,3120,3130,3140,3300,3300,3300,3300), KEY          CHNL0522
      3110 KS=KCX(K)
      BA=QCXN(K)
      IF(J.EQ.1) GO TO 3115          CHNL0524
      HA=H(I,J-1)-QCXN(KS)/TCF          CHNL0525
      DIV=1.0+AOGY(K)/TCF          CHNL0526
      QCXN(K)=(RAM+AOGX(K)+HAM)/DIV          CHNL0527
      HC(KS)=(HAM-BAM/TCF)/DIV          CHNL0528
      GO TO 3116          CHNL0529
  630      3115 HC(KS)=HG(I)          CHNL0530
      CHNL0531
  625
  635

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SUBROUTINE CHNL

74/74 CPT#2

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	QCXN(K)=RAM+AOGY(K)*HC(KS)	CHNL0532
	GO TO 3300	CHNL0533
640	H(I+J-1)=HC(KS)	CHNL0534
	QCYN(KS)=OCXN(K)	CHNL0535
	GO TU 3300	CHNL0536
	3120 K\$=QCY(K)	CHNL0537
	BAH=QCY(K)	CHNL0538
645	IF(I.EQ.1) GO TO 3125	CHNL0539
	HAM=H(I,J)=QCYN(KS)/TCF	CHNL0540
	DIV=1.0+AOGY(K)/TCF	CHNL0541
	QCYN(K)=(RAM+AOGY(K)*HAM)/DIV	CHNL0542
	HC(KS)=(HAM*RAM/TCF)/DIV	CHNL0543
	GO TO 3126	CHNL0544
650	HC(KS)=HG(1)	CHNL0545
	QCYN(K)=RAM+AOGY(K)*HC(KS)	CHNL0546
	GO TO 3300	CHNL0547
	3126 H(I+J)=HC(KS)	CHNL0548
	QCYN(KS)=QCYN(K)	CHNL0549
655	GO TU 3300	CHNL0550
	3130 KS=XCP(K)	CHNL0551
	BAH=WCXP(K)	CHNL0552
	VAR=0.5/C4	CHNL0553
660	IF(KS.GT.KCM) GO TO 3132	CHNL0554
	WC= I*CY(KS)	CHNL0555
	WC= ABSF(WC)	CHNL0556
	VAR=C3*0.5/(DEFLX-WC)	CHNL0557
665	HAM=(I+J+1)+QCXP(KS)*VAR	CHNL0558
	DIV=1.0+VAR*AOGX(K)	CHNL0559
	QCXP(K)=(RAM+HAM+AOGX(K))/DIV	CHNL0560
	HC(K)=(RAM*VAR+HAM)/DIV	CHNL0561
	H(I+J+1)=HC(K)	CHNL0562
	HP(KS)=HC(K)	CHNL0563
	QCXP(KS)=QCXP(K)	CHNL0564
670	GO TO 3300	CHNL0565
	3140 K\$=CYP(K)	CHNL0566
	BAH=QCYP(K)	CHNL0567
675	IF(I.EQ.IHM) GO TO 3145	CHNL0568
	VAR=0.5/C4	CHNL0569
	IF(KS.GT.KCM) GO TO 3142	CHNL0570
	WC= I*CX(K)	CHNL0571
	WC= ABSF(WC)	CHNL0572
	VAR=C3*0.5/(DEFLX-WC)	CHNL0573
680	HAM=(I+J)+QCYP(KS)*VAR	CHNL0574
	DIV=1.0+VAR*AOGY(K)	CHNL0575
	QCYP(K)=(RAM+HAM+AOGY(K))/DIV	CHNL0576
	HC(K)=(RAM*VAR+HAM)/DIV	CHNL0577
	GO TO 3146	CHNL0578
685	HC(K)=HG(1HM)	CHNL0579
	QCYP(K)=RAM+AOGY(K)*HC(K)	CHNL0580
	GO TO 3300	CHNL0581
	3146 H(I+J)=HC(K)	CHNL0582
	HP(KS)=HC(K)	CHNL0583
	QCYP(KS)=QCYP(K)	CHNL0584

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690      C
       3300 IF(JCG(K).GT.0) GO TO 3500
       IF(L.FG.2) GO TO 3500
       L=2
       GO TO 3105
695      C
       3500 CONTINUE
       C
       RETURN
       C
700      C ENTRY POINT 4 FOR LIST OF CHANNEL OUTPUT
       C
       4000 IMOUR=TIME/NF
       4010 FORMAT(1H1)
       PRINT 4020, IMOUR, NTIME
705      4020 FORMAT(10X,1CHANNEL OUTPUT FOR HOURS(+I3+40X*NTIME=1,15,//)
       1 20X,1ALL H VALUES IN FEET, ALL Q VALUES IN CFS(+//)
       PRINT 4030
       4030 FORMAT(7X,[K(+7X,[I(+7X,[J(+5X,[QXN(+5X[QXP(+6X,[HY(+5X,[QYN(+5X,[QYP(+6X,[HC(+5X,[QXT(+5X,[QXF(+5X,[QYT(+5X,[QYF(+/])
710      C
       IR=1
       IS=JCG(1)
       IS=IABS(IS)
       JS=JCG(1)
       JS=IABS(JS)
       DO 4100 K=1+KC
       KX=KCX(K)
       KY=KCY(K)
       GXT=UCT(K)*DELX
       QXF=UCF(K)*DELX
       QYT=VCT(K)*DELX
       QYF=VCF(K)*DELX
       IT=JCG(K)
       IT=IABS(IT)
       JT=JCG(K)
       JT=IABS(JT)
       IF((IT-TS)**2.EQ.1.AND.JT.EQ.JS) GO TO 4200
       IF((JT-TS)**2.EQ.1.AND.IT.EQ.IS) GO TO 4200
       PRINT 4050, IR
720      4100 IR=IR+1
       IS=IT
       JS=JT
       PRINT 4040, K,[JCG(K)+JCG(K)+MC(KX)+QCXN(K)+QCXN(K)+MC(KY)+]
       1 QCYN(K)+QCYP(K)+MC(K),GXT+QXF+QYT+QYF
       4040 FORMAT(3I4,F8.3+2F8.0,F8.3,2F8.0,F8.3+4F8.0)
       4050 FORMAT(1/+5X,1CHANNEL REACH(+I3,+)
       4100 CONTINUE
       C
       VOLUME COMPUTATION
       C
740      5000 VOL=0,
       C6=DELX**2

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SUBROUTINE CHANL 74/74 OPT#2 F7N 4.6+420 08/22/77 16.51.06

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JL= JRL
IF(JER.GT.JRL) JL=JRR
745 JL= JL+1
DO 5500 I=1,IMM
DO 5400 J=JL,JMM
Z=IZ(I,J)
HIJ=H(I,J)
IF(Z.GT.0) HIJ=HIJ-Z
IF(KCM.EQ.0) GO TO 5200
DO 5100 K=1,KCM
IC= ICG(K)
JC= JCG(K)
IF(IABS(IC).EQ.J.AND.IABS(JC).EQ.J) GO TO 5300
5100 CONTINUE
5200 VOL=VOL+HTJ*C6
GO TO 5400
5300 XX=INCX(X)
750 XX= ABSF(XX)
YY= INCY(Y)
YY= ABSF(YY)
KXXCX(X)
KYCY(Y)
VOL=VOL+HIJ*((DELX-XX)*(DELX-YY)+((HC(K)+HC(KX))*XX+
1 (HC(K)+HC(KY))*YY)*DELX/2.+HC(K)*XX*YY
5400 CONTINUE
5500 CONTINUE
VOL= VOL/1000000.
JL= JL+1
PRINT 5600, VOL, JL
5600 FORMAT(1 //, 10X,(VOLUME OF WATER ABOVE MSL #1, F12.1,
1 ( MILLIONS OF CU FT //, 10X,(THE SEAWARD ROADS THRU J=1, I3,
2 ( ARE EXCLUDED) //)
PRINT 4010
C
C IF(N. EQ. 4) RETURN
C
775 PRINT 5700
5700 FORMAT(1 //, ( PROBLEM TERMINATED BECAUSE A CHANNEL HAS GONE DRY(CHA
1 //, //)
STOP
C
END
CHNL0628
CHNL0629
CHNL0630
CHNL0631
CHNL0632
CHNL0633
CHNL0634
CHNL0635
CHNL0636
CHNL0637
CHNL0638
CHNL0639
CHNL0640
CHNL0641
CHNL0642
CHNL0643
CHNL0644
CHNL0645
CHNL0646
CHNL0647
CHNL0648
CHNL0649
CHNL0650
CHNL0651
CHNL0652
CHNL0653
CHNL0654
CHNL0655
CHNL0656
CHNL0657
CHNL0658
CHNL0659
CHNL0660
CHNL0661
CHNL0662
CHNL0663
CHNL0664
CHNL0665
CHNL0666

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SUBROUTINE SAVE

74/74 OPT=2

#T 4.6\*420

8/22/77 1A.51. 5

```

1      C
2      C
3      C      SUBROUTINE SAVE(JIN)                                SAVE0001
4      C
5      COMMON/PLK2/ IZ(28,20),U(28,20),V(28,20),ME28,20),NTIME   SAVE0004
6      COMMON/PLK3/ NW,MMIN,MMAX,NFU,INFLD,IMEM,KM,KMAX,LMAX,DELX,DELT   SAVE0005
7      1,CDD,FK,HRI,IOUT,KI,LJ,KII,LJJ,JBL,JBR,KMF,RF,CONST,S   SAVE0006
8      2,TRG,JMD,KR,ISTR,IND,NOW,KIM,NORT,NTIME,ZNTIT,E,NOWIND,GRAV   SAVE0007
9      3,KCMF,DFU,INTER                                         SAVE0008
10     COMMON/PLK5/ JCG(130),JCF(130),ICX(130),ICY(130),IZCX(130)   SAVE0009
11     1,IZCY(130),OCXP(130),OCXN(130),OCYP(130),OCY(130),HC(130),HP(130)SAVE0010
12     2,KCM,KCX(130),KCY(130),KCB(130),UCF(130),UCF(130),KRI(130),IBOM   SAVE0011
13     3,EN(2,130),VCY(130),VCF(130),ADGX(130),ADGY(130),KCXP(130)   SAVE0012
14     4,KCYP(130),KLB(50),KLM,IFC(130),FC                                         SAVE0013
15     COMMON/PLK8/ HS(9,72),QS(6,72),TIME(72)                   SAVE0014
16     COMMON/PLK9/ KZ,LZ,NUMRO,C1,C2,C3,IMM,JKH,NEXT1,IT,JC,IFIRST,SAVE0015
17     1,JIND,NEW1,XNOW,NEW3,XNORT,C4,PAIN,AJ,AI,LJK,KIK   SAVE0016
18     COMMON/PLK10/ NGAGE,NFLOW,IGAGE(12),JGAGE(12),KFLOW(6),XMIN,XMAX  SAVE0017
19
20     C      THIS ROUTINE SAVES WATER LEVELS AND FLOW RATES AT CERTAIN
21     C      KEY POINTS AS SPECIFIED IN INPUT BY USER. THE TIME SEQUENCES OF
22     C      THESE QUANTITIES ARE OUTPUTTED BY THE THIRD PART OF THIS ROUTINE.
23
24     GO TO(100,2000,3000), JIN
25     1000 READ 135, (JGAGE(K),JGAGE(K),K=1,NGAGE)           SAVE0018
26     135 FORMAT(20I4)                                         SAVE0019
27     PRINT 136                                         SAVE0020
28     136 FORMAT(1 I,3X,[HYDROGRAPH GAGE LOCATIONS])        SAVE0021
29     DO 100 K=1,NGAGE                                     SAVE0022
30     IE IGAGE(K)                                         SAVE0023
31     JE JGAGE(K)                                         SAVE0024
32     IF(J,NE,0) GO TO 105                               SAVE0025
33     PRINT 130, K,I                                      SAVE0026
34     130 FORMAT(5X,[GAGE I,I CHANNEL H], K=1,14)          SAVE0027
35     GO TO 100                                         SAVE0028
36     105 PRINT 137, K,I,J                                SAVE0029
37     137 FORMAT(5X,[GAGE I,I BLOCK H], I=1,13, J=1,13)    SAVE0030
38     100 CONTINUE                                         SAVE0031
39     PRINT 138                                         SAVE0032
40     138 FORMAT(1 I,3X,[KEY FLOW LOCATIONS])            SAVE0033
41
42     C      READ 135, (KFLOW(K), K=1,NFLOW)
43     PRINT 139, (KFLOW(K), K=1,NFLOW)                   SAVE0035
44     139 FORMAT(5X,[CHANNEL BLOCKS I,10I4,])
45     RETURN                                              SAVE0036
46
47     C      2000 T=NTIME-NTIME
48     MINTS INTER
49     NET/MINT + 1
50     TT=NTIME
51     TIME(N)=TT*DFLT/3600
52     DO 200 K=1,NGAGE
53     IE IGAGE(K)

```

SUBROUTINE SAVE 74/74 OPT#2 FTM 4.6+420 08/22/77 16.51.06

```

      J=JGAGE(K)
      IF (I.GT.KCM) GO TO 199
      HS(K,N)= HC(I)
199 IF(J.NE.0) HS(K,N)= H(I,J)
200 CONTINUF
C
60   DO 300 J=1,NFLDN
     K=NFLDN(J)
     KEY= KEY(1,K)
     IF(KEY.NE.0) GO TO 205
     KEY= 2
85   IF(I*CX(K).NE.0) KEY= 1
205 GO TO(210,220,230,240,210,220,230,240)*KEY
210 QS(J,N)= QCXN(K)/1000.
     GO TO 300
220 QS(J,N)= QCYN(K)/1000.
     GO TO 300
230 QS(J,N)= QCXP(K)/1000.
     GO TO 300
240 QS(J,N)= QCYP(K)/1000.
300 CONTINUE
75   NU=72
     IF(NU.EQ.72) GO TO 310
     RETURN
C
80   3000 NU = (NM-INTIME)/INTER
     IF(NU.EQ.0) RETURN
310 PRINT 400
400 FORMAT(20X, (WATER LEVEL HYDROGRAPHHS (FT) AND KEY FLOWS (1000 CFS)) (SAVE0072
1 //)
     PRINT 410, (J,J=1,NGAGE), (K,K=1,NFLDN)
410 FORMAT(2X, (HOUR), I6.15I8)
     DO 500 N=1,NU
     PRINT 420, TIME(N), (HS(J,N),J=1,NGAGE), (QS(K,N),K=1,NFLDN)
420 FORMAT(F6.1+15FA,2)
500 CONTINUE
     DO 510 N=1,NU
430 FORMAT(F6.1+10FA,2)
510 CONTINUE
     PRINT 10
10  FORMAT(1H1)
     IF(NU.EQ.72) INTIME=NTIME
     RETURN
100  C
     END
C
1   C
     SUBROUTINE CONTIN(L)                               CONT0001
C
5   COMMON//BLK1/A(822)/BLK2/B(1961)/BLK3/C(42)/BLK4/D(2585)/
     1BLK5/E(2759)/BLK6/F(2800)/BLK7/G(44)/BLK9/H(25)/BLK10/P(34)  CONT0002
     C
     GO TO (100,200)+L                               CONT0003
C
10  100 C
     CONTINUE
     70C FORMAT(20A4)
     RETURN
C
15  200 CONTINUE
     RETURN
C
     END                                              CONT0009
C
                                         CONT0010
  
```

```

SUBROUTINE PLOT      74/74   OPT#2      FTN 4.6+420      08/22/77  16.51.06
1      C
2      C
3      C
4      C
5      C      SUBROUTINE PLOT          PLOT0005
6      C
7      C      PROGRAM TO PLOT CHANNELS AND BARRIERS
8      C
9      C
10     COMMON/BLK1/ ITR(100),JB(100),IZX(100),IZY(100),ICDOX(100)
11     1,ICDOY(100),ICDSX(100),ICDSY(100),LR01(B),LR02(B),DIST(24)    PLUT0010
12     2,CHST(30),R0C(8,30),HGR(8),XP(8,6),YP(8,6),HAR(8)           PLUT0015
13     COMMUN/BLK2/ TZ(28,20)*U(28,20),V(28,20),H(28,20),NTIME       PLUT0020
14     COMMON/BLK5/ ICG(130),JCG(130),I+CX(130),I-CY(130),IZCX(130)  PLCT0030
15     1,IZCY(130),GCXP(130),GCXY(130),GCYP(130),GCVN(130),HC(130),H(130) PLCT0035
16     2,KCM(KCX(130),KCY(130),KCB(130),UCY(130),UCF(130),KPI(130),T30M  PLCT0040
17     3,KEN(2,130),VCT(130),VCF(130),ANGX(130),ANGY(130),KCXP(130)  PLUT0045
18     4,KCYF(130),KLR(50),KLM                         PLUT0050
19     COMMON/BLK10/ ,GAGE, KFLGX,IGAGE(12),JGAGE(12),KFLGX(e),XMIN,XMAX  YAIN0110
20     DIMENSION NUMBER(10),PAGE(114,135)
21     LOGICAL XPRR,YPRR,X,ELNK,NUMBER,O-E,PAGE,VLINE,HLINE,PLUS,PERIOD
22     BLANK
23     DATA BLANK/!/,*X/{X}/,BLNK/!/,{/,NUMBER/{0!,1!,2!,3!,4!,5!,6!,7!,8!,9!},ONE/{1!/},VLINE/{1!/},HLINE/{-1!/},PLUS/{+1!/},PERIOD/{2!/}/
24
25     C
26     DO 100 I=1,15390
27     PAGE(I,:)=BLANK
28     100 CONTINUE
29
30     C
31     DO 101 J=6,114,4
32     PAGE(I,3)=PERIOD
33     PAGE(I,134)=PERIOD
34     DO 101 J=9,135,7
35     101 PAGE(I,J)=PERIOD
36
37     C      DRAW THE BARRIERS
38
39     DO 800 KI=1,KCM
40     K = KCNA+KI
41     I = IA8S(TCG(K))*4+1
42     J = IA8S(JCG(K))*7+4
43     I4 = I+3
44     J4 = J+6
45     IF ( KCR(K) .EQ. 0 ) GO TO 800
46
47     C      TEST FOR BARRIER IN X DIRECTION, Y DIRECTION, OR BOTH
48
49     KB = KCR(K)
50     II = IR(KP)
51     JJ = JR(KP)
52     IZI = IZ(TI,JJ)*10

```

SUBROUTINE PLOT      74/74      OPT#2      FTM 4.6+420      08/22/77 16.51.06

```

      IZ2 = IZ(TI+1, JJ)*10
      IZ6 = IZx(KR)
      XBARR = .TRUE.
      IF ( IZB .EQ. IZ1 .OR. IZB .EQ. IZ2 ) XBARR = .FALSE.
      IZ2 = IZ(I, JJ+1)*10
      IZ6 = IZY(KR)
      YBARR = .TRUE.
      IF ( IZB .EQ. IZ1 .OR. IZB .EQ. IZ2 ) YBARR = .FALSE.
      C      IF (.NOT. XBARR) GO TO 250
      C      X BARRIERS
      C      IF ( IXCX(K) .LT. 0 ) GO TO 230
      C      IF ( IZCX(K) .LE. 0 ) GO TO 231
      C      OUTER BARRIER
      DO 202 L=1,10
      202 PAGE(I+4,J+L-3) = X
      GO TO 250
      C      INNER BARRIER
      231 DO 203 L=1,10
      203 PAGE(I+2,J+L-3) = X
      GO TO 250
      C      BOTH BARRIERS
      230 DO 204 L=1,10
      204 PAGE(I+2,J+L-3) = X
      204 PAGE(I+4,J+L-3) = X
      C      Y BARRIERS
      C      250 IF (.NOT. YBARR) GO TO 200
      C      IF ( IXCY(K) .LT. 0 ) GO TO 240
      C      IF ( IZCY(K) .LE. 0 ) GO TO 241
      C      OUTER BARRIER
      DO 205 L=1,5
      205 PAGE(I+L-2,J+8) = X
      GO TO 800
      C      INNER BARRIER
      241 DO 206 L=1,5
      206 PAGE(I+L-2,J+4) = X
      GO TO 800
      C      BOTH BARRIERS
      240 DO 207 L=1,5
      207 PAGE(I+L-2,J+8) = X
      207 PAGE(I+L-2,J+4) = X
      800 CONTINUE
      C
  
```

PLOT0185  
PLOT0190  
PLOT0195  
PLOT0200  
PLOT0205  
PLOT0210  
PLOT0215  
PLOT0220  
PLOT0225  
PLOT0230  
PLOT0235  
PLOT0240  
PLOT0245  
PLOT0250  
PLOT0255  
PLOT0260  
PLOT0265  
PLOT0270  
PLOT0275  
PLOT0280  
PLOT0285  
PLOT0290  
PLOT0295  
PLOT0300  
PLOT0305  
PLOT0310  
PLOT0315  
PLOT0320  
PLOT0325  
PLOT0330  
PLOT0335  
PLOT0340  
PLOT0345

SUBROUTINE PLOT      74/74      OPT#2      FTR 4.0+420      08/22/77 16.51.06

```

C      LAND BARRIERS
C
110     DO 804 K=1,KLM      PL0T0350
C      TEST FOR BARRIER IN X DIRECTION, Y DIRECTION, OR BOTH
C
115     KB = KLB(K)
116     II = IZ(KB)
117     JJ = JZ(KB)
118     I = IABS(II)*4=1      PL0T0355
119     J = IABS(JJ)*7=4      PL0T0360
120     IZ1 = IZ(II,JJ)*10      PL0T0365
121     IZ2 = IZ(II+1,JJ)*10      PL0T0370
122     IZB = IZX(KB)      PL0T0375
123     XBARR = .TRUE.
124     IF ( IZB .EQ. IZ1 ,OR. IZB .EQ. IZ2 ) XBARR = .FALSE.
125     IZB = IZY(KB)
126     YBARR = .TRUE.
127     IF ( IZB .EQ. IZ1 ,OR. IZB .EQ. IZ2 ) YBARR = .FALSE.
C      X BARRIER
C
130     IF ( .NOT. XBARR ) GO TO 220      PL0T0425
131     DO 208 L=1,7      PL0T0430
132     PAGE(I+L,J+L-3) = X      PL0T0435
C      Y BARRIER
C
135     220 IF ( .NOT. YBARR ) GO TO 804      PL0T0440
136     DO 209 L=1,5      PL0T0445
137     PAGE(I+L-3,J+4) = X      PL0T0450
138     804 CONTINUE      PL0T0455
C      DRAW CHANNELS
C
140     251 DO 802 K=1,KCM      PL0T0460
141     I = IABS(TCG(K))*4=1      PL0T0465
142     J = IABS(JCG(K))*7=4      PL0T0470
143     I4 = I+3      PL0T0475
144     J4 = J+6
145     IF ( IWCX(K) .EQ. 0 ) GO TO 300      PL0T0480
146     DO 200 L=1,7      PL0T0485
147     200 PAGE(I4,J+L-1) = HLINE      PL0T0490
148     IF ( KCX(K) .GT. KCM ) PAGE(I4,J) = PLUS      PL0T0495
149     300 IF ( IWCY(K) .EQ. 0 ) GO TO 301      PL0T0500
150     DO 201 L=1,3      PL0T0505
151     PAGE(I+L-1,J+5) = BLNK      PL0T0510
152     PAGE(I+L-1,J+7) = BLNK      PL0T0515
153     201 PAGE(I+L-1,J4) = VLINE      PL0T0520
154     IF ( KCY(K) .GT. KCM ) PAGE(I,J4) = PLUS      PL0T0525
155     301 PAGE(I4,J4) = PLUS      PL0T0530
156     802 CONTINUE      PL0T0535
157     PL0T0540
C      WRITE OUT THE PAGE
C
158     4PITE(6+501)(J,J=1,19),((PAGE(4*K-1,J),J=3,134),K,(PAGE(4*K-1,J),J,PL0T0545
159     1=5,134),((PAGE(4*K+1,J),J=3,134),I=1,2),K=1,28)      PL0T0550
160     501 FORMAT(11(1B(74+3X),14,/,11(5X,17(11,16X),11,15X,11,11//+28(1X,132A1+/,2(1X,132A1+/,11))
161     1 132A1,/,1X,12+130A1+/,2(1X,132A1+/,11)) )
162     RETURN      PL0T0560
163     END      PL0T0565
164     PL0T0570

```

## APPENDIX B

### DESCRIPTION OF THE SURGE II CODED PROGRAM

The general strategy of the program is discussed and certain special features are pointed out which may not be apparent without detailed study of the program. Operational aspects of the program are discussed in some detail in Appendix C.

The version of the program adapted for use on the GE 400 computer system by the Corps of Engineers consists of the following parts or subroutines:

- MAIN whose primary job is to read and check the sequencing of the basic data for the block computations;
- PART 2 which controls the basic computational sequencing, initialization, and updating of storage, interpolation of coarse wind fields for the actual grid, and routine computation of U, V, and H for all blocks, considering barriers (basically, the SURGE I program);
- CHANL(1) which is called only once to read channel data and to establish certain key arrays for routine calculation;
- CHANL(2) which is called routinely to compute flow and water levels in channels and at channel end points;
- CHANL(3) whose task is the routine calculation of H on blocks containing channels;
- CHANL(4) which is called for listing of channel computations;
- LIST(1) which is called only once to read control data for block listings and to list the topographic Z field;
- LIST(2) which lists the H field for blocks if called;
- LIST(3) which lists the U, V, and H fields for blocks if called in place of LIST(2);
- SAVE(1) which is called only once to read the positions of certain gage locations for water level or flow;
- SAVE(2) which is called routinely at preselected time intervals to save water levels and flow for gage locations defined by SAVE(1);
- CONTIN(1) which is called only once to read basic storage in COMMON BLOCKS 1 to 10 in the case of a continuation of a given problem;
- CONTIN(2) which is called at the termination of a run to output the continuation data called for by CONTIN(1).

The version of the program used in the testing and calibration work, using an IBM 360/65 computer system, has an additional assembler language subroutine for plotting positions of barriers and channels (see Fig. 15). This is useful in checking input data for channels and barriers to spot possible errors in coding the positions of channel blocks and barrier blocks. Unfortunately, this subroutine is not compatible with the GE 400 system. Subroutine PLOT in Appendix A however can be used for this purpose. Subroutine LIST is not used in the version of the program in Appendix A.

### 1. Flow Diagram.

A schematic flow diagram for the SURGE II program is given in Figure B-1. If a new problem is being run then the first phase is reading in the basic data and checking the data sequencing to make sure it is in order and complete. This is carried out in MAIN and the beginning of PART 2 which calls subroutines CHANL(1), SAVE(1), and LIST(1).

Initialization of block arrays is carried out in PART 2; initialization of channel arrays and establishing of key arrays are carried out by CHANL(1). These key arrays are discussed in a subsequent subsection.

Step 4 of the flow diagram is the beginning (or reentry point) of the routine computations for each time. After generating, the detailed interpolated fields of  $x$  and  $y$  components of wind stress for the blocks (step 4) and all blocks (i.e., all  $I,J$ ) are swept to compute the flow components,  $U$  and  $V$ , ignoring at first the presence (if any) of subgrid scale channels, but considering barriers for any barrier blocks (step 5).

In step 6 CHANL(2) is called to sweep through all channel blocks to evaluate all channels  $Q$  and  $H$  except those for  $H$ -end points and all lateral flows to and from channels. In the latter operation, the flows  $U$  and  $V$  computed in step 5 are replaced by corrected  $U$  or  $V$  between blocks, considering the presence of the channels.

Step 7, which is carried out in PART 2, sweeps all  $I,J$  to compute water levels on blocks ignoring for the present, the presence of any subgrid scale channels.

In step 8, CHANL(3) is called to correct the block  $H$  values on those blocks containing channels and to compute the  $H$  and  $Q$  values at  $H$ -end points of channels. This also provides corrected  $H$  values for those blocks into which the channels discharge.

Steps 10 and 11 are output operations for block and channel computations carried out in PART 2 and CHANL(4). This is followed by a time updating and test for end, dependent upon a prescribed maximum number of time steps. Before termination of a run, the contents of all data in COMMON are saved for possible continuation of the problem, if desired.

### 2. Identification of Adjacent Channel Blocks.

To provide rapid access to values of  $H$  and  $Q$  in channels adjoining a given channel reach, special arrays are generated in subroutine

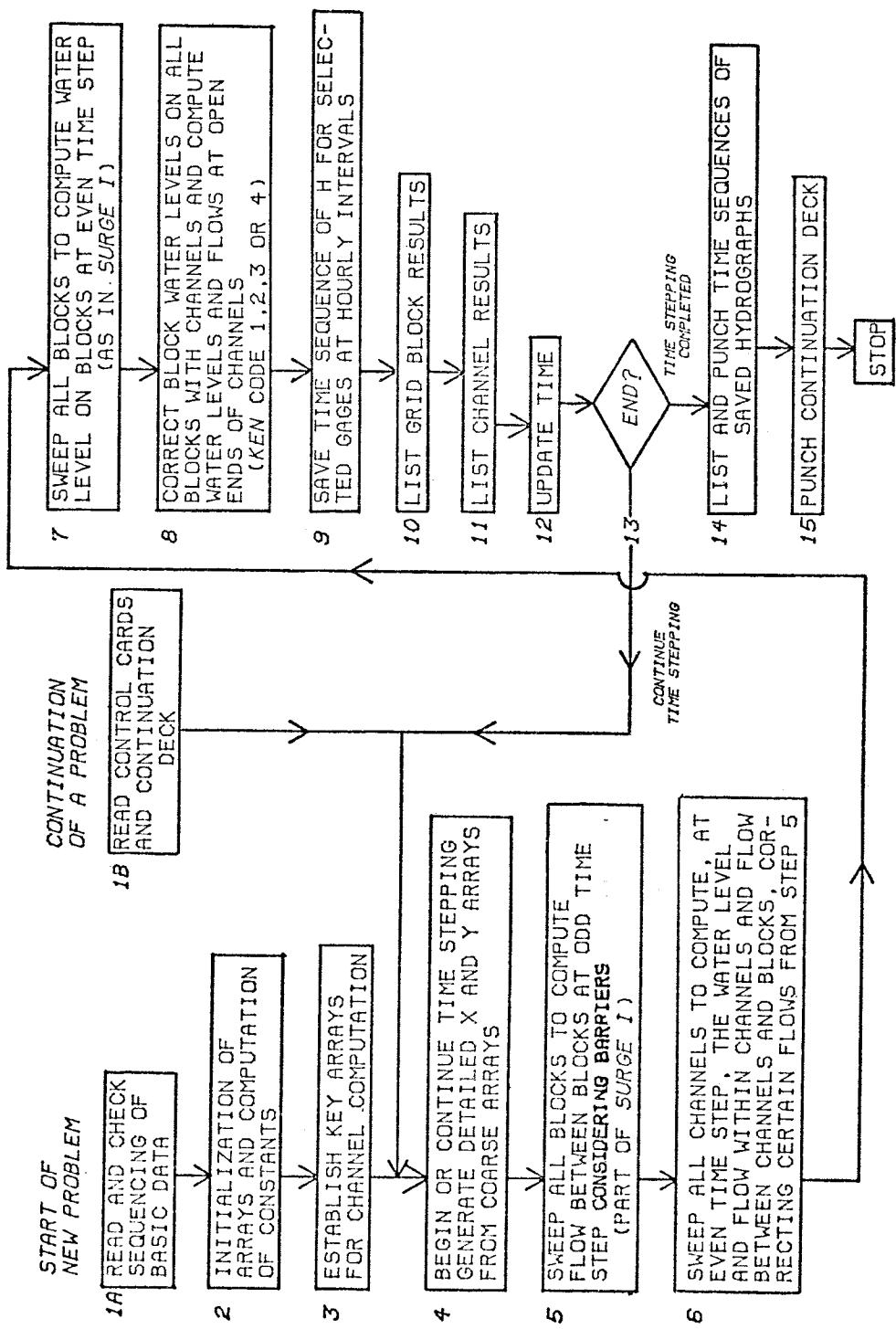


Figure B-1. Generalized flow diagram for SURGE II.

CHANL(1). There are four such arrays: KCX(K), KCY(K), KCXP(K), and KCYP(K). These give the channel block identification index for those channel blocks which are adjacent to the Kth channel block as indicated in Figure B-2. Thus, KCX(K) is the identification of the channel block which has an x-side channel adjoining channel block K on the negative characteristic side (i.e., on a preceding row), while KCXP(K) is the identification of the channel block which has an x-side channel adjoining channel block K on the positive side (i.e., on a following row). KCY(K) and KCYP(K) have analogous meanings for blocks with y-side channels adjoining that of block K. These arrays are generated by an appropriate series of tests in which the I,J values of blocks adjacent to that of channel block K are compared with the ICG and JCG values of all other channel blocks. This is carried out only once during any run, and is not particularly time consuming; moreover, it avoids any human error which may easily occur if such arrays were required as input.

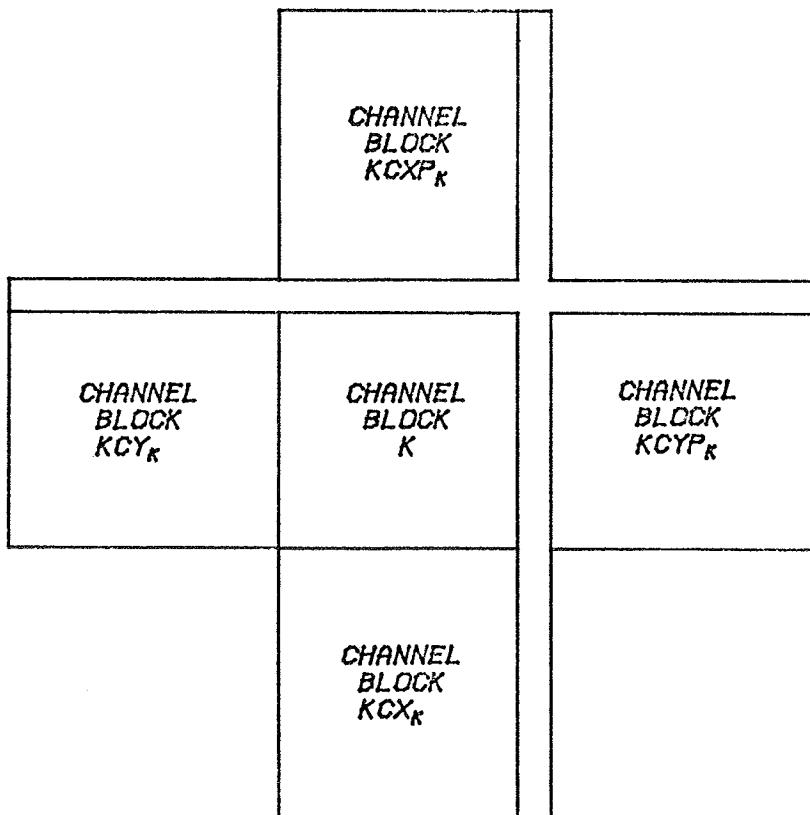


Figure B-2. Channel block identification for channels adjacent to those of block K.

The arrays KCX and KCXP have the properties  $KCXP(KCX(K)) = K$  and  $KCX(KCXP(K)) = K$  with similar relations for KCY and KCYP.

As an example of the use of such arrays, suppose the value of HC in an x channel adjoining that of channel block K is needed. This could

be addressed as  $HC(KX)$  where  $KX = KXC(K)$ . Using Figure 8 as an example, the values of channel flow entering the junction from channels 1 and 2 would be addressed by  $QCXP(K1)$  and  $QCYP(K1)$ , respectively, where  $K1$  designates the channel block containing channels 1 and 2. However, the flow leaving the junction would be addressed by  $QCYN(K2)$  where  $K2 = KCYP(K1)$  and  $QCXN(K3)$  where  $K3 = KCXP(K1)$ . While redundant storage of such  $H$  and  $Q$  values would also satisfy the requirement of rapid access to such values adjoining a given channel block, the use of the integral arrays  $KCX$ ,  $KCY$ ,  $KCXP$ , and  $KCYP$  saves storage for most computer systems.

An examination of the listings of the values of the arrays  $KCX$ ,  $KCY$ ,  $KCXP$ , and  $KCYP$ , as output by the program, indicates that the maximum value of any of these can and usually does exceed the number of input channel blocks ( $KCM$ ). The reason for this is that dummy storage positions are created for blocks adjoining channel end points. This is an artifice of the program which allows routine computation for all channel reaches before special computation for channel end points.

### 3. Barrier Identification.

The position of the  $K$ th barrier block is given by the array pair,  $IB(K)$  and  $JB(K)$ , which is input to the program. It is convenient to have rapid access to barrier information for those barriers which happen to fall on a given channel block. The array  $KCB(K)$  gives the identification of the barrier block which coincides with channel block  $K$ . Thus,  $ICG(K) = IB(KCB(K))$  and  $JCG(K) = JB(KCB(K))$ . If no barriers exist in a given channel block then the corresponding value of  $KCB$  is zero. Thus, in the routine program, a test for zero value  $KCB$  is made; if nonzero, then a call can be made for barrier data such as elevation and barrier coefficients via the barrier index  $KB = KCB(KC)$  where  $KC$  is the channel block concerned.

The array  $KCB(K)$  is generated in  $CHANL(1)$ , via a scan of all  $IB$  and  $JB$  values for given  $ICG$  and  $JCG$  for channel block  $K$ .

An array  $KLB(K)$  is also generated which identifies those barrier blocks not common to channel blocks. This is used only in the IBM 360/65 assembler language plotting routine, not in routine calculations.

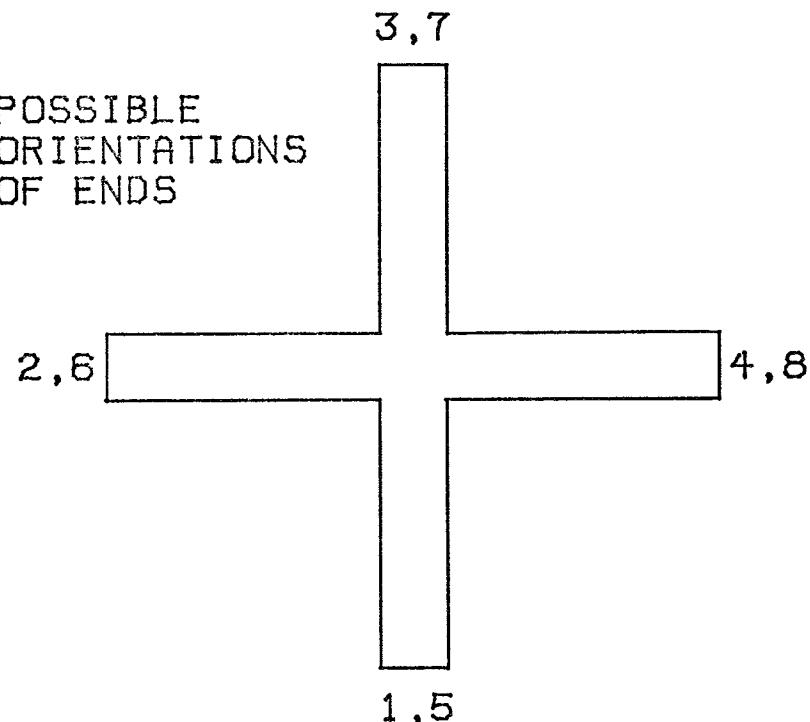
### 4. Channel End-Point Identification.

As a signal that at least one channel end point occurs in a channel block  $K$ , the value of  $ICG(K)$  is negative. If two end points occur, the value of  $JCG(K)$  is also negative; otherwise, it is positive. If no channel end point occurs, then both  $ICG$  and  $JCG$  for the block are positive. This positive-negative coding is generated automatically in  $CHANL(1)$  by appropriate testing; namely, to check if a valid channel connects at each end of a valid channel in the block concerned.

In addition, the arrays  $KEN(1,K)$  and  $KEN(2,K)$  are generated in  $CHANL(1)$  to identify the type of end point for, at most, two potential

channel terminations in given channel block K. If there is no channel termination both KEN(1,K) and KEN(2,K) are zero; if one termination occurs for block K, KEN(1,K) will have an integral value from 1 to 8 and KEN(2,K) will be zero; if two terminations occur, both KEN arrays will have nonzero value. In use, KEN(2,K) is called only if JCG(K) is negative.

The coding for the type of end point is indicated schematically in Figure B-3. Values of KEN from 1 to 4 represent "H-end" type terminations where a ponding block immediately adjoins the channel end. Values of KEN from 5 to 8 are those for which Q is specified; e.g., river discharge. Values within either group indicate the relative orientation of the channel end point in question to assure calling the correct data and using the right signs in the routine calculations.



<u>TYPE OF END</u>	
$KEN_K = 1,2,3,4$	HC=H OF ADJACENT BLOCK
$KEN_K = 5,6,7,8$	Q SPECIFIED

Figure B-3. Identification of type and orientation of a channel end point by the coded identifier KEN(K).

## APPENDIX C

### USER'S GUIDE TO SURGE II

The coded program SURGE II is intended for use in the numerical simulation of storm surges or astronomical tides in bays and estuaries for specified time sequences of water level at the seaward boundary of the bay or estuary and specified wind stress and other storm data over the bay or estuary. The user may use one of two distinct modes of operation: (a) the storm mode, in which all storm data are required as well as seaward hydrograph data; or (b) the tide mode, in which no storm data are required, the only forcing being the input water level variation at the seaward boundary. Moreover, in both modes the user has the option of initiating a new simulation or continuing a previous simulation, the input requirements being different for each.

In general, the input consists of the following types of information:

- (a) Control Data--For input-output operations, initialization, array size, time stepping, and run duration.
- (b) Bay Schematization Data--including block topography, barrier data, and channel data.
- (c) Forcing Data--including sequences of water level at seaward boundary, wind-stress components over bay, rainfall data over bay, and river discharge data.
- (d) Problem Specification Information.

Certain checks are made as the data are read in, with regard to proper order of input, proper amount of sequential data, and proper size arrays. All stops resulting from these editing checks of input are identified.

In the subsequent subsections, the individual input parameters are identified (with appropriate units), the sequence of data input for the different modes of operation is given in some detail, and special requirements concerning data input for barriers and channels are discussed, followed by a summary of output information and output options.

#### 1. Definition of Input Variables.

The following variables are listed in the order in which they are input (asterisks separate data blocks):

ICARD      Control index: 0 for starting, 1 for continuation.

\*\*\*\*\*      *Block 0*

IDENT      Data block identification;

IBL starting column (I value) for listing of block H output  
 (normally taken as 1);  
  
 KCM total number of blocks with channels (including null channels,  
 see subsec. 6 of this app.);  
  
 NOWIND control for storm data input: 0 for normal input operation for  
 wind stress, rainfall, and runoff; -1 for omitting such input  
 for tide computations;  
  
 INTER interval in SAVE operation (time interval is INTER\*DELT);  
  
 NGAGE number of H gage locations saved;  
  
 NFLOW number of Q gage locations saved;  
  
 IMIN minimum expected H (feet);  
  
 IMAX maximum expected H (feet).

NOTE---IMIN and IMAX are used only in subroutine GRAF, applicable to  
 IBM 360 or 370.

\*\*\*\*\* *Block 1*

NTIME Initial time level (normally 0, unless a continuation run is  
 being carried out, in which case NTIME should equal the final  
 value of the previous run);  
  
 NM maximum number of time steps for the problem;  
  
 MMIN minimum "map time" for wind-stress input;  
  
 MMAX maximum map time for wind-stress input;  
  
 NFU number of iterations per map time interval;  
  
 IOUT interval for routine output from blocks and channels equals  
 IOUT + 1;  
  
 INFLD special output flag: 0 for standard output, 1 for extra  
 listing of channel output for one iteration preceding normal  
 listing.

\*\*\*\*\* *Block 2*

IM Total number of x-grid intervals;  
  
 JM total number of y-grid intervals;  
  
 KM total number of blocks having barriers;

KMAX total number of coarse x-grid points for wind-stress input;  
 LMAX total number of coarse y-grid points for wind-stress input.  
 \*\*\*\*\* *Block 3*  
 DELX Spatial grid interval or block size (nautical miles);  
 DELT time interval between block H and flow computations (seconds);  
 CDO overflow coefficient for natural low-lying ground such as barrier islands;  
 FK bed-resistance coefficient for blocks;  
 FC bed-resistance coefficient for channels (used only if values for individual channels are not entered);  
 HGI initial water level above MSL in the bay (feet).  
 \*\*\*\*\* *Block 4*  
 KI Number of interpolation subdivisions of each coarse x-grid interval  $KI * (KMAX - 1) = IM$ ;  
 LJ number of interpolation subdivisions of each coarse y-grid interval  $LJ * (LMAX - 1) = JM$ ;  
 KII number of coarse x-grid intervals;  
 LJJ number of coarse y-grid intervals;  
 JBL, JBR number of "open boundary" J-intervals on left and right of system (not used in version in App. A).  
 \*\*\*\*\* *Block 5*  
 IB(K) I location index for barrier block K;  
 JB(K) J location index for barrier block K;  
 IZX(K) elevation of x-barrier (right side) on barrier block K (tenths of feet);  
 IZY(K) elevation of y-barrier (upper side) on barrier block K (tenths of feet);  
 ICDOX(K) overflow coefficient for x-barrier (value  $\times 1,000$ ) on Kth barrier block;

ICDOY(K) overflow coefficient for y-barrier (value  $\times$  1,000) on Kth barrier block;

ICDSX(K) submerged wier coefficient for x-barrier (value  $\times$  1,000) on Kth barrier block;

ICDSY(K) submerged wier coefficient for y-barrier (value  $\times$  1,000) on Kth barrier block.

\*\*\*\*\* *Block 6*

IZ(I,J) Elevation of ground or seabed (feet) relative to MSL datum for block location I,J.

\*\*\*\*\* *Block 7*

IMRO Number of river input (runoff) locations;

JMRO number of map times with runoff values;

KR number of channel-stress values (normally same as JMRO);

ISTR start of rain (map time);

IND end of rain (map time);

NOW number of iterations between river input values (normally same as NFU);

KIM number of iterations between channel-stress values (normally same as NFU);

NORT number of iterations per hour for rain (normally same as INTER).

\*\*\*\*\* *Block 8*

RF Total rainfall (inches);

CONST fraction of rainfall not absorbed by ground;

S conversion factor for wind stress  $(5,280/3,600)^2 \times 1.1/10.$

\*\*\*\*\* *Block 9*

LROI(K) I location index for Kth river input block;

LROJ(K) J location index for Kth river input block.

*****	<i>Block 10</i>
DIST(M)	Percent of total rainfall per hour for 24 hours.
*****	<i>Block 11</i>
CHST(M)	Channel-stress values at map time M (entries are used only if KCM = 0).
*****	<i>Block 12</i>
RO(K,M)	Discharge (cubic feet per second) from Kth river input block at map time M.
*****	<i>Block 13</i>
MTIME	Map time for given block of wind-stress input and seaward water level.
*****	<i>Block 14</i>
HGR(K)	Seaward water level above MSL (feet) at MTIME for coarse grid position K.
*****	<i>Block 15</i>
HBR(J)	Water level on right open boundary above MSL (feet) at MTIME for grid position J (not used in version in App. A).
*****	<i>Block 16</i>
XR(K,L)	Wind-stress component in the x direction (units of (miles per hour) <sup>2</sup> /10) for coarse grid position K,L at time MTIME.
*****	<i>Block 17</i>
YR(K,L)	Wind-stress component in the y direction (units of (miles per hour) <sup>2</sup> /10) for coarse grid position K,L at time MTIME.
*****	<i>Block 18</i>
ICG(K)	I location index for channel block K;
JCG(K)	J location index for channel block K;
IWCX(K)	width of x channel (right side) on channel block K (feet), with sign (see subsec. 6 of this app.);
IZCX(K)	depth of x channel bed on channel block K (feet), with sign (see subsec. 6 of this app.);

IWCY(K) width of y channel (upper side) on channel block K (feet),  
 with sign (see subsec. 6 of this app.);  
  
 IZCY(K) depth of y channel bed on channel block K (feet), with sign  
 (see subsec. 6 of this app.);  
  
 IFC(K) bed-resistance coefficient for channels on block K (value ×  
 10,000), if entry is zero (blank) then IFC is taken as FC  
 (entered in *Block 3*) × 10,000.

\*\*\*\*\* *Block 19*

IGAGE(K) Location index for the Kth hydrograph, if JGAGE(K) ≠ 0 then  
 IGAGE(K) is the I location of a block H; if JGAGE(K) = 0  
 then IGAGE(K) is the channel block index for a channel H;  
  
 JGAGE(K) if not zero, this is the J location of a block H; if zero,  
 a channel H is indicated;  
  
 KFLOW(K) channel block index for the Kth flow gage, the flow being  
 that of the lower end of the x channel, or the left end of a  
 y channel if an x channel does not exist, or a channel end  
 point if one exists in the identified channel block.

\*\*\*\*\* *Block 20*

IEND Maximum I in listing of block arrays of H, U, and V;  
  
 NF number of iterations between listings;  
  
 IBEGIN first I in listing of block arrays;  
  
 NJ maximum J in listing of block arrays;  
  
 NCARD total number of alphanumeric problem identification cards;  
  
 ALPHA(J) alphanumeric character data which identify the problem and  
 gage locations by name.

2. Input for Initiating Storm Surge Simulation.

The sequence of input for starting a problem in the storm surge mode is given below in the form of a summary of the READ statements active in this mode, together with a summary of the appropriate FORMATS for data input in different blocks. For all data blocks requiring an entry of the identification integer IDENT, only the *units digit* of the data block number is entered in column 1 of the data input card.

*Control Card*

READ 1 , ICARD (0 for starting)

*Block 0 (1 card)*

READ 1 , IDENT, IBL, KCM, NOWIND, INTER, NGAGE, NFLOW, IMIN, IMAX

NOTE-----IMIN and IMAX are left blank unless subroutine GRAF is used.

*Block 1 (1 card)*

READ 100 , IDENT, NTIME, NM, MMIN, MMAX, NFU, IOUT, INFLD

*Block 2 (1 card)*

READ 100 , IDENT, IM, JM, KM, KMAX, LMAX

*Block 3 (1 card)*

READ 250 , IDENT, DELX, DELT, CDO, FK, FC, HGI

*Block 4 (1 card)*

READ 100 , IDENT, KI, LJ, KII, LJJ, JBL, JBR

*Block 5 (total of KM cards of barrier data)*

DO 500 K = 1, KM

READ 100 , IDENT, IB(J), JB(K), IZX(K), IZY(K), ICDOX(K), ICDOY(K),  
ICDSX(K), ICDSY(K)

500 CONTINUE

*Block 6 (total of 2\*IM cards of block topography)*

DO 550 I = 1, IM

READ 100 , IDENT, (IZ(I,J), J = 1,10)

READ 100 , IDENT, (IZ(I,J), J = 11, JM)

550 CONTINUE

*Block 7* (1 card)

```
READ 100 , IDENT, IMRO, JMRO, KR, ISTR, IND, NOW, KIM, NORT
```

*Block 8* (1 card)

```
READ 250 , IDENT, RF, CONST, S
```

*Block 9* (1 or 2 cards, dependent on IMRO)

```
READ 100 , IDENT, (LROI(K), LROJ(K), K = 1,5)
```

```
IF (IMRO.LT.6) GO TO 575
```

```
READ 100 , IDENT, (LROI(K), LROJ(K), K = 6, IMRO)
```

```
575      CONTINUE
```

*Block 10* (3 cards)

```
READ 250 , IDENT, (DIST(M), M = 1,10)
```

```
READ 250 , IDENT, (DIST(M), M = 11,20)
```

```
READ 250 , IDENT, (DIST(M), M = 21,24)
```

*Block 11* (L + 1 card where L = KR/10. If KR = 0, block 11 input is omitted.)

```
READ 250 , IDENT, (CHST(K), K = 1,11)
```

```
READ 250 , IDENT, (CHST(K), K = 11,20)
```

```
...
```

```
READ 250 , IDENT, (CHST(K), K = KL, KR (KL = 10 * L + 1))
```

*Block 12* (JMRO cards of river discharge data)

```
DO 700      M = 1, JMRO
```

```
READ 250 , IDENT, (RO(K,M), K = 1, IMRO)
```

```
700      CONTINUE
```

*Wind Stress and Water Level Forcing*

(MTL sets of blocks 13 to 17 where MTL = MMAX - MMIN + 1)

```
710      CONTINUE
```

*Block 13* (1 card)

```
READ 100 , IDENT, MTIME
```

*Block 14* (1 card)

```
READ 250 , IDENT, (HGR(K), K = 1, KMAX)
```

*Block 15* (1 card)

```
READ 250 , IDENT, (HBR(J), J = 2,8)
```

*Block 16* (KMAX cards)

```
DO 790 K = 1, KMAX
```

```
READ 250 , IDENT, (XR(K,L), L = 1, LMAX)
```

```
790 CONTINUE
```

*Block 17* (KMAX cards)

```
DO 800 K = 1, KMAX
```

```
READ 250 , IDENT, (YR(K,L), L = 1, LMAX)
```

```
800 CONTINUE
```

```
IF (MTIME - MMAX) 710, 1,015, 1,015 (710 returns to read block 13)
```

```
1,015 (CONTINUE)
```

*Block 18* (KCM cards with channel data. If KCM = 0, the READ statement is bypassed and block 18 should be omitted.)

```
IF (KCM.GT.0) CALL CHANL(1)
```

```
DO 50 K = 1, KCM
```

```
READ 100 , IDENT, ICG(K), JCG(K), IWGX(K), IZCX(K), IWGY(K), IZY(K),  
IFC(K)
```

```
50 CONTINUE
```

*Block 19* (2 cards)

```
CALL SAVE(1)

READ 350 , (IGAGE(K), JGAGE(K), K = 1, NGAGE)
READ 350 , (KFLOW(K), K = 1, NFLOW)
```

*Block 20* (NCARD + 1 card)

```
CALL LIST(1)

READ 1 , IDENT, IEND, NF, IBEGIN, NJ, NCARD
DO    250   J = 1, NCARD
READ 450 , (ALPHA(J), J = 1,40)
250   CONTINUE
```

Format Statements for Input. The following formats were used in all the testing operations. It is recommended, however, that for routine operations those READ statements using FORMAT 1 be replaced by FORMAT 100 to make all basic numerical input consistent in card column range.

```
1   FORMAT (I1, I3, 19, I4)
100 FORMAT (I1, 2X, I5, 9(3X, I5)
250 FORMAT (I1, F7.0, 9F 8.0)
350 FORMAT (20 I 4)
450 FORMAT (15A2, 15A2, 10A2)
```

### 3. Input for Tide Mode.

For calibration of a given bay system, under virtually no wind conditions, for its response to forcing by astronomical tide at the seaward boundary and a steady-state river discharge, allowance is made in the coded program to bypass the detailed input of wind-stress components, and rainfall and channel-stress data; moreover, since a steady river discharge is assumed only a single card is required to define this input. In essence, the data blocks 10 to 17 are replaced by a shortened version of block 12 plus a modified version of block 14 in which tide data at the seaward boundary are prescribed at hourly intervals as the map time intervals. The input is summarized as follows:

*Control Card:* 0 in column 1

*Block 0:* see Section IV,1, NOWIND = -1

*Blocks 1 to 9* see Section IV,2

*Block 12* (1 card for steady river discharges)

READ 250 , IDENT, (RO(K,M), K = 1, IMRO)

*Astrotide Block* (1 card for each 12 hours)

905 READ 910, IGA, MTIME, (H(1,J), J = 1,12)

MU = MTIME + 12

IF (MU.LT.MMAX) GO TO 905

910 FORMAT (I2, I4, 12F 6.2)

(IGA = 1)

*Blocks 18 to 20:* see Section IV,2

Comments on Tide Mode. The map time interval for the tide mode is 1 hour. The MTIME entry for the astrotide block is the time (hour) of the first of 12-hourly values of HG (entered as H(1,J)). The tide is assumed uniform along the seaward boundary of the bay system, hence one HG value per hour is sufficient.

In starting the tide mode from rest state ( $U = V = 0$  and  $H = HGI = 0$ ), usually one or two diurnal tide cycles are required for the numerical model to reach a nearly periodic response to an almost periodic input. Thus, if the final diurnal cycle is to be free of initial transients, at least 72 hours of HG data should be provided. This may require an adjustment in the dimensions given in COMMON/BLK6/ which appears in subroutines MAIN, PART 2, and CONTIN, if the full data set is to be stored for one run. An alternative is to make use of the continuation option, using less data input per run (e.g., 24 hours).

#### 4. Input for Continuation of a Run.

Since the main purpose of the tide mode is for calibration of the bed friction coefficients for blocks and channels, it is expected that many trial runs will be made for a given bay system. In order to keep the machine time to a minimum for each successive run, it is desirable to use an initial field of  $U$ ,  $V$ , or  $H$  which is close to the true response at the starting time. This can be accomplished by using the resulting  $U$ ,  $V$ , and  $H$  arrays from a previous tide run for the bay system as the initial values. (This should be done even if the previous run has different values of the bed friction coefficients.) The mechanism for

accomplishing this is the use of the continuation mode option, as controlled by ICARD. In this mode, the contents of common from a previous run are input along with any additional forcing function data.

To make the program as flexible as possible, the continuation option can be used for either storm surge problems or astrotide problems, the only difference in input being in the type of forcing function input. Such forcing function data should be consistent with the continuation time. Moreover, the value of NTIME input in data block 1 should be equal to the final NTIME in the previous run which is continued.

The sequence of input for continuation of a problem is as follows:

*Control Card:* 1 in column 1

*Contin Deck:* Contents of COMMON output from a previous run

*Blocks 0 to 3:* see subsection 2 of this app. (4 cards)

*Forcing Deck:* For storm surge mode, blocks 13 to 17, inclusive.  
For tide mode-astrotide deck.

A flow diagram summarizing the READ operations as controlled by ICARD and NOWIND is given in Figure C-1.

##### 5. Comments on Barrier Input.

a. Possible Barrier Locations. All barriers in the schematization occur parallel to the sides of a given barrier block. Barrier data qualified by an X in the coded name (e.g., IZX, ICDOX, ICDXS) refer to barriers normal to the x-axis on the right side of the barrier block; those qualified by a Y in the coded name (e.g., IXY, etc.) refer to barriers normal to the y-axis on the upper side of the barrier block. If a channel exists parallel to either barrier, then such a barrier may occur on either or both sides of the parallel channel, depending upon the coding of the associated channel input data (as discussed in a subsequent subsection). Barriers which might exist along the left or lower side of a given block are represented by appropriate data coding of a barrier block in a previous row or column.

b. Precaution. It should be emphasized that for any barrier block it is up to the user to supply appropriate barrier elevations ZB for both the right and upper sides of the barrier block even if a real barrier occurs only on one side of the block. The important point to observe is that the specified ZB values should always equal or exceed the larger of the block elevations at or adjacent to the side of the barrier block in question. Otherwise, errors can occur in the computations.

c. Array Size. The number of barrier blocks KM is normally limited to less than 100.

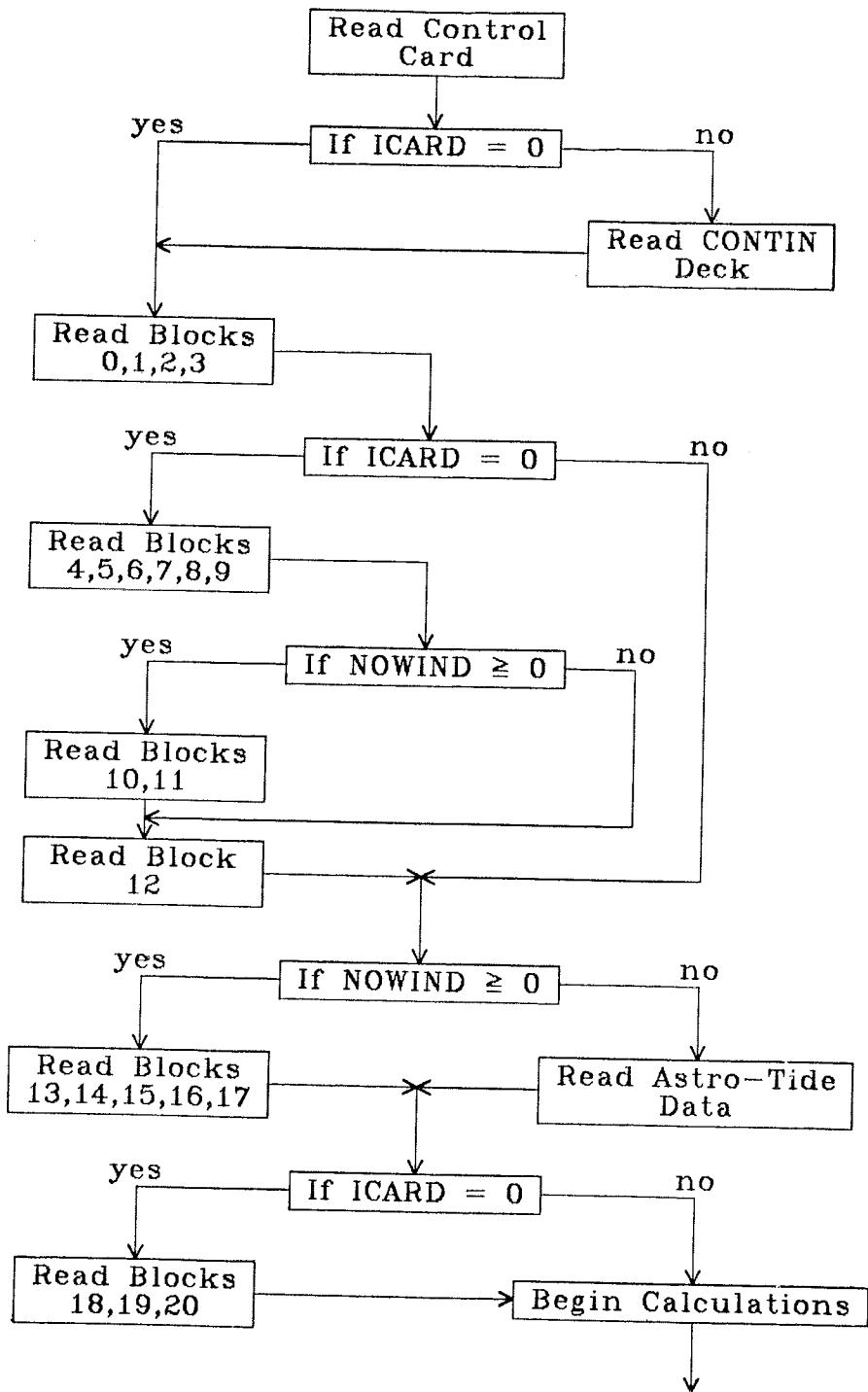


Figure C-1. Flow diagram for read statements.

6. Comments on Channel Input.

a. Possible Channel Locations. All channels in the schematization occur along the right side or the upper side of a given channel block. Channel data qualified by an X in the coded name (e.g., IWCX, IZCX) refer to channels normal to the X-axis on the right side of the channel block; those qualified by a Y in the coded name (e.g., IWCY, IZCY) refer to channels normal to the Y-axis on the upper side of the channel block. If a block has both an X and Y channel, one data card specifies both.

b. Channel Junctions. In the schematization of a channel system junctions can occur with adjoining channel reaches parallel to each other or perpendicular. Moreover, one-, two-, or three-way branches are possible.

Four possible right-angle channel junctions are illustrated in Figure C-2. The simplest junction is that shown in the upper right panel of the figure where the joining channel reaches are in the same channel block K1. Right-angle junctions involving two adjacent channel blocks are illustrated in the upper left and lower right panels of Figure C-2.

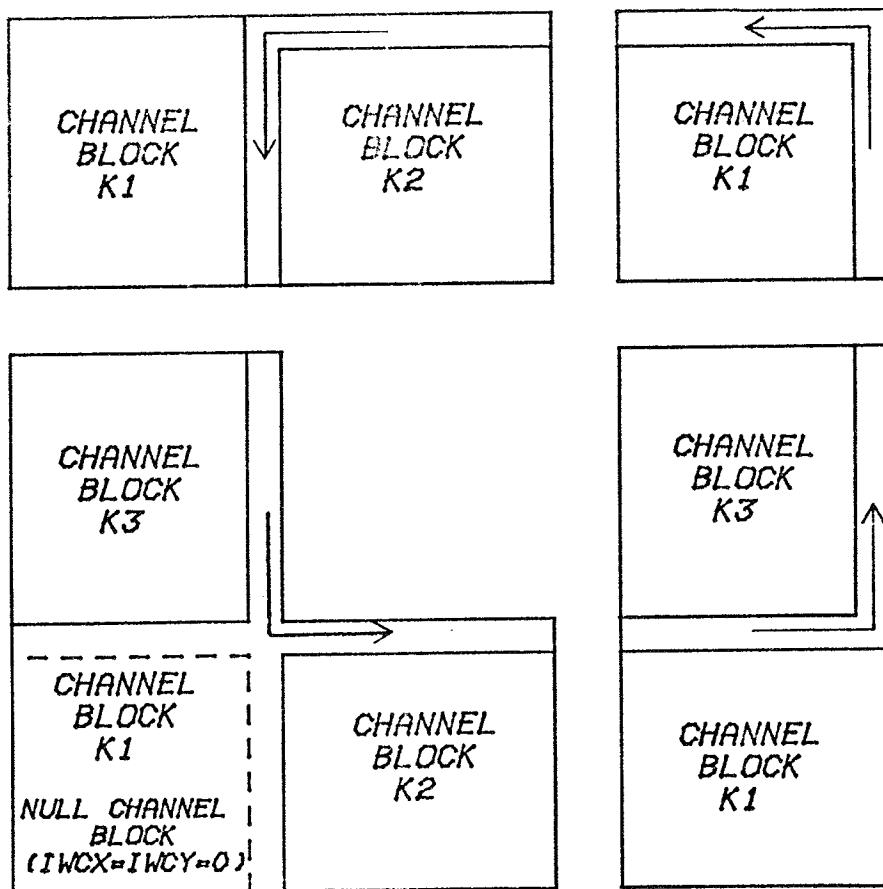


Figure C-2. Four possible simple bends for a channel reach.

The final possible right turn is illustrated in the lower left panel of the figure. In this case, the program requires that a channel block (K1) join the connecting channels of the nonadjoining channel blocks (K2 and K3) even though no channels exist on the joining block K1. In such circumstances, the required "null" channel block would have zero width for both the X and Y channels ( $IWCS = IWCY = 0$ ) as input. The H value at the junction of the connecting channel reaches for this case is stored as  $HC(K1)$ ; i.e., in association with the null channel block.

Colinear adjoining channels always involve two adjacent channel blocks. Four possible junctions of this type are illustrated in Figure B-2 in relation to central channel block K.

c. Channels with Levees. The program allows for the following possible situations with respect to barriers parallel to channels:

- (a) Single barrier on the "inner" lateral boundary of a channel;
- (b) single barrier on the "outer" lateral boundary of a channel;
- (c) barriers of *equal* elevation on both sides of a channel.

NOTE.--The term inner or outer side of a channel refers respectively to the side common to the channel block containing the channel or the side common to an adjacent block.

The barrier elevation information is input separately from the channel block data and allows only one elevation for the right side and one for the top side of a block (hence, the restriction of equal barrier heights for the double levee situation c above). The specification for situations a, b, or c is accomplished by a sign coding in the channel block data as follows:

- (a) Channel width (ICW) positive, channel-bed elevation (IZC)
- (b) channel width positive, channel-bed elevation positive;
- (c) channel width negative, channel-bed elevation negative.

It is understood that only the magnitude of IWC and IZC for a given channel is used in calculations.

d. Channel Terminations. A channel system can terminate at (a) a larger body of water representing a lake, bay, or sea; or (b) at a boundary or in a landlocked block within the system. In the second case, the program assumes that the flow at the channel end is zero unless a river discharge to the channel is specified (see input) and that the channel end block is one block inside the boundary block.

e. Restriction. Only channels with the channel bed below the mean water level (MWL) reference are allowed. The actual elevation used in calculations is - |IZC|, regardless of the sign on the input of IZC for a given channel.

f. Array Size. The number of channel blocks (including null channel blocks) is KCM. However, (CHANL(1)) creates arrays of length KCMP > KCM. The value of KCMP exceeds KCM by one plus the number of channels which terminate on the exterior boundary of the grid including the seaward boundary. Since KCMP is limited to 130, KCM should be less by the amount described above.

## 7. Output.

a. Listings of Input and Key Arrays. All input data are listed in easily identifiable form in the order in which the data are entered through block 18. Immediately following the basic channel input is a listing of the key arrays for channels, as discussed in Appendix A, including the assignment of sign coding for ICG and JCG.

Also printed out, in the same block format as the routine listings of H, are the block elevations.

b. Sequential Output. Normally, the routine output of computed values includes block H arrays and listings of all channel variables at pre-determined intervals of time (as determined by IOUT). It is possible to list the U, V, and H arrays for blocks by changing the CALL LIST(2) statement following statement 2,100 in PART 2 to CALL LIST(3).

For channel listings, refer to Figure 6 for notation; the listings are ordered by channel block number K. The block location I,J is repeated (negative signs indicating end points). This is followed by HX, the water level (feet) and QXN, the volume transport (cubic feet per second) at the lower end of the x channel, then QCP, the transport at the upper end of the x channel. These are followed by HY, QYN, and QYP representing, respectively, the water level and flow at the left end and flow at the right end of the y channel. Next is HC, the water level at the junction of the x and y channels. The last four entries in the channel listings are the transports (in cubic feet per second) to the channel from the channel block and from the channel to an adjacent block for the x and y channels. The HC value is meaningful for null channels only.

c. Saved Time Sequences. Subroutine SAVE, if used, saves sequences of water level and flow at preselected locations (as identified in block 19 of the input). In the original version of the subroutine used with an IBM 360-65 computer the saved information was punched on cards to facilitate later graphing of the sequences.

APPENDIX D

COMPLETE DATA LISTING OF INPUT FOR  
SABINE-CALCASIEU REGION WITH  
FORCING DATA FOR HURRICANE CARLA

ICARD= 0 ISBL= 1 KCM= 121 NOWIND= 1 INTER= 15 NGAGE= 9 NFWLW= 2 IMIN= +1 IMAX= 10  
 IDNT= 1 NTIMES= 0 NM= 900 MMINS= 0 MMAX= 24 NFU= 45 IOUT= 449 INFLD= 0  
 IDNT= 2 IM= 28 JM= 20 KM= 91 KMAX= 8 LMAX= 6  
 IDNT= 3 DELX= 2.0 N MI DELT=240. SEC CDO= .200 FK= .0010 FC= .0010 HG= 3.200 FT  
 IDNT= 4 KI= 4 LJ= 4 KII= 7 LJII= 5 JBL= 2 JBR= 1

MIN OR MMAX IN ERROR

IDNT= 5 BARRIER DATA= Z VALUES IN TENTHS OF FEET. CD VALUES ARE TIMES 1000  
 K= 1 I= 8 J= 1 ZX= 50 ZY= 10 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 2 I= 20 J= 1 ZX= -150 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 3 I= 21 J= 1 ZX= -150 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 4 I= 22 J= 1 ZX= 50 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 5 I= 23 J= 1 ZX= -80 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 6 I= 24 J= 1 ZX= -100 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 7 I= 25 J= 1 ZX= -100 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 8 I= 26 J= 1 ZX= -100 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 9 I= 27 J= 1 ZX= -100 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 10 I= 28 J= 1 ZX= -100 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 11 I= 1 J= 2 ZX= -80 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 12 I= 2 J= 2 ZX= -100 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 13 I= 3 J= 2 ZX= -120 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 14 I= 4 J= 2 ZX= -100 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 15 I= 5 J= 2 ZX= -70 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 16 I= 6 J= 2 ZX= -10 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 17 I= 8 J= 2 ZX= -50 ZY= 60 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 18 I= 13 J= 2 ZX= -130 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 19 I= 14 J= 2 ZX= -120 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 20 I= 15 J= 2 ZX= -120 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 21 I= 16 J= 2 ZX= -120 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 22 I= 17 J= 2 ZX= -110 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 23 I= 18 J= 2 ZX= -80 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 24 I= 19 J= 2 ZX= -60 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 25 I= 22 J= 2 ZX= -80 ZY= 10 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 26 I= 6 J= 3 ZX= 60 ZY= 10 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 27 I= 7 J= 3 ZX= 20 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 28 I= 8 J= 3 ZX= 50 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 29 I= 9 J= 3 ZX= 10 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 30 I= 10 J= 3 ZX= 10 ZY= 40 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 31 I= 11 J= 3 ZX= 30 ZY= 40 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 32 I= 12 J= 3 ZX= 50 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 33 I= 22 J= 3 ZX= 80 ZY= 10 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 34 I= 1 J= 4 ZX= 10 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 35 I= 2 J= 4 ZX= 10 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 36 I= 3 J= 4 ZX= 10 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 37 I= 7 J= 4 ZX= 50 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 38 I= 22 J= 4 ZX= 30 ZY= -30 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 39 I= 3 J= 5 ZX= 50 ZY= 10 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 40 I= 4 J= 5 ZX= 10 ZY= 50 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 41 I= 5 J= 5 ZX= 10 ZY= 30 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 42 I= 4 J= 6 ZX= 30 ZY= 10 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400  
 K= 43 I= 6 J= 5 ZX= 50 ZY= 10 CDOX= 200 CDUY= 200 CDSX= 400 CDSY= 400

Ks	44	Ihs	5	Jhs	6	Zxs	30	Zys	-10	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	45	Ihs	6	Jhs	5	Zxs	50	Zys	40	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	44	Ihs	15	Jhs	6	Zxs	10	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	47	Ihs	16	Jhs	6	Zxs	10	Zys	60	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	48	Ihs	17	Jhs	6	Zxs	0	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	49	Ihs	23	Jhs	6	Zxs	100	Zys	10	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	50	Ihs	4	Jhs	7	Zxs	30	Zys	10	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	51	Ihs	6	Jhs	7	Zxs	50	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	52	Ihs	5	Jhs	7	Zxs	30	Zys	-10	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	53	Ihs	7	Jhs	7	Zxs	-40	Zys	140	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	54	Ihs	8	Jhs	7	Zxs	-60	Zys	140	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	55	Ihs	14	Jhs	7	Zxs	50	Zys	10	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	56	Ihs	17	Jhs	7	Zxs	50	Zys	0	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	57	Ihs	23	Jhs	7	Zxs	100	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	58	Ihs	4	Jhs	8	Zxs	30	Zys	10	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	59	Ihs	5	Jhs	8	Zxs	50	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	60	Ihs	6	Jhs	8	Zxs	50	Zys	30	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	61	Ihs	7	Jhs	8	Zxs	50	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	62	Ihs	8	Jhs	8	Zxs	140	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	63	Ihs	9	Jhs	8	Zxs	-60	Zys	140	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	64	Ihs	14	Jhs	8	Zxs	50	Zys	10	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	65	Ihs	15	Jhs	8	Zxs	0	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	66	Ihs	16	Jhs	8	Zxs	0	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	67	Ihs	17	Jhs	8	Zxs	50	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	68	Ihs	23	Jhs	8	Zxs	120	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	69	Ihs	6	Jhs	9	Zxs	50	Zys	30	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	70	Ihs	7	Jhs	9	Zxs	50	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	71	Ihs	9	Jhs	9	Zxs	140	Zys	70	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	72	Ihs	23	Jhs	9	Zxs	120	Zys	10	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	73	Ihs	9	Jhs	10	Zxs	140	Zys	70	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	74	Ihs	20	Jhs	10	Zxs	10	Zys	150	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	75	Ihs	21	Jhs	10	Zxs	10	Zys	150	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	76	Ihs	22	Jhs	10	Zxs	10	Zys	150	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	77	Ihs	23	Jhs	10	Zxs	120	Zys	120	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	78	Ihs	15	Jhs	11	Zxs	20	Zys	80	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	79	Ihs	16	Jhs	11	Zxs	10	Zys	100	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	80	Ihs	17	Jhs	11	Zxs	20	Zys	70	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	81	Ihs	18	Jhs	11	Zxs	50	Zys	70	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	82	Ihs	19	Jhs	11	Zxs	100	Zys	100	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	83	Ihs	6	Jhs	12	Zxs	80	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	84	Ihs	7	Jhs	12	Zxs	50	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	85	Ihs	14	Jhs	12	Zxs	20	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	86	Ihs	5	Jhs	13	Zxs	50	Zys	150	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	87	Ihs	13	Jhs	13	Zxs	50	Zys	120	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	88	Ihs	14	Jhs	13	Zxs	30	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	89	Ihs	5	Jhs	14	Zxs	10	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	90	Ihs	5	Jhs	16	Zxs	50	Zys	50	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400
Ks	91	Ihs	4	Jhs	17	Zxs	50	Zys	100	CDOxs	200	CDUYs	200	CDSx=	400	CDSY=	400

IDNT#	BLOCK	TOPOGRAPHY	Z VALUES IN FEET								
1	-24	-8	1	1	2	3	3	8	7	16	
1	11	19	23	26	32	35	30	20	35	100	
2	-24	-10	0	1	2	2	3	8	8	11	
2	7	14	15	23	25	35	30	20	35	100	
3	-24	-13	1	1	1	1	1	2	6	9	
3	4	7	11	16	22	25	30	7	7	100	
4	-24	-12	1	1	1	0	1	1	1	4	
4	12	13	14	20	23	15	3	7	7	100	
5	-24	-10	-1	0	1	-1	-1	-1	5	8	
5	15	17	15	1	1	1	7	7	7	100	
6	15	7	1	1	1	0	1	2	3	3	
7	-8	1	1	1	-5	-5	-4	5	-1	5	
7	13	1	3	16	18	15	20	25	25	100	
8	-4	1	2	0	1	-5	-6	2	5	12	
8	12	1	6	15	18	20	25	25	30	100	
9	-15	2	2	1	1	-6	-7	-6	2	7	
9	1	1	13	15	17	15	25	27	30	100	
10	-20	-8	1	1	1	-5	-8	-8	-6	-4	
10	1	7	11	12	18	21	22	28	30	100	
11	-22	-10	3	1	1	-6	-7	-7	-6	-6	
11	1	7	8	11	18	20	22	30	35	100	
12	-22	-13	3	2	1	1	1	1	-4	-3	
12	4	6	12	12	16	20	22	25	30	100	
13	-23	-15	5	2	1	1	1	1	1	1	
13	1	5	10	12	13	18	22	30	32	100	
14	-23	-13	4	1	1	1	1	1	1	1	
14	1	1	3	7	13	18	20	25	20	100	
15	-23	-12	2	1	1	1	1	0	0	1	
15	2	2	1	2	3	7	7	10	12	100	
16	-23	-12	1	1	1	1	1	0	0	1	
16	1	1	3	8	8	10	10	15	15	100	
17	-23	-12	1	1	1	1	0	0	1	3	
17	1	1	1	9	16	20	15	25	30	100	
18	-22	-11	1	1	1	1	0	0	1	3	
18	2	2	4	10	15	18	20	25	30	100	
19	-10	-8	-1	1	1	1	1	1	1	4	
19	5	8	8	11	14	18	20	20	32	100	
20	-17	1	1	1	1	1	1	1	1	1	
20	3	8	10	14	10	18	20	22	25	100	
21	-15	1	1	1	-4	1	0	7	-1	1	
21	3	6	10	12	14	18	20	22	25	100	
22	-15	1	1	-3	-4	1	1	5	0	1	
22	5	6	10	12	14	14	20	22	25	100	
23	-8	1	1	-4	-5	1	1	5	1	1	
23	4	6	12	13	14	14	20	22	25	100	
24	-10	1	1	-6	-7	-7	-7	-6	-6	1	
24	3	1	10	12	15	15	20	22	25	100	
25	-10	1	1	-5	-6	-5	-2	-6	-1	1	
25	5	8	8	12	15	15	20	22	25	100	
26	-10	1	1	1	1	1	1	1	1	1	
26	4	8	1	-5	15	15	20	22	25	100	
27	-10	1	1	1	1	1	1	1	1	1	
27	4	8	8	1	12	15	20	22	25	100	
28	-10	100	100	100	100	100	100	100	100	100	
28	100	100	100	100	100	100	100	100	100	100	

IDNT= 7 IMRO= 3 JMRO= 25 KF= 25 ISTR= 25 IND= 16 NUW= 45 KIM= 45 NORT= 15  
IDNT= 8 RF= 4.000 CONST= .9000 SE= .23662

IDNT= 9 3 PAIRS OF I,J FOR RUNOFF LOCATIONS

9	28	15	4	19	14	19	0	0	0	0
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IDNT= 10 PERCENT RAINFALL EACH MPTIME

0	.006	.0070	.0070	.0080	.0100	.0140	.0180	.0230	.0240	.0280
0	.032	.0360	.0470	.0630	.0840	.1070	.1460	.1400	.0650	.0390
0	.030	.0250	.0210	.0200						

IDNT= 11 CHANNEL STRESS VALUES AT MPTIMES

1	0.000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
1	0.000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000
1	0.000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000	0.0000

IDNT= 12 3 SETS OF RUNOFF VALUES IN CFS FOR 25 MPTIMES

2	800.	1107.	1520.
2	800.	1099.	1480.
2	800.	1099.	1480.
2	800.	1099.	1480.
2	800.	1099.	1480.
2	800.	1099.	1480.
2	800.	1099.	1480.
2	800.	1099.	1480.
2	800.	1099.	1480.
2	900.	1152.	1560.
2	900.	1152.	1560.
2	900.	1152.	1560.
2	900.	1152.	1560.
2	900.	1152.	1560.
2	900.	1152.	1560.
2	900.	1152.	1560.
2	1310.	1812.	2060.
2	1310.	1812.	2060.
2	1310.	1812.	2060.
2	1310.	1812.	2060.
2	1310.	1812.	2060.
2	1310.	1812.	2060.
2	1310.	1812.	2060.

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIMES = 0  
 4 4.500 4.5000 4.5000 4.5000 4.5000 4.5000 4.5000 4.5000  
 5 3.200 3.2000 3.2000 3.2000 3.2000 3.2000 3.2000 3.2000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MAPTIME= 0  
 6 -.029 -.0101 -.0090 -.0080 -.0062 -.0054  
 6 -.027 -.0169 -.0091 -.0081 -.0063 -.0055  
 6 -.030 -.0128 -.0100 -.0073 -.0064 -.0055  
 6 -.028 -.0143 -.0116 -.0073 -.0064 -.0055  
 6 -.028 -.0130 -.0105 -.0073 -.0064 -.0056  
 6 -.026 -.0225 -.0094 -.0074 -.0065 -.0056  
 6 -.026 -.0225 -.0094 -.0074 -.0065 -.0056  
 7 -.011 -.0037 -.0033 -.0029 -.0023 -.0020  
 7 -.009 -.0055 -.0030 -.0025 -.0020 -.0018  
 7 -.009 -.0037 -.0030 -.0020 -.0017 -.0015  
 7 -.007 -.0036 -.0031 -.0020 -.0017 -.0015  
 7 -.006 -.0030 -.0024 -.0017 -.0015 -.0013  
 7 -.005 -.0044 -.0018 -.0014 -.0013 -.0011  
 7 -.005 -.0044 -.0018 -.0014 -.0013 -.0011  
 7 -.005 -.0044 -.0018 -.0014 -.0013 -.0011

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIMES = 1  
 4 5.500 5.5000 5.5000 5.5000 5.5000 5.5000 5.5000 5.5000  
 5 4.500 4.5000 4.5000 4.5000 4.5000 4.5000 4.5000 4.5000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MAPTIME= 1  
 6 -.033 -.0112 -.0112 -.0090 -.0070 -.0054  
 6 -.031 -.0200 -.0102 -.0091 -.0071 -.0054  
 6 -.032 -.0141 -.0120 -.0091 -.0071 -.0055  
 6 -.032 -.0172 -.0142 -.0092 -.0072 -.0055  
 6 -.032 -.0144 -.0117 -.0093 -.0073 -.0055  
 6 -.030 -.0244 -.0118 -.0083 -.0073 -.0064  
 6 -.030 -.0244 -.0118 -.0083 -.0073 -.0064  
 6 -.030 -.0244 -.0118 -.0083 -.0073 -.0064  
 7 -.013 -.0043 -.0043 -.0035 -.0027 -.0021  
 7 -.011 -.0059 -.0035 -.0031 -.0025 -.0019  
 7 -.010 -.0043 -.0038 -.0030 -.0023 -.0018  
 7 -.009 -.0046 -.0041 -.0027 -.0022 -.0017  
 7 -.008 -.0033 -.0029 -.0023 -.0020 -.0015  
 7 -.006 -.0048 -.0025 -.0018 -.0017 -.0015  
 7 -.006 -.0048 -.0025 -.0018 -.0017 -.0015  
 7 -.006 -.0048 -.0025 -.0018 -.0017 -.0015

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 2

4	6.200	6.2000	6.2000	6.2000	6.2000	6.2000	6.2000
5	5.200	5.2000	5.2000	5.2000	5.2000	5.2000	5.2000

XR VALUES(IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 2

6	=.037	=.0136	=.0123	=.0111	=.0098	=.0053
6	=.036	=.0216	=.0112	=.0101	=.0090	=.0080
6	=.036	=.0169	=.0130	=.0102	=.0091	=.0081
6	=.036	=.0187	=.0155	=.0103	=.0091	=.0081
6	=.034	=.0143	=.0142	=.0104	=.0092	=.0082
6	=.035	=.0282	=.0144	=.0094	=.0083	=.0073
6	=.035	=.0282	=.0144	=.0094	=.0083	=.0073
6	=.035	=.0282	=.0144	=.0094	=.0083	=.0073
7	=.015	=.0058	=.0052	=.0047	=.0038	=.0022
7	=.013	=.0079	=.0043	=.0039	=.0035	=.0031
7	=.012	=.0055	=.0045	=.0035	=.0031	=.0028
7	=.011	=.0054	=.0048	=.0032	=.0030	=.0026
7	=.009	=.0036	=.0038	=.0028	=.0027	=.0024
7	=.007	=.0060	=.0033	=.0022	=.0021	=.0018
7	=.007	=.0060	=.0033	=.0022	=.0021	=.0018

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 3

4	6.000	6.0000	6.0000	6.0000	6.0000	6.0000	6.0000
5	5.200	5.2000	5.2000	5.2000	5.2000	5.2000	5.2000

XR VALUES(IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 3

6	=.040	=.0138	=.0125	=.0112	=.0100	=.0089
6	=.038	=.0201	=.0126	=.0102	=.0091	=.0081
6	=.039	=.0203	=.0130	=.0103	=.0092	=.0081
6	=.037	=.0205	=.0142	=.0104	=.0092	=.0082
6	=.035	=.0174	=.0158	=.0105	=.0083	=.0073
6	=.033	=.0305	=.0145	=.0094	=.0083	=.0074
6	=.033	=.0305	=.0145	=.0094	=.0083	=.0074
7	=.015	=.0053	=.0048	=.0043	=.0040	=.0036
7	=.013	=.0066	=.0044	=.0035	=.0031	=.0028
7	=.011	=.0059	=.0062	=.0032	=.0028	=.0025
7	=.009	=.0051	=.0038	=.0028	=.0027	=.0024
7	=.007	=.0037	=.0037	=.0024	=.0021	=.0018
7	=.005	=.0054	=.0026	=.0018	=.0018	=.0016
7	=.005	=.0054	=.0026	=.0018	=.0018	=.0016
7	=.005	=.0054	=.0026	=.0018	=.0018	=.0016

HGR FOLLOWED BY HBR ARRAY (IN FEET), AT MTIME= 4  
 4 5.800 5.8000 5.8000 5.8000 5.8000 5.8000 5.8000 5.8000  
 5 5.200 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 4  
 6 -.052 -.0205 -.0172 -.0156 -.0142 -.0128  
 6 -.053 -.0282 -.0173 -.0158 -.0130 -.0117  
 6 -.050 -.0226 -.0175 -.0145 -.0131 -.0118  
 6 -.048 -.0247 -.0176 -.0146 -.0118 -.0095  
 6 -.045 -.0229 -.0211 -.0133 -.0119 -.0106  
 6 -.043 -.0378 -.0194 -.0133 -.0107 -.0085  
 6 -.043 -.0378 -.0194 -.0133 -.0107 -.0085  
 7 -.012 -.0051 -.0046 -.0045 -.0041 -.0039  
 7 -.010 -.0060 -.0040 -.0037 -.0032 -.0029  
 7 -.008 -.0040 -.0047 -.0028 -.0028 -.0025  
 7 -.006 -.0031 -.0025 -.0023 -.0021 -.0017  
 7 -.003 -.0020 -.0022 -.0016 -.0017 -.0017  
 7 -.002 -.0020 -.0014 -.0012 -.0011 -.0010  
 7 -.002 -.0020 -.0014 -.0012 -.0011 -.0010  
 7 -.002 -.0020 -.0014 -.0012 -.0011 -.0010

HGR FOLLOWED BY HBR ARRAY (IN FEET), AT MTIME= 5  
 4 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000  
 5 5.500 5.5000 5.5000 5.5000 5.5000 5.5000 5.5000 5.5000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 5  
 6 -.065 -.0244 -.0225 -.0189 -.0173 -.0158  
 6 -.062 -.0373 -.0208 -.0192 -.0159 -.0131  
 6 -.059 -.0308 -.0210 -.0176 -.0160 -.0145  
 6 -.057 -.0248 -.0229 -.0177 -.0161 -.0146  
 6 -.057 -.0268 -.0248 -.0162 -.0147 -.0133  
 6 -.051 -.0454 -.0212 -.0162 -.0147 -.0133  
 6 -.051 -.0454 -.0212 -.0162 -.0147 -.0133  
 7 -.012 -.0048 -.0048 -.0044 -.0040 -.0039  
 7 -.009 -.0060 -.0037 -.0034 -.0031 -.0026  
 7 -.005 -.0033 -.0043 -.0028 -.0026 -.0026  
 7 -.003 -.0018 -.0020 -.0019 -.0020 -.0021  
 7 -.001 -.0010 -.0013 -.0011 -.0013 -.0014  
 7 .001 0.0000 -.0004 -.0006 -.0008 -.0009  
 7 .001 0.0000 -.0004 -.0006 -.0008 -.0009  
 7 .001 0.0000 -.0004 -.0006 -.0008 -.0009

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 6  
 4 6.500 6.5000 6.5000 6.5000 6.5000 6.5000 6.5000 6.5000  
 5 5.600 5.6000 5.6000 5.6000 5.6000 5.6000 5.6000 5.6000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 6  
 6 -.069 -.0248 -.0211 -.0193 -.0161 -.0146  
 6 -.063 -.0332 -.0211 -.0177 -.0162 -.0146  
 6 -.060 -.0268 -.0220 -.0162 -.0147 -.0133  
 6 -.054 -.0230 -.0230 -.0162 -.0133 -.0108  
 6 -.051 -.0229 -.0229 -.0147 -.0120 -.0096  
 6 -.045 -.0352 -.0194 -.0133 -.0120 -.0108  
 6 -.045 -.0352 -.0194 -.0133 -.0120 -.0108  
 6 -.045 -.0352 -.0194 -.0133 -.0120 -.0108  
 7 -.005 -.0022 -.0022 -.0024 -.0023 -.0023  
 7 -.001 -.0012 -.0011 -.0016 -.0017 -.0018  
 7 .002 .0.0000 -.0006 -.0006 -.0010 -.0012  
 7 .004 .0.0008 -.0004 .0.0000 -.0005 -.0008  
 7 .005 .0.0020 .0.0012 .0.0005 .0.0002 .0.0000  
 7 .006 .0.0043 .0.0017 .0.0009 .0.0006 .0.0004  
 7 .006 .0.0043 .0.0017 .0.0009 .0.0006 .0.0004

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 7  
 4 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000  
 5 5.400 5.4000 5.4000 5.4000 5.4000 5.4000 5.4000 5.4000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 7  
 6 -.065 -.0247 -.0211 -.0194 -.0178 -.0162  
 6 -.059 -.0307 -.0211 -.0177 -.0162 -.0147  
 6 -.056 -.0286 -.0210 -.0162 -.0147 -.0133  
 6 -.050 -.0207 -.0209 -.0166 -.0132 -.0119  
 6 -.044 -.0205 -.0207 -.0131 -.0119 -.0107  
 6 -.039 -.0278 -.0189 -.0117 -.0106 -.0095  
 6 -.039 -.0278 -.0189 -.0117 -.0106 -.0095  
 7 .008 .0.0026 .0.0015 .0.0010 .0.0006 .0.0003  
 7 .010 .0.0043 .0.0022 .0.0015 .0.0011 .0.0008  
 7 .010 .0.0040 .0.0030 .0.0014 .0.0010 .0.0007  
 7 .011 .0.0044 .0.0037 .0.0023 .0.0018 .0.0015  
 7 .012 .0.0051 .0.0044 .0.0025 .0.0019 .0.0015  
 7 .012 .0.0079 .0.0047 .0.0027 .0.0020 .0.0017  
 7 .012 .0.0079 .0.0047 .0.0027 .0.0020 .0.0017

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 8  
 4 6.000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000 6.0000  
 5 5.300 5.3000 5.3000 5.3000 5.3000 5.3000 5.3000 5.3000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 8  
 6 -.048 -.0185 -.0156 -.0143 -.0130 -.0118  
 6 -.042 -.0216 -.0155 -.0128 -.0116 -.0105  
 6 -.039 -.0198 -.0150 -.0114 -.0103 -.0092  
 6 -.034 -.0150 -.0138 -.0114 -.0091 -.0072  
 6 -.030 -.0134 -.0136 -.0089 -.0080 -.0072  
 6 -.026 -.0175 -.0109 -.0078 -.0079 -.0080  
 6 -.026 -.0175 -.0109 -.0078 -.0079 -.0080  
 7 .017 .0060 .0045 .0038 .0030 .0025  
 7 .016 .0078 .0050 .0037 .0031 .0026  
 7 .017 .0076 .0060 .0037 .0031 .0026  
 7 .016 .0063 .0053 .0042 .0030 .0022  
 7 .015 .0062 .0056 .0036 .0029 .0023  
 7 .014 .0085 .0051 .0035 .0032 .0029  
 7 .014 .0095 .0051 .0035 .0032 .0029  
 7 .014 .0085 .0051 .0035 .0032 .0029

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 9  
 4 7.200 7.2000 7.2000 7.2000 7.2000 7.2000 7.2000 7.2000  
 5 6.200 6.2000 6.2000 6.2000 6.2000 6.2000 6.2000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 9  
 6 -.060 -.0249 -.0216 -.0201 -.0186 -.0171  
 6 -.057 -.0303 -.0213 -.0183 -.0148 -.0155  
 6 -.050 -.0296 -.0240 -.0165 -.0152 -.0139  
 6 -.045 -.0222 -.0208 -.0148 -.0137 -.0126  
 6 -.037 -.0170 -.0189 -.0133 -.0122 -.0110  
 6 -.036 -.0248 -.0142 -.0119 -.0108 -.0161  
 6 -.036 -.0248 -.0142 -.0119 -.0108 -.0161  
 7 .027 .0100 .0078 .0065 .0057 .0049  
 7 .026 .0135 .0086 .0066 .0058 .0050  
 7 .027 .0150 .0090 .0067 .0058 .0050  
 7 .025 .0113 .0097 .0066 .0055 .0046  
 7 .022 .0094 .0096 .0065 .0054 .0049  
 7 .023 .0148 .0079 .0061 .0053 .0075  
 7 .023 .0148 .0079 .0061 .0053 .0075

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 10

4	7.000	7.0000	7.0000	7.0000	7.0000	7.0000	7.0000	7.0000
5	6.000	6.0000	6.0000	6.0000	6.0000	6.0000	6.0000	6.0000

XR VALUES(IOUNT=6) AND YR VALUES (IOUNT=7) AT MTIME= 10

6	.048	.0194	.0179	.0166	.0140	.0115
6	.042	.0224	.0178	.0137	.0125	.0113
6	.037	.0187	.0150	.0122	.0112	.0101
6	.032	.0170	.0132	.0109	.0099	.0090
6	.028	.0141	.0157	.0096	.0087	.0078
6	.024	.0165	.0104	.0085	.0076	.0068
6	.024	.0165	.0104	.0085	.0076	.0068
6	.024	.0165	.0104	.0085	.0076	.0068
7	.024	.0086	.0076	.0064	.0048	.0035
7	.023	.0109	.0079	.0055	.0048	.0041
7	.021	.0099	.0080	.0054	.0045	.0037
7	.020	.0094	.0067	.0051	.0042	.0034
7	.018	.0081	.0083	.0049	.0041	.0035
7	.016	.0103	.0060	.0045	.0039	.0033
7	.016	.0103	.0060	.0045	.0039	.0033
7	.016	.0103	.0060	.0045	.0039	.0033

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 11

4	5.800	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000
5	5.800	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000

XR VALUES(IOUNT=6) AND YR VALUES (IOUNT=7) AT MTIME= 11

6	.054	.0213	.0199	.0169	.0156	.0143
6	.046	.0245	.0196	.0153	.0140	.0128
6	.040	.0225	.0180	.0138	.0126	.0114
6	.037	.0189	.0147	.0123	.0112	.0102
6	.033	.0157	.0175	.0110	.0099	.0089
6	.028	.0199	.0130	.0098	.0088	.0079
6	.028	.0199	.0130	.0098	.0088	.0079
6	.028	.0199	.0130	.0098	.0088	.0079
7	.024	.0086	.0072	.0055	.0045	.0036
7	.022	.0109	.0079	.0055	.0045	.0037
7	.021	.0105	.0075	.0053	.0046	.0039
7	.021	.0096	.0069	.0052	.0043	.0035
7	.019	.0083	.0085	.0049	.0042	.0036
7	.018	.0115	.0069	.0045	.0039	.0033
7	.018	.0115	.0069	.0045	.0039	.0033
7	.018	.0115	.0069	.0045	.0039	.0033

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME# 12

4	6.200	6.2000	6.2000	6.2000	6.2000	6.2000	6.2000	6.2000	6.2000
5	5.300	5.3000	5.3000	5.3000	5.3000	5.3000	5.3000	5.3000	5.3000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 12

6	-.040	-.0186	-.0148	-.0139	-.0129	-.0119
6	-.035	-.0169	-.0146	-.0124	-.0115	-.0105
6	-.029	-.0165	-.0130	-.0100	-.0091	-.0083
6	-.025	-.0136	-.0110	-.0087	-.0081	-.0074
6	-.021	-.0101	-.0115	-.0077	-.0070	-.0063
6	-.018	-.0121	-.0092	-.0059	-.0061	-.0055
6	-.018	-.0121	-.0092	-.0059	-.0061	-.0055
6	-.018	-.0121	-.0092	-.0059	-.0061	-.0055
7	.031	.0135	.0099	.0084	.0071	.0061
7	.029	.0127	.0102	.0080	.0069	.0058
7	.025	.0133	.0095	.0067	.0057	.0048
7	.022	.0114	.0091	.0063	.0052	.0043
7	.020	.0088	.0093	.0058	.0049	.0041
7	.018	.0109	.0077	.0046	.0044	.0037
7	.018	.0109	.0077	.0046	.0044	.0037
7	.018	.0109	.0077	.0046	.0044	.0037

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME# 13

4	6.600	6.6000	6.6000	6.6000	6.6000	6.7000	6.7000	6.7000	6.7000
5	5.800	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000	5.8000

XR VALUES (IDNT#6) AND YR VALUES (IDNT#7) AT MPTIME# 13

6	-.049	-.0255	-.0218	-.0211	-.0201	-.0191
6	-.042	-.0251	-.0199	-.0179	-.0169	-.0159
6	-.037	-.0247	-.0180	-.0174	-.0153	-.0133
6	-.033	-.0197	-.0179	-.0145	-.0126	-.0109
6	-.028	-.0154	-.0150	-.0108	-.0111	-.0102
6	-.024	-.0193	-.0135	-.0096	-.0079	-.0066
6	-.024	-.0193	-.0135	-.0096	-.0079	-.0066
6	-.024	-.0193	-.0135	-.0096	-.0079	-.0066
7	.053	.0246	.0189	.0165	.0146	.0128
7	.047	.0251	.0179	.0144	.0127	.0111
7	.043	.0255	.0170	.0151	.0120	.0093
7	.039	.0211	.0173	.0130	.0102	.0076
7	.033	.0171	.0150	.0101	.0097	.0086
7	.029	.0214	.0140	.0093	.0091	.0085
7	.029	.0214	.0140	.0093	.0091	.0085
7	.029	.0214	.0140	.0093	.0091	.0085

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 14  
4 5.800 5.8000 5.8000 5.9000 6.0000 6.1000 6.2000 6.2000  
5 5.600 5.6000 5.6000 5.6000 5.6000 5.6000 5.6000

XR VALUES(IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 14  
6 -.041 -.0223 -.0188 -.0179 -.0171 -.0163  
6 -.036 -.0235 -.0157 -.0151 -.0155 -.0146  
6 -.032 -.0201 -.0150 -.0162 -.0142 -.0121  
6 -.029 -.0186 -.0155 -.0134 -.0128 -.0121  
6 -.025 -.0147 -.0152 -.0110 -.0115 -.0097  
6 -.022 -.0183 -.0126 -.0099 -.0102 -.0105  
6 -.022 -.0183 -.0126 -.0099 -.0102 -.0105  
7 .044 .0215 .0163 .0144 .0124 .0106  
7 .040 .0234 .0142 .0122 .0117 .0102  
7 .036 .0207 .0140 .0136 .0107 .0085  
7 .032 .0193 .0144 .0117 .0100 .0085  
7 .029 .0152 .0147 .0099 .0093 .0071  
7 .025 .0196 .0126 .0089 .0086 .0082  
7 .025 .0196 .0126 .0089 .0086 .0082

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 15  
4 4.300 4.3000 4.3000 4.5000 4.7000 4.9000 5.1000 5.1000  
5 5.200 5.2000 5.2000 5.2000 5.2000 5.2000 5.2000

XR VALUES(IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 15  
6 -.040 -.0222 -.0190 -.0185 -.0191 -.0181  
6 -.036 -.0264 -.0173 -.0168 -.0176 -.0169  
6 -.031 -.0233 -.0170 -.0182 -.0160 -.0138  
6 -.030 -.0203 -.0173 -.0152 -.0147 -.0140  
6 -.029 -.0163 -.0170 -.0128 -.0132 -.0113  
6 -.025 -.0218 -.0145 -.0115 -.0119 -.0102  
6 -.025 -.0218 -.0145 -.0115 -.0119 -.0102  
7 .049 .0246 .0190 .0166 .0160 .0141  
7 .044 .0304 .0179 .0157 .0148 .0127  
7 .040 .0267 .0180 .0169 .0139 .0112  
7 .037 .0234 .0179 .0147 .0127 .0110  
7 .035 .0187 .0182 .0124 .0119 .0095  
7 .032 .0250 .0155 .0115 .0111 .0086  
7 .032 .0250 .0155 .0115 .0111 .0086

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 16

4	4.600	4.6000	4.6000	4.7000	4.9000	5.0000	5.1000	5.1000
5	5.100	5.1000	5.1000	5.1000	5.1000	5.1000	5.1000	5.1000

\* XR VALUES(IDNT=6) AND YR VALUES (IDNT=7) AT MTIME= 16

6	.027	.0156	.0149	.0162	.0174	.0160
6	.026	.0201	.0138	.0150	.0150	.0160
6	.025	.0184	.0140	.0150	.0138	.0125
6	.023	.0161	.0153	.0139	.0128	.0115
6	.021	.0150	.0142	.0109	.0117	.0125
6	.019	.0155	.0132	.0100	.0110	.0099
6	.019	.0155	.0132	.0100	.0110	.0099
6	.019	.0155	.0132	.0100	.0110	.0099
7	.057	.0293	.0247	.0239	.0230	.0190
7	.054	.0378	.0230	.0222	.0198	.0190
7	.051	.0331	.0240	.0222	.0183	.0149
7	.045	.0290	.0245	.0206	.0169	.0136
7	.040	.0271	.0227	.0161	.0155	.0149
7	.038	.0268	.0211	.0148	.0140	.0110
7	.038	.0268	.0211	.0148	.0140	.0110
7	.038	.0268	.0211	.0148	.0140	.0110

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 17

4	5.400	5.4000	5.4000	5.6000	5.8000	6.0000	6.0000	6.0000
5	5.300	5.3000	5.3000	5.3000	5.3000	5.3000	5.3000	5.3000

\* XR VALUES(IDNT=6) AND YR VALUES (IDNT=7) AT MTIME= 17

6	.015	.0139	.0149	.0138	.0124	.0114
6	.015	.0125	.0089	.0111	.0121	.0131
6	.013	.0118	.0089	.0111	.0118	.0122
6	.014	.0111	.0108	.0104	.0101	.0096
6	.013	.0097	.0101	.0086	.0094	.0093
6	.013	.0103	.0092	.0086	.0096	.0085
6	.013	.0103	.0092	.0086	.0086	.0085
6	.013	.0103	.0092	.0086	.0086	.0085
7	.071	.0518	.0486	.0378	.0308	.0243
7	.064	.0464	.0274	.0289	.0285	.0281
7	.058	.0411	.0280	.0289	.0264	.0239
7	.055	.0362	.0314	.0269	.0227	.0189
7	.049	.0317	.0293	.0213	.0210	.0190
7	.044	.0316	.0252	.0213	.0193	.0175
7	.044	.0316	.0252	.0213	.0193	.0175
7	.044	.0316	.0252	.0213	.0193	.0175

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 18

4	4.700	4.7000	4.7000	4.9000	5.1000	5.3000	5.4000	5.4000
5	5.000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000	5.0000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 18

6	.002	.0000	-.0007	-.0020	-.0028	-.0040
6	.001	-.0006	-.0011	-.0022	-.0030	-.0037
6	0.000	-.0011	-.0028	-.0024	-.0031	-.0031
6	-.001	-.0014	-.0019	-.0024	-.0028	-.0028
6	-.001	-.0015	-.0022	-.0023	-.0028	-.0026
6	-.002	-.0016	-.0020	-.0021	-.0026	-.0025
6	-.002	-.0016	-.0020	-.0021	-.0026	-.0025
6	-.002	-.0016	-.0020	-.0021	-.0026	-.0025
7	.048	.0249	.0212	.0229	.0228	.0226
7	.045	.0332	.0211	.0211	.0210	.0208
7	.040	.0310	.0210	.0193	.0192	.0175
7	.038	.0268	.0211	.0193	.0176	.0160
7	.033	.0211	.0211	.0161	.0160	.0131
7	.031	.0229	.0193	.0146	.0145	.0118
7	.031	.0229	.0193	.0146	.0145	.0118
7	.031	.0229	.0193	.0146	.0145	.0118

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 19

4	3.200	3.2000	3.2000	3.5000	3.8000	4.0000	4.1000	4.1000
5	4.400	4.4000	4.4000	4.4000	4.4000	4.4000	4.4000	4.4000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 19

6	.016	.0075	.0056	.0050	.0035	.0025
6	.014	.0096	.0046	.0037	.0028	.0019
6	.012	.0072	.0050	.0030	.0022	.0013
6	.011	.0061	.0043	.0030	.0017	.0007
6	.009	.0048	.0035	.0020	.0012	.0004
6	.008	.0046	.0032	.0015	.0011	.0007
6	.008	.0046	.0032	.0015	.0011	.0007
6	.008	.0046	.0032	.0015	.0011	.0007
7	.037	.0301	.0262	.0294	.0286	.0288
7	.055	.0417	.0264	.0266	.0267	.0268
7	.052	.0371	.0280	.0247	.0248	.0248
7	.050	.0349	.0307	.0297	.0248	.0212
7	.047	.0306	.0286	.0229	.0229	.0230
7	.045	.0329	.0308	.0211	.0211	.0212
7	.045	.0329	.0308	.0211	.0211	.0212
7	.045	.0329	.0308	.0211	.0211	.0212

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME# 20  
 4 2.700 2.7000 2.7000 2.9000 3.2000 3.4000 3.6000 3.6000  
 5 4.000 4.0000 4.0000 4.0000 4.0000 4.0000 4.0000 4.0000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7), AT MAPTIME# 20  
 6 .018 .0086 .0073 .0076 .0075 .0073  
 6 .015 .0113 .0064 .0063 .0062 .0059  
 6 .014 .0082 .0060 .0055 .0054 .0051  
 6 .012 .0078 .0065 .0054 .0046 .0039  
 6 .011 .0060 .0057 .0042 .0036 .0028  
 6 .009 .0062 .0049 .0032 .0033 .0034  
 6 .009 .0062 .0049 .0032 .0033 .0034  
 7 .036 .0193 .0180 .0198 .0217 .0238  
 7 .035 .0266 .0186 .0184 .0203 .0222  
 7 .033 .0215 .0185 .0169 .0187 .0205  
 7 .031 .0216 .0201 .0187 .0172 .0158  
 7 .029 .0185 .0186 .0157 .0158 .0145  
 7 .027 .0203 .0171 .0130 .0144 .0159  
 7 .027 .0203 .0171 .0130 .0144 .0159

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME# 21  
 4 3.700 3.7000 3.7000 3.8000 3.9000 4.0000 4.1000 4.1000  
 5 4.600 4.6000 4.6000 4.6000 4.6000 4.6000 4.6000 4.6000

XR VALUES(IDNT#6) AND YR VALUES (IDNT#7), AT MAPTIME# 21  
 6 .016 .0074 .0062 .0075 .0070 .0083  
 6 .013 .0093 .0062 .0063 .0067 .0070  
 6 .012 .0072 .0060 .0053 .0055 .0058  
 6 .011 .0070 .0064 .0061 .0048 .0037  
 6 .010 .0053 .0053 .0043 .0039 .0031  
 6 .008 .0061 .0043 .0037 .0035 .0030  
 6 .008 .0061 .0043 .0037 .0035 .0030  
 7 .029 .0145 .0134 .0161 .0164 .0195  
 7 .026 .0190 .0134 .0150 .0165 .0142  
 7 .026 .0163 .0150 .0138 .0153 .0168  
 7 .025 .0164 .0166 .0167 .0140 .0114  
 7 .023 .0138 .0138 .0126 .0128 .0116  
 7 .022 .0167 .0126 .0114 .0115 .0104  
 7 .022 .0167 .0126 .0114 .0115 .0104

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 22

4	3.500	3.5000	3.5000	3.7000	3.9000	4.0000	4.1000	4.1000
5	4.400	4.4000	4.4000	4.4000	4.4000	4.4000	4.4000	4.4000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 22

6	.013	.0060	.0058	.0063	.0067	.0071
6	.012	.0079	.0056	.0061	.0058	.0062
6	.010	.0067	.0060	.0051	.0054	.0058
6	.010	.0065	.0069	.0060	.0047	.0030
6	.008	.0049	.0049	.0040	.0039	.0033
6	.007	.0058	.0040	.0034	.0037	.0039
6	.007	.0058	.0040	.0034	.0037	.0039
6	.007	.0058	.0040	.0034	.0037	.0039
7	.021	.0104	.0105	.0118	.0132	.0146
7	.020	.0142	.0106	.0119	.0120	.0134
7	.019	.0132	.0120	.0109	.0122	.0136
7	.019	.0133	.0147	.0135	.0111	.0089
7	.018	.0110	.0110	.0100	.0101	.0090
7	.016	.0136	.0100	.0090	.0101	.0114
7	.016	.0136	.0100	.0090	.0101	.0114

HGR FOLLOWED BY HBR ARRAY (IN FEET) AT MTIME= 23

4	2.200	2.2000	2.2000	2.4000	2.6000	2.8000	2.9000	2.9000
5	3.500	3.5000	3.5000	3.5000	3.5000	3.5000	3.5000	3.5000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MAPTIME= 23

6	.009	.0041	.0046	.0052	.0055	.0058
6	.008	.0052	.0044	.0049	.0048	.0047
6	.007	.0049	.0050	.0043	.0047	.0051
6	.007	.0047	.0058	.0045	.0044	.0033
6	.006	.0035	.0039	.0033	.0036	.0039
6	.006	.0044	.0032	.0032	.0031	.0029
6	.006	.0044	.0032	.0032	.0031	.0029
6	.006	.0044	.0032	.0032	.0031	.0029
7	.013	.0063	.0072	.0091	.0092	.0105
7	.012	.0081	.0073	.0082	.0083	.0084
7	.011	.0082	.0090	.0074	.0084	.0095
7	.013	.0084	.0105	.0085	.0086	.0068
7	.011	.0066	.0076	.0068	.0077	.0088
7	.011	.0086	.0068	.0068	.0069	.0069
7	.011	.0086	.0068	.0068	.0069	.0069
7	.011	.0086	.0068	.0068	.0069	.0069

HGP FOLLOWED BY HBR ARRAY (IN FEET) AT MTIMES 24  
 4 1.300 1.3000 1.3000 1.5000 1.7000 1.9000 2.1000 2.1000  
 5 3.500 3.5000 3.5000 3.5000 3.5000 3.5000 3.5000

XR VALUES (IDNT=6) AND YR VALUES (IDNT=7) AT MPTIMES 24

6	.006	.0030	.0035	.0040	.0044	.0049
6	.006	.0034	.0033	.0038	.0042	.0040
6	.005	.0037	.0037	.0032	.0036	.0040
6	.005	.0031	.0045	.0030	.0034	.0038
6	.004	.0026	.0030	.0029	.0033	.0038
6	.004	.0033	.0024	.0024	.0027	.0027
6	.004	.0033	.0024	.0024	.0027	.0027
7	.008	.0040	.0040	.0053	.0061	.0070
7	.008	.0046	.0047	.0054	.0062	.0072
7	.007	.0055	.0055	.0048	.0055	.0064
7	.007	.0048	.0072	.0049	.0056	.0065
7	.006	.0042	.0049	.0050	.0057	.0065
7	.006	.0057	.0043	.0043	.0051	.0051
7	.006	.0057	.0043	.0043	.0051	.0051
7	.006	.0057	.0043	.0043	.0051	.0051

THE FOLLOWING ARE SURGRID CHANNEL DATA Z VALUES IN FEET

K <sub>1</sub>	1	JCG <sub>2</sub>	8	JCG <sub>2</sub>	1	IwCXz=2330	IzCYz =20	IwCYz	0	IzCYz	0	IFCs	15		
K <sub>2</sub>	2	JCG <sub>2</sub>	8	JCG <sub>2</sub>	2	IwCXz=2330	IzCYz =20	IwCYz	0	IzCYz	0	IFCs	15		
K <sub>3</sub>	3	JCG <sub>2</sub>	8	JCG <sub>2</sub>	3	IwCXz=2860	IzCYz =21	IwCYz	2860	IzCYz =21	IwCYz	0	IFCs	15	
K <sub>4</sub>	4	JCG <sub>2</sub>	7	JCG <sub>2</sub>	3	IwCXz	IzCYz =21	IwCYz	0	IzCYz	0	IFCs	15		
K <sub>5</sub>	5	JCG <sub>2</sub>	7	JCG <sub>2</sub>	4	IwCXz =2860	IzCYz =21	IwCYz	1000	IzCYz =26	IwCYz	0	IFCs	15	
K <sub>6</sub>	6	JCG <sub>2</sub>	6	JCG <sub>2</sub>	4	IwCXz	IzCYz =21	IwCYz	0	IwCYz	0	IzCYz	0	IFCs	15
K <sub>7</sub>	7	JCG <sub>2</sub>	6	JCG <sub>2</sub>	5	IwCXz =1000	IzCYz =26	IwCYz	0	IzCYz	0	IwCYz	0	IFCs	15
K <sub>8</sub>	8	JCG <sub>2</sub>	6	JCG <sub>2</sub>	6	IwCXz =900	IzCYz =26	IwCYz	300	IzCYz =12	IwCYz	0	IFCs	20	
K <sub>9</sub>	9	JCG <sub>2</sub>	6	JCG <sub>2</sub>	7	IwCXz =900	IzCYz =21	IwCYz	=300	IzCYz =15	IwCYz	0	IFCs	20	
K <sub>10</sub>	10	JCG <sub>2</sub>	7	JCG <sub>2</sub>	7	IwCXz	IzCYz =21	IwCYz	=900	IzCYz =26	IwCYz	0	IFCs	20	
K <sub>11</sub>	11	JCG <sub>2</sub>	8	JCG <sub>2</sub>	7	IwCXz	IzCYz =21	IwCYz	=900	IzCYz =26	IwCYz	0	IFCs	20	
K <sub>12</sub>	12	JCG <sub>2</sub>	8	JCG <sub>2</sub>	8	IwCXz =900	IzCYz =26	IwCYz	0	IzCYz	0	IFCs	20		
K <sub>13</sub>	13	JCG <sub>2</sub>	9	JCG <sub>2</sub>	8	IwCXz	IzCYz =26	IwCYz	=900	IzCYz =26	IwCYz	0	IFCs	20	
K <sub>14</sub>	14	JCG <sub>2</sub>	9	JCG <sub>2</sub>	9	IwCXz =900	IzCYz =26	IwCYz	0	IzCYz	0	IFCs	20		
K <sub>15</sub>	15	JCG <sub>2</sub>	9	JCG <sub>2</sub>	10	IwCXz =900	IzCYz =26	IwCYz	0	IzCYz	0	IFCs	20		
K <sub>16</sub>	16	JCG <sub>2</sub>	9	JCG <sub>2</sub>	11	IwCXz =400	IzCYz =35	IwCYz	400	IzCYz =35	IwCYz	0	IFCs	25	
K <sub>17</sub>	17	JCG <sub>2</sub>	8	JCG <sub>2</sub>	11	IwCXz	IzCYz =35	IwCYz	400	IzCYz =35	IwCYz	0	IFCs	25	
K <sub>18</sub>	18	JCG <sub>2</sub>	7	JCG <sub>2</sub>	11	IwCXz	IzCYz =35	IwCYz	400	IzCYz =35	IwCYz	0	IFCs	25	
K <sub>19</sub>	19	JCG <sub>2</sub>	7	JCG <sub>2</sub>	12	IwCXz =400	IzCYz =35	IwCYz	=400	IzCYz =35	IwCYz	0	IFCs	25	
K <sub>20</sub>	20	JCG <sub>2</sub>	6	JCG <sub>2</sub>	12	IwCXz	IzCYz =35	IwCYz	=350	IzCYz =43	IwCYz	0	IFCs	25	
K <sub>21</sub>	21	JCG <sub>2</sub>	5	JCG <sub>2</sub>	12	IwCXz	IzCYz =35	IwCYz	0	IzCYz	0	IFCs	25		
K <sub>22</sub>	22	JCG <sub>2</sub>	5	JCG <sub>2</sub>	13	IwCXz =350	IzCYz =43	IwCYz	0	IzCYz	0	IFCs	25		
K <sub>23</sub>	23	JCG <sub>2</sub>	5	JCG <sub>2</sub>	14	IwCXz =350	IzCYz =43	IwCYz	=350	IzCYz =43	IwCYz	0	IFCs	25	
K <sub>24</sub>	24	JCG <sub>2</sub>	4	JCG <sub>2</sub>	14	IwCXz	IzCYz =43	IwCYz	0	IzCYz	0	IFCs	25		
K <sub>25</sub>	25	JCG <sub>2</sub>	4	JCG <sub>2</sub>	15	IwCXz =300	IzCYz =25	IwCYz	0	IzCYz	0	IFCs	25		
K <sub>26</sub>	26	JCG <sub>2</sub>	5	JCG <sub>2</sub>	15	IwCXz	IzCYz =25	IwCYz	400	IzCYz =40	IwCYz	0	IFCs	25	
K <sub>27</sub>	27	JCG <sub>2</sub>	5	JCG <sub>2</sub>	16	IwCXz =400	IzCYz =40	IwCYz	=400	IzCYz =30	IwCYz	0	IFCs	25	
K <sub>28</sub>	28	JCG <sub>2</sub>	4	JCG <sub>2</sub>	16	IwCXz	IzCYz =40	IwCYz	0	IwCYz	0	IzCYz	0	IFCs	25
K <sub>29</sub>	29	JCG <sub>2</sub>	4	JCG <sub>2</sub>	17	IwCXz =400	IzCYz =30	IwCYz	150	IzCYz =20	IwCYz	0	IFCs	25	
K <sub>30</sub>	30	JCG <sub>2</sub>	5	JCG <sub>2</sub>	17	IwCXz	IzCYz =30	IwCYz	=300	IzCYz =30	IwCYz	0	IFCs	25	

K#	31	ICG#	5	JCG#	18	IwCX#	400	IZCX#	-30	IwCY#	300	IZCY#	-30	IFCs	25
K#	32	ICG#	4	JCG#	18	IwCX#	0	IZCX#	0	IwCY#	0	IZCY#	0	IFCs	25
K#	33	ICG#	4	JCG#	19	IwCX#	300	IZCX#	-30	IwCY#	0	IZCY#	0	IFCs	25
K#	34	ICG#	3	JCG#	17	IwCX#	0	IZCX#	0	IwCY#	0	IZCY#	0	IFCs	25
K#	35	ICG#	3	JCG#	18	IwCX#	150	IZCX#	-20	IwCY#	100	IZCY#	-20	IFCs	25
K#	36	ICG#	2	JCG#	18	IwCX#	0	IZCX#	0	IwCY#	100	IZCY#	-20	IFCs	25
K#	37	ICG#	10	JCG#	10	IwCX#	0	IZCX#	0	IwCY#	200	IZCY#	-12	IFCs	25
K#	38	ICG#	11	JCG#	10	IwCX#	0	IZCX#	0	IwCY#	200	IZCY#	-12	IFCs	25
K#	39	ICG#	12	JCG#	10	IwCX#	0	IZCX#	0	IwCY#	200	IZCY#	-12	IFCs	25
K#	40	ICG#	12	JCG#	11	IwCX#	200	IZCX#	-27	IwCY#	200	IZCY#	-20	IFCs	25
K#	41	ICG#	13	JCG#	11	IwCX#	0	IZCX#	0	IwCY#	200	IZCY#	-27	IFCs	25
K#	42	ICG#	14	JCG#	11	IwCX#	0	IZCX#	0	IwCY#	200	IZCY#	-27	IFCs	25
K#	43	ICG#	14	JCG#	12	IwCX#	200	IZCX#	-27	IwCY#	-200	IZCY#	-27	IFCs	25
K#	44	ICG#	13	JCG#	12	IwCX#	0	IZCX#	0	IwCY#	0	IZCY#	0	IFCs	25
K#	45	ICG#	13	JCG#	13	IwCX#	200	IZCX#	27	IwCY#	0	IZCY#	0	IFCs	25
K#	46	ICG#	14	JCG#	13	IwCX#	0	IZCX#	0	IwCY#	200	IZCY#	27	IFCs	25
K#	47	ICG#	14	JCG#	14	IwCX#	500	IZCX#	-20	IwCY#	0	IZCY#	0	IFCs	25
K#	48	ICG#	15	JCG#	14	IwCX#	0	IZCX#	0	IwCY#	350	IZCY#	-20	IFCs	25
K#	49	ICG#	15	JCG#	15	IwCX#	350	IZCX#	-20	IwCY#	200	IZCY#	-20	IFCs	25
K#	50	ICG#	14	JCG#	15	IwCX#	0	IZCX#	0	IwCY#	0	IZCY#	0	IFCs	25
K#	51	ICG#	14	JCG#	16	IwCX#	200	IZCX#	-20	IwCY#	0	IZCY#	0	IFCs	25
K#	52	ICG#	14	JCG#	17	IwCX#	200	IZCX#	-15	IwCY#	0	IZCY#	0	IFCs	25
K#	53	ICG#	14	JCG#	18	IwCX#	100	IZCX#	-10	IwCY#	0	IZCY#	0	IFCs	25
K#	54	ICG#	14	JCG#	19	IwCX#	100	IZCX#	-10	IwCY#	0	IZCY#	0	IFCs	25
K#	55	ICG#	11	JCG#	11	IwCX#	0	IZCX#	0	IwCY#	0	IZCY#	0	IFCs	9
K#	56	ICG#	11	JCG#	12	IwCX#	200	IZCX#	-20	IwCY#	200	IZCY#	-20	IFCs	9
K#	57	ICG#	10	JCG#	12	IwCX#	0	IZCX#	0	IwCY#	0	IZCY#	0	IFCs	9
K#	58	ICG#	10	JCG#	13	IwCX#	200	IZCX#	-20	IwCY#	100	IZCY#	-20	IFCs	9
K#	59	ICG#	9	JCG#	13	IwCX#	0	IZCX#	0	IwCY#	0	IZCY#	0	IFCs	9
K#	60	ICG#	9	JCG#	14	IwCX#	100	IZCX#	-20	IwCY#	0	IZCY#	0	IFCs	9
K#	61	ICG#	15	JCG#	11	IwCX#	0	IZCX#	0	IwCY#	300	IZCY#	-12	IFCs	9
K#	62	ICG#	16	JCG#	11	IwCX#	0	IZCX#	0	IwCY#	300	IZCY#	-12	IFCs	9
K#	63	ICG#	17	JCG#	11	IwCX#	0	IZCX#	0	IwCY#	-300	IZCY#	-12	IFCs	9
K#	64	ICG#	18	JCG#	11	IwCX#	0	IZCX#	0	IwCY#	-300	IZCY#	-12	IFCs	9
K#	65	ICG#	19	JCG#	11	IwCX#	300	IZCX#	-12	IwCY#	-300	IZCY#	-12	IFCs	9
K#	66	ICG#	19	JCG#	10	IwCX#	0	IZCX#	0	IwCY#	0	IZCY#	0	IFCs	9
K#	67	ICG#	20	JCG#	10	IwCX#	0	IZCX#	0	IwCY#	300	IZCY#	-12	IFCs	9
K#	68	ICG#	21	JCG#	10	IwCX#	0	IZCX#	0	IwCY#	300	IZCY#	-12	IFCs	9
K#	69	ICG#	22	JCG#	10	IwCX#	0	IZCX#	0	IwCY#	300	IZCY#	-12	IFCs	9
K#	70	ICG#	23	JCG#	10	IwCX#	-400	IZCX#	-40	IwCY#	300	IZCY#	-12	IFCs	9
K#	71	ICG#	22	JCG#	1	IwCX#	-800	IZCX#	-32	IwCY#	0	IZCY#	0	IFCs	9
K#	72	ICG#	23	JCG#	1	IwCX#	0	IZCY#	0	IwCY#	1000	IZCY#	-16	IFCs	15
K#	73	ICG#	23	JCG#	2	IwCX#	1000	IZCX#	-16	IwCY#	1000	IZCY#	-15	IFCs	15
K#	74	ICG#	22	JCG#	2	IwCX#	500	IZCX#	-40	IwCY#	0	IZCY#	0	IFCs	15
K#	75	ICG#	22	JCG#	3	IwCX#	800	IZCX#	-32	IwCY#	0	IZCY#	0	IFCs	15
K#	76	ICG#	22	JCG#	4	IwCX#	1000	IZCX#	-20	IwCY#	0	IZCY#	0	IFCs	15
K#	77	ICG#	22	JCG#	5	IwCX#	1000	IZCX#	-20	IwCY#	0	IZCY#	0	IFCs	5
K#	78	ICG#	23	JCG#	5	IwCX#	0	IZCX#	0	IwCY#	400	IZCY#	-40	IFCs	5
K#	79	ICG#	23	JCG#	6	IwCX#	400	IZCX#	40	IwCY#	0	IZCY#	0	IFCs	5
K#	80	ICG#	23	JCG#	7	IwCX#	400	IZCX#	40	IwCY#	0	IZCY#	0	IFCs	5
K#	81	ICG#	23	JCG#	8	IwCX#	400	IZCX#	40	IwCY#	0	IZCY#	0	IFCs	5
K#	82	ICG#	23	JCG#	9	IwCX#	400	IZCX#	40	IwCY#	0	IZCY#	0	IFCs	5
K#	83	ICG#	24	JCG#	9	IwCX#	0	IZCX#	0	IwCY#	800	IZCY#	-25	IFCs	5
K#	84	ICG#	24	JCG#	10	IwCX#	800	IZCX#	-25	IwCY#	1000	IZCY#	-20	IFCs	5

Kz	85	ICGz	24	JCGz	11	IWCXz	800	IZCXz	-25	IWCYz	0	IZCYz	0	IFCs	5
Kz	86	ICGz	24	JCGz	12	IWCXz	900	IZCXz	-20	IWCYz	0	IZCYz	0	IFCs	5
Kz	87	ICGz	24	JCGz	13	IWCXz	1000	IZCXz	-25	IWCYz	0	IZCYz	0	IFCs	5
Kz	88	ICGz	25	JCGz	13	IWCXz	0	IZCXz	0	IWCYz	1000	IZCYz	-25	IFCs	5
Kz	89	ICGz	25	JCGz	14	IWCXz	400	IZCXz	-40	IWCYz	0	IZCYz	0	IFCs	5
Kz	90	ICGz	26	JCGz	14	IWCXz	0	IZCXz	0	IWCYz	800	IZCYz	-20	IFCs	5
Kz	91	ICGz	27	JCGz	14	IWCXz	0	IZCXz	0	IWCYz	800	IZCYz	-20	IFCs	5
Kz	92	ICGz	27	JCGz	15	IWCXz	800	IZCXz	-20	IWCYz	0	IZCYz	0	IFCs	5
Kz	93	ICGz	28	JCGz	15	IWCXz	0	IZCXz	0	IWCYz	800	IZCYz	-20	IFCs	5
Kz	94	ICGz	25	JCGz	10	IWCXz	0	IZCXz	0	IWCYz	300	IZCYz	-12	IFCs	9
Kz	95	ICGz	26	JCGz	10	IWCXz	300	IZCXz	-12	IWCYz	300	IZCYz	-12	IFCs	9
Kz	96	ICGz	26	JCGz	9	IWCXz	0	IZCXz	0	IWCYz	0	IZCYz	0	IFCs	9
Kz	97	ICGz	27	JCGz	9	IWCXz	300	IZCXz	-12	IWCYz	300	IZCYz	-12	IFCs	9
Kz	98	ICGz	27	JCGz	8	IWCXz	0	IZCXz	0	IWCYz	300	IZCYz	-12	IFCs	9
Kz	99	ICGz	28	JCGz	8	IWCXz	0	IZCXz	0	IWCYz	300	IZCYz	-12	IFCs	9
Kz	100	ICGz	1	JCGz	4	IWCXz	0	IZCXz	0	IWCYz	300	IZCYz	-12	IFCs	9
Kz	101	ICGz	2	JCGz	4	IWCXz	0	IZCXz	0	IWCYz	300	IZCYz	-12	IFCs	9
Kz	102	ICGz	3	JCGz	4	IWCXz	0	IZCXz	0	IWCYz	300	IZCYz	-12	IFCs	9
Kz	103	ICGz	3	JCGz	5	IWCXz	300	IZCXz	12	IWCYz	0	IZCYz	-12	IFCs	9
Kz	104	ICGz	4	JCGz	5	IWCXz	0	IZCXz	0	IWCYz	300	IZCYz	-12	IFCs	9
Kz	105	ICGz	5	JCGz	5	IWCXz	0	IZCXz	0	IWCYz	300	IZCYz	-12	IFCs	9
Kz	106	ICGz	5	JCGz	6	IWCXz	300	IZCXz	-12	IWCYz	0	IZCYz	0	IFCs	9
Kz	107	ICGz	5	JCGz	7	IWCXz	0	IZCXz	0	IWCYz	0	IZCYz	0	IFCs	9
Kz	108	ICGz	5	JCGz	8	IWCXz	400	IZCXz	15	IWCYz	400	IZCYz	15	IFCs	9
Kz	109	ICGz	4	JCGz	8	IWCXz	0	IZCXz	0	IWCYz	0	IZCYz	0	IFCs	9
Kz	110	ICGz	4	JCGz	9	IWCXz	200	IZCXz	-12	IWCYz	200	IZCYz	-10	IFCs	9
Kz	111	ICGz	3	JCGz	9	IWCXz	200	IZCXz	10	IWCYz	0	IZCYz	0	IFCs	9
Kz	112	ICGz	3	JCGz	8	IWCXz	0	IZCXz	0	IWCYz	200	IZCYz	-10	IFCs	9
Kz	113	ICGz	2	JCGz	8	IWCXz	0	IZCXz	0	IWCYz	200	IZCYz	-10	IFCs	9
Kz	114	ICGz	1	JCGz	8	IWCXz	0	IZCXz	0	IWCYz	200	IZCYz	-10	IFCs	9
Kz	115	ICGz	3	JCGz	10	IWCXz	200	IZCXz	-10	IWCYz	200	IZCYz	-10	IFCs	9
Kz	116	ICGz	2	JCGz	10	IWCXz	0	IZCXz	0	IWCYz	0	IZCYz	0	IFCs	9
Kz	117	ICGz	2	JCGz	11	IWCXz	200	IZCXz	-8	IWCYz	0	IZCYz	0	IFCs	9
Kz	118	ICGz	2	JCGz	12	IWCXz	200	IZCXz	-8	IWCYz	0	IZCYz	0	IFCs	9
Kz	119	ICGz	6	JCGz	8	IWCXz	-400	IZCXz	-20	IWCYz	0	IZCYz	0	IFCs	9
Kz	120	ICGz	7	JCGz	8	IWCXz	0	IZCXz	0	IWCYz	200	IZCYz	-20	IFCs	9
Kz	121	ICGz	7	JCGz	5	IWCXz	3000	IZCXz	-12	IWCYz	0	IZCYz	0	IFCs	9

#### HYDROGRAPH GAGE LOCATIONS

GAGE 1 BLOCK H, I<sub>s</sub> 8 J<sub>s</sub> 1  
 GAGE 2 CHANNEL H, K<sub>s</sub> 11  
 GAGE 3 BLOCK H, I<sub>s</sub> 11 J<sub>s</sub> 10  
 GAGE 4 CHANNEL H, K<sub>s</sub> 25  
 GAGE 5 CHANNEL H, K<sub>s</sub> 46  
 GAGE 6 CHANNEL H, K<sub>s</sub> 72  
 GAGE 7 CHANNEL H, K<sub>s</sub> 100  
 GAGE 8 CHANNEL H, K<sub>s</sub> 3  
 GAGE 9 CHANNEL H, K<sub>s</sub> 92

#### KEY FLOW LOCATIONS

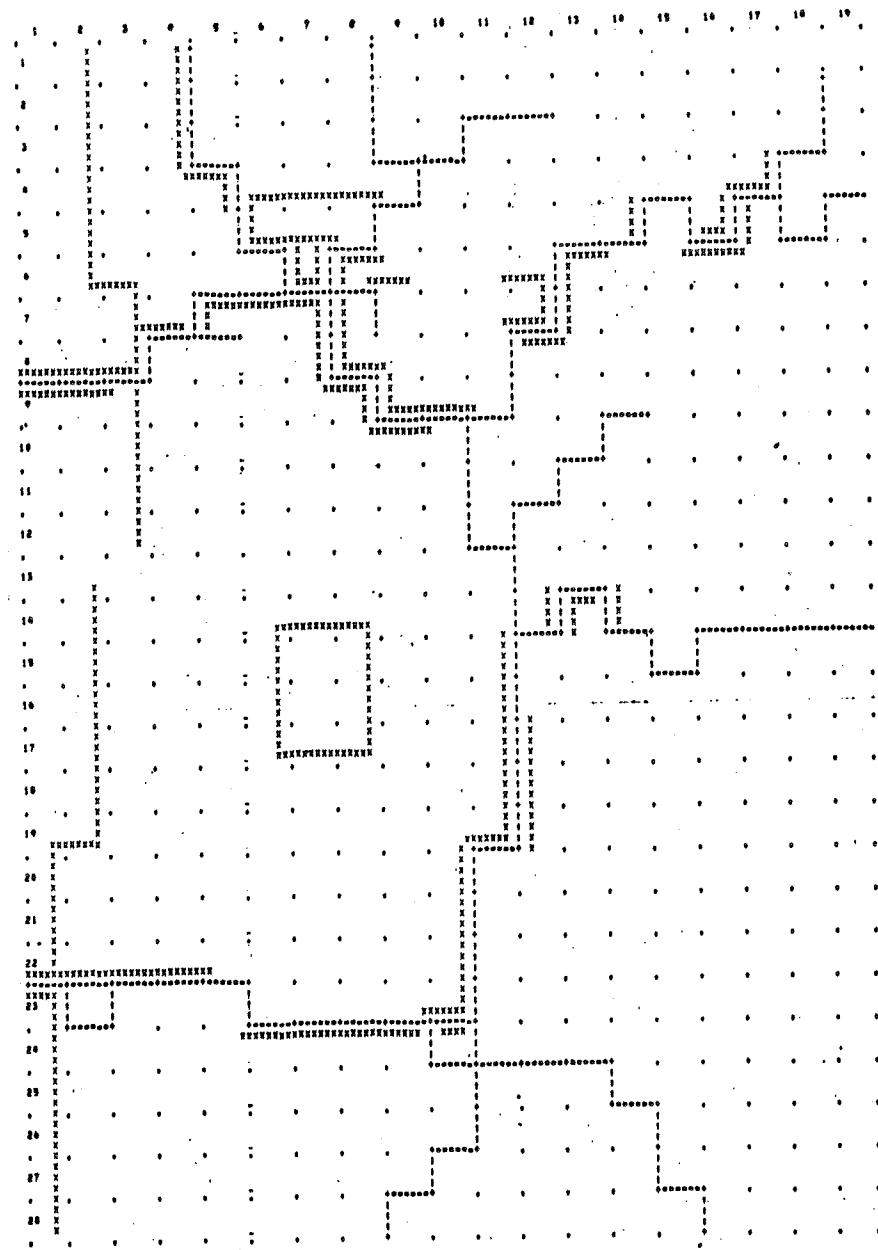
CHANNEL BLOCKS 1 71

APPENDIX E

LISTING OF KEY ARRAYS AND CHANNEL AND BARRIER PLOT  
FOR SABINE-CALCASIEU REGION

Kz	1	KCXB <sub>122</sub>	KCY <sub>128</sub>	KCXP <sub>8</sub>	2	KCVP <sub>8128</sub>	KCB <sub>8</sub>	1	ICG <sub>8</sub>	=8	JCG <sub>8</sub>	1	KEN <sub>1</sub> <sub>8</sub>	1	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0			
Kz	2	KCXB <sub>8</sub>	1	KCY <sub>128</sub>	KCXP <sub>8</sub>	3	KCVP <sub>8128</sub>	KCB <sub>8</sub>	17	ICG <sub>8</sub>	8	JCG <sub>8</sub>	2	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	3	KCXB <sub>8</sub>	2	KCY <sub>8</sub>	4	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	28	ICG <sub>8</sub>	8	JCG <sub>8</sub>	3	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	4	KCXB <sub>128</sub>	KCY <sub>128</sub>	KCXP <sub>8</sub>	5	KCVP <sub>8</sub>	3	KCB <sub>8</sub>	27	ICG <sub>8</sub>	7	JCG <sub>8</sub>	3	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	5	KCXB <sub>8</sub>	4	KCY <sub>8</sub>	6	KCXP <sub>8121</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	37	ICG <sub>8</sub>	7	JCG <sub>8</sub>	4	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	6	KCXB <sub>128</sub>	KCY <sub>128</sub>	KCXP <sub>8</sub>	7	KCVP <sub>8</sub>	5	KCB <sub>8</sub>	0	ICG <sub>8</sub>	6	JCG <sub>8</sub>	4	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	7	KCXB <sub>8</sub>	6	KCY <sub>8105</sub>	KCXP <sub>8</sub>	8	KCVP <sub>8121</sub>	KCB <sub>8</sub>	43	ICG <sub>8</sub>	6	JCG <sub>8</sub>	5	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	8	KCXB <sub>8</sub>	7	KCY <sub>8106</sub>	KCXP <sub>8</sub>	9	KCVP <sub>8128</sub>	KCB <sub>8</sub>	45	ICG <sub>8</sub>	6	JCG <sub>8</sub>	6	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	9	KCXB <sub>8</sub>	8	KCY <sub>8107</sub>	KCXP <sub>8119</sub>	KCVP <sub>8</sub>	10	KCB <sub>8</sub>	51	ICG <sub>8</sub>	6	JCG <sub>8</sub>	7	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	10	KCXB <sub>128</sub>	KCY <sub>8</sub>	9	KCXP <sub>8120</sub>	KCVP <sub>8</sub>	11	KCB <sub>8</sub>	53	ICG <sub>8</sub>	7	JCG <sub>8</sub>	7	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	11	KCXB <sub>128</sub>	KCY <sub>8</sub>	10	KCXP <sub>8</sub>	12	KCVP <sub>8128</sub>	KCB <sub>8</sub>	54	ICG <sub>8</sub>	8	JCG <sub>8</sub>	7	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	12	KCXB <sub>8</sub>	11	KCY <sub>8120</sub>	KCXP <sub>8128</sub>	KCVP <sub>8</sub>	13	KCB <sub>8</sub>	62	ICG <sub>8</sub>	8	JCG <sub>8</sub>	8	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	13	KCXB <sub>128</sub>	KCY <sub>8</sub>	12	KCXP <sub>8</sub>	14	KCVP <sub>8128</sub>	KCB <sub>8</sub>	63	ICG <sub>8</sub>	9	JCG <sub>8</sub>	8	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	14	KCXB <sub>8</sub>	13	KCY <sub>8128</sub>	KCXP <sub>8</sub>	15	KCVP <sub>8128</sub>	KCB <sub>8</sub>	71	ICG <sub>8</sub>	9	JCG <sub>8</sub>	9	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	15	KCXB <sub>8</sub>	14	KCY <sub>8128</sub>	KCXP <sub>8</sub>	16	KCVP <sub>8128</sub>	KCB <sub>8</sub>	73	ICG <sub>8</sub>	9	JCG <sub>8</sub>	10	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	16	KCXB <sub>8</sub>	15	KCY <sub>8</sub>	17	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	9	JCG <sub>8</sub>	11	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	17	KCXB <sub>8</sub>	16	KCY <sub>8</sub>	18	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	16	ICG <sub>8</sub>	8	JCG <sub>8</sub>	11	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	18	KCXB <sub>8</sub>	18	KCY <sub>8128</sub>	KCXP <sub>8</sub>	19	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	7	JCG <sub>8</sub>	11	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	19	KCXB <sub>8</sub>	19	KCY <sub>8</sub>	20	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	84	ICG <sub>8</sub>	7	JCG <sub>8</sub>	12	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	20	KCXB <sub>8</sub>	20	KCY <sub>8</sub>	21	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	83	ICG <sub>8</sub>	6	JCG <sub>8</sub>	12	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	21	KCXB <sub>8</sub>	22	KCY <sub>8</sub>	23	KCXP <sub>822</sub>	KCVP <sub>8</sub>	KCB <sub>8</sub>	20	ICG <sub>8</sub>	5	JCG <sub>8</sub>	12	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	22	KCXB <sub>8</sub>	23	KCY <sub>8128</sub>	KCXP <sub>8</sub>	23	KCVP <sub>8128</sub>	KCB <sub>8</sub>	86	ICG <sub>8</sub>	5	JCG <sub>8</sub>	13	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	23	KCXB <sub>8</sub>	22	KCY <sub>8</sub>	24	KCXP <sub>8</sub>	26	KCVP <sub>8128</sub>	KCB <sub>8</sub>	89	ICG <sub>8</sub>	5	JCG <sub>8</sub>	14	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	24	KCXB <sub>8</sub>	28	KCY <sub>8128</sub>	KCXP <sub>8</sub>	25	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	4	JCG <sub>8</sub>	14	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	25	KCXB <sub>8</sub>	24	KCY <sub>8128</sub>	KCXP <sub>8</sub>	26	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	4	JCG <sub>8</sub>	15	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	26	KCXB <sub>8</sub>	23	KCY <sub>8</sub>	25	KCXP <sub>8</sub>	27	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	5	JCG <sub>8</sub>	15	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	27	KCXB <sub>8</sub>	26	KCY <sub>8</sub>	28	KCXP <sub>8</sub>	30	KCVP <sub>8128</sub>	KCB <sub>8</sub>	90	ICG <sub>8</sub>	5	JCG <sub>8</sub>	16	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	28	KCXB <sub>8</sub>	25	KCY <sub>8128</sub>	KCXP <sub>8</sub>	29	KCVP <sub>8128</sub>	KCB <sub>8</sub>	27	ICG <sub>8</sub>	4	JCG <sub>8</sub>	16	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	29	KCXB <sub>8</sub>	28	KCY <sub>8</sub>	34	KCXP <sub>8</sub>	32	KCVP <sub>8128</sub>	KCB <sub>8</sub>	30	ICG <sub>8</sub>	4	JCG <sub>8</sub>	17	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	30	KCXB <sub>8</sub>	27	KCY <sub>8</sub>	29	KCXP <sub>8</sub>	31	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	5	JCG <sub>8</sub>	17	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	31	KCXB <sub>8</sub>	30	KCY <sub>8</sub>	32	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	4	JCG <sub>8</sub>	18	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	32	KCXB <sub>8</sub>	29	KCY <sub>8</sub>	35	KCXP <sub>8</sub>	33	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	4	JCG <sub>8</sub>	18	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	33	KCXB <sub>8</sub>	32	KCY <sub>8128</sub>	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	4	JCG <sub>8</sub>	19	KEN <sub>1</sub> <sub>8</sub>	7	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0			
Kz	34	KCXB <sub>8</sub>	28	KCY <sub>8128</sub>	KCXP <sub>8</sub>	35	KCVP <sub>8</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	3	JCG <sub>8</sub>	17	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	35	KCXB <sub>8</sub>	34	KCY <sub>8</sub>	36	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	32	ICG <sub>8</sub>	3	JCG <sub>8</sub>	18	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	36	KCXB <sub>8</sub>	128	KCY <sub>8123</sub>	KCXP <sub>8128</sub>	KCVP <sub>8</sub>	35	KCB <sub>8</sub>	0	ICG <sub>8</sub>	=2	JCG <sub>8</sub>	18	KEN <sub>1</sub> <sub>8</sub>	6	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	37	KCXB <sub>8</sub>	128	KCY <sub>8</sub>	15	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	38	ICG <sub>8</sub>	10	JCG <sub>8</sub>	10	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	38	KCXB <sub>8</sub>	38	KCY <sub>8</sub>	37	KCXP <sub>85</sub>	55	KCVP <sub>8128</sub>	KCB <sub>8</sub>	39	ICG <sub>8</sub>	11	JCG <sub>8</sub>	10	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	39	KCXB <sub>8</sub>	38	KCY <sub>8</sub>	40	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	12	JCG <sub>8</sub>	10	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	40	KCXB <sub>8</sub>	39	KCY <sub>8</sub>	55	KCXP <sub>8128</sub>	KCVP <sub>8</sub>	KCB <sub>8</sub>	41	ICG <sub>8</sub>	12	JCG <sub>8</sub>	11	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	41	KCXB <sub>8</sub>	128	KCY <sub>8</sub>	40	KCXP <sub>8</sub>	44	KCVP <sub>8</sub>	42	KCB <sub>8</sub>	0	ICG <sub>8</sub>	13	JCG <sub>8</sub>	11	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0
Kz	42	KCXB <sub>8</sub>	128	KCY <sub>8</sub>	41	KCXP <sub>8</sub>	43	KCVP <sub>8</sub>	61	KCB <sub>8</sub>	0	ICG <sub>8</sub>	14	JCG <sub>8</sub>	11	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0
Kz	43	KCXB <sub>8</sub>	42	KCY <sub>8</sub>	44	KCXP <sub>8</sub>	46	KCVP <sub>8128</sub>	KCB <sub>8</sub>	85	ICG <sub>8</sub>	14	JCG <sub>8</sub>	12	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	44	KCXB <sub>8</sub>	41	KCY <sub>8128</sub>	KCXP <sub>8</sub>	45	KCVP <sub>8</sub>	43	KCB <sub>8</sub>	0	ICG <sub>8</sub>	13	JCG <sub>8</sub>	12	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	45	KCXB <sub>8</sub>	44	KCY <sub>8128</sub>	KCXP <sub>8</sub>	47	KCVP <sub>8128</sub>	KCB <sub>8</sub>	46	ICG <sub>8</sub>	87	ICG <sub>8</sub>	13	JCG <sub>8</sub>	13	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0
Kz	46	KCXB <sub>8</sub>	43	KCY <sub>8</sub>	45	KCXP <sub>8</sub>	47	KCVP <sub>8128</sub>	KCB <sub>8</sub>	88	ICG <sub>8</sub>	14	JCG <sub>8</sub>	13	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	47	KCXB <sub>8</sub>	46	KCY <sub>8128</sub>	KCXP <sub>8</sub>	50	KCVP <sub>8128</sub>	KCB <sub>8</sub>	48	ICG <sub>8</sub>	14	JCG <sub>8</sub>	14	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	48	KCXB <sub>8</sub>	128	KCY <sub>8</sub>	47	KCXP <sub>8</sub>	49	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	14	JCG <sub>8</sub>	18	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	49	KCXB <sub>8</sub>	48	KCY <sub>8</sub>	50	KCXP <sub>8128</sub>	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	15	JCG <sub>8</sub>	19	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	50	KCXB <sub>8</sub>	47	KCY <sub>8128</sub>	KCXP <sub>8</sub>	51	KCVP <sub>8</sub>	49	KCB <sub>8</sub>	0	ICG <sub>8</sub>	14	JCG <sub>8</sub>	15	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0	
Kz	51	KCXB <sub>8</sub>	50	KCY <sub>8128</sub>	KCXP <sub>8</sub>	52	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	14	JCG <sub>8</sub>	16	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	52	KCXB <sub>8</sub>	51	KCY <sub>8128</sub>	KCXP <sub>8</sub>	53	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	14	JCG <sub>8</sub>	17	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	53	KCXB <sub>8</sub>	52	KCY <sub>8128</sub>	KCXP <sub>8</sub>	54	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	14	JCG <sub>8</sub>	18	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	54	KCXB <sub>8</sub>	53	KCY <sub>8128</sub>	KCXP <sub>8</sub>	55	KCVP <sub>8128</sub>	KCB <sub>8</sub>	0	ICG <sub>8</sub>	14	JCG <sub>8</sub>	19	KEN <sub>1</sub> <sub>8</sub>	7	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	55	KCXB <sub>8</sub>	38	KCY <sub>8128</sub>	KCXP <sub>8</sub>	56	KCVP <sub>8128</sub>	KCB <sub>8</sub>	40	ICG <sub>8</sub>	11	JCG <sub>8</sub>	11	KEN <sub>1</sub> <sub>8</sub>	0	KEN <sub>2</sub> <sub>8</sub>	0	KRIS	0		
Kz	56																				

K#	64	KCX#128	KCY# 63	KCXP#128	KCYP# 65	KCB# 81	ICG# 18	JCG# 11	KEN1# 0	KEN2# 0	KRIS 0
K#	65	KCX# 66	KCY# 64	KCXP#128	KCYP#124	KCB# 82	ICG# 19	JCG# 11	KEN1# 0	KEN2# 0	KRIS 0
K#	66	KCX#128	KCY#128	KCXP# 65	KCYP# 67	KCB# 0	ICG# 19	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	67	KCX#128	KCY# 66	KCXP#128	KCYP# 68	KCB# 74	ICG# 20	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	68	KCX#128	KCY# 67	KCXP#128	KCYP# 69	KCB# 75	ICG# 21	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	69	KCX#128	KCY# 68	KCXP#128	KCYP# 70	KCB# 76	ICG# 22	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	70	KCX# 82	KCY# 69	KCXP#128	KCYP# 84	KCB# 77	ICG# 23	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	71	KCX#124	KCY#128	KCXP# 74	KCYP# 72	KCB# 4	ICG# 22	JCG# 1	KEN1# 1	KEN2# 0	KRIS 0
K#	72	KCX#128	KCY# 71	KCXP# 73	KCYP# 78	KCB# 5	ICG# 23	JCG# 1	KEN1# 0	KEN2# 0	KRIS 0
K#	73	KCX# 72	KCY# 74	KCXP#128	KCYP#128	KCB# 0	ICG# 23	JCG# 2	KEN1# 0	KEN2# 0	KRIS 0
K#	74	KCX# 71	KCY#128	KCXP# 75	KCYP# 73	KCB# 25	ICG# 22	JCG# 2	KEN1# 0	KEN2# 0	KRIS 0
K#	75	KCX# 74	KCY#128	KCXP# 76	KCYP#128	KCB# 33	ICG# 22	JCG# 3	KEN1# 0	KEN2# 0	KRIS 0
K#	76	KCX# 75	KCY#128	KCXP# 77	KCYP#128	KCB# 38	ICG# 22	JCG# 4	KEN1# 0	KEN2# 0	KRIS 0
K#	77	KCX# 76	KCY#128	KCXP#128	KCYP# 78	KCB# 0	ICG# 22	JCG# 5	KEN1# 0	KEN2# 0	KRIS 0
K#	78	KCX#128	KCY# 77	KCXP# 79	KCYP#128	KCB# 0	ICG# 23	JCG# 5	KEN1# 0	KEN2# 0	KRIS 0
K#	79	KCX# 78	KCY#128	KCXP# 80	KCYP#128	KCB# 49	ICG# 23	JCG# 6	KEN1# 0	KEN2# 0	KRIS 0
K#	80	KCX# 79	KCY#128	KCXP# 81	KCYP#128	KCB# 57	ICG# 23	JCG# 7	KEN1# 0	KEN2# 0	KRIS 0
K#	81	KCX# 80	KCY#128	KCXP# 82	KCYP#128	KCB# 68	ICG# 23	JCG# 8	KEN1# 0	KEN2# 0	KRIS 0
K#	82	KCX# 81	KCY#128	KCXP# 70	KCYP# 83	KCB# 72	ICG# 23	JCG# 9	KEN1# 0	KEN2# 0	KRIS 0
K#	83	KCX#128	KCY# 82	KCXP# 84	KCYP#128	KCB# 0	ICG# 24	JCG# 9	KEN1# 0	KEN2# 0	KRIS 0
K#	84	KCX# 83	KCY# 70	KCXP# 85	KCYP# 94	KCB# 0	ICG# 24	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	85	KCX# 84	KCY#128	KCXP# 86	KCYP#128	KCB# 0	ICG# 24	JCG# 11	KEN1# 0	KEN2# 0	KRIS 0
K#	86	KCX# 85	KCY#128	KCXP# 87	KCYP#128	KCB# 0	ICG# 24	JCG# 12	KEN1# 0	KEN2# 0	KRIS 0
K#	87	KCX# 86	KCY#128	KCXP#128	KCYP# 88	KCB# 0	ICG# 24	JCG# 13	KEN1# 0	KEN2# 0	KRIS 0
K#	88	KCX#128	KCY# 87	KCXP# 89	KCYP#128	KCB# 0	ICG# 25	JCG# 13	KEN1# 0	KEN2# 0	KRIS 0
K#	89	KCX# 88	KCY#128	KCXP#128	KCYP# 90	KCB# 0	ICG# 25	JCG# 14	KEN1# 0	KEN2# 0	KRIS 0
K#	90	KCX#128	KCY# 89	KCXP#128	KCYP# 91	KCB# 0	ICG# 26	JCG# 14	KEN1# 0	KEN2# 0	KRIS 0
K#	91	KCX#128	KCY# 90	KCXP# 92	KCYP#128	KCB# 0	ICG# 27	JCG# 14	KEN1# 0	KEN2# 0	KRIS 0
K#	92	KCX# 91	KCY#128	KCXP# 128	KCYP# 93	KCB# 0	ICG# 27	JCG# 15	KEN1# 0	KEN2# 0	KRIS 0
K#	93	KCX#128	KCY# 92	KCXP#128	KCYP#128	KCB# 0	ICG# 28	JCG# 15	KEN1# 8	KEN2# 0	KRIS 1
K#	94	KCX#128	KCY# 84	KCXP#128	KCYP# 95	KCB# 0	ICG# 25	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	95	KCX# 96	KCY# 94	KCXP#128	KCYP#128	KCB# 0	ICG# 26	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	96	KCX#128	KCY#128	KCXP# 95	KCYP# 97	KCB# 0	ICG# 26	JCG# 9	KEN1# 0	KEN2# 0	KRIS 0
K#	97	KCX# 98	KCY# 98	KCXP#128	KCYP#128	KCB# 0	ICG# 27	JCG# 9	KEN1# 0	KEN2# 0	KRIS 0
K#	98	KCX#128	KCY#128	KCXP# 97	KCYP# 99	KCB# 0	ICG# 27	JCG# 8	KEN1# 0	KEN2# 0	KRIS 0
K#	99	KCX#128	KCY# 98	KCXP#128	KCYP#128	KCB# 0	ICG# 28	JCG# 8	KEN1# 8	KEN2# 0	KRIS 0
K#	100	KCX#128	KCY#125	KCXP#128	KCYP#101	KCB# 34	ICG# -1	JCG# 6	KEN1# 0	KEN2# 0	KRIS 0
K#	101	KCX#128	KCY#100	KCXP#128	KCYP#102	KCB# 35	ICG# 2	JCG# 4	KEN1# 0	KEN2# 0	KRIS 0
K#	102	KCX#128	KCY#101	KCXP#103	KCYP#128	KCB# 36	ICG# 3	JCG# 4	KEN1# 0	KEN2# 0	KRIS 0
K#	103	KCX#102	KCY#128	KCXP#128	KCYP#104	KCB# 39	ICG# 3	JCG# 5	KEN1# 0	KEN2# 0	KRIS 0
K#	104	KCX#128	KCY#103	KCXP#128	KCYP#105	KCB# 40	ICG# 4	JCG# 5	KEN1# 0	KEN2# 0	KRIS 0
K#	105	KCX#128	KCY#104	KCXP#106	KCYP# 7	KCB# 41	ICG# 5	JCG# 5	KEN1# 0	KEN2# 0	KRIS 0
K#	106	KCX#105	KCY#128	KCXP#107	KCYP# 8	KCB# 44	ICG# 5	JCG# 6	KEN1# 0	KEN2# 0	KRIS 0
K#	107	KCX#106	KCY#128	KCXP#108	KCYP# 9	KCB# 52	ICG# 5	JCG# 7	KEN1# 0	KEN2# 0	KRIS 0
K#	108	KCX#107	KCY#109	KCXP#128	KCYP#119	KCB# 59	ICG# 5	JCG# 8	KEN1# 0	KEN2# 0	KRIS 0
K#	109	KCX#128	KCY#112	KCXP#110	KCYP#108	KCB# 58	ICG# 4	JCG# 8	KEN1# 0	KEN2# 0	KRIS 0
K#	110	KCX#109	KCY#111	KCXP#128	KCYP#128	KCB# 0	ICG# 4	JCG# 9	KEN1# 0	KEN2# 0	KRIS 0
K#	111	KCX#112	KCY#128	KCXP#115	KCYP#110	KCB# 0	ICG# 3	JCG# 9	KEN1# 0	KEN2# 0	KRIS 0
K#	112	KCX#128	KCY#113	KCXP#111	KCYP#109	KCB# 0	ICG# 3	JCG# 9	KEN1# 0	KEN2# 0	KRIS 0
K#	113	KCX#128	KCY#114	KCXP#128	KCYP#112	KCB# 0	ICG# 2	JCG# 8	KEN1# 0	KEN2# 0	KRIS 0
K#	114	KCX#128	KCY#126	KCXP#128	KCYP#113	KCB# 0	ICG# -1	JCG# 8	KEN1# 6	KEN2# 0	KRIS 0
K#	115	KCX#111	KCY#116	KCXP#128	KCYP#128	KCB# 0	ICG# 3	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	116	KCX#128	KCY#128	KCXP#117	KCYP#115	KCB# 0	ICG# 2	JCG# 10	KEN1# 0	KEN2# 0	KRIS 0
K#	117	KCX#116	KCY#128	KCXP#11A	KCYP#128	KCB# 0	ICG# -2	JCG# 11	KEN1# 0	KEN2# 0	KRIS 0
K#	118	KCX#117	KCY#128	KCXP#128	KCYP#128	KCB# 0	ICG# -2	JCG# 12	KEN1# 7	KEN2# 0	KRIS 0
K#	119	KCX# 9	KCY#108	KCXP#128	KCYP#120	KCB# 60	ICG# 6	JCG# 8	KEN1# 0	KEN2# 0	KRIS 0
K#	120	KCX# 10	KCY#119	KCXP#128	KCYP#127	KCB# 61	ICG# -7	JCG# 8	KEN1# 4	KEN2# 0	KRIS 0
K#	121	KCX# 5	KCY# 7	KCXP#128	KCYP#128	KCB# 0	ICG# -7	JCG# 5	KEN1# 3	KEN2# 0	KRIS 0



## APPENDIX F

### IDENTIFICATION OF GAGES FOR SABINE-CALCASIEU ASTROTIDE CALIBRATION

Gages for Sabine-Calcasieu astrotide calibration and time sequences of accepted astrotide simulation at those gages for 72 hours are identified. Also included are listings of the channel output at  $t = 30, 60$ , and  $90$  hours. For explanation of each column see Appendix C, 7, b.

ASTRO TIDE CALIBRATION FOR SABINE-CALCASIEU AREA

SABINE PASS TIDES USED AS INPUT

PERIOD OF RECORD= 0000 AUG,22 TO 2400 AUG,26,1973

CALCULATIONS ALLOW FOR SUB-GRID SCALE CHANNELS AND BARRIERS

TIME SEQUENCES OF WATER LEVEL AND FLOW ARE SAVED FOR THE FOLLOWING PLACES=

GAGE 1 SABINE PASS, SOUTHWEST JETTY

GAGE 2 PORT ARTHUR, CE AREA OFFICE

GAGE 3 NORTH SABINE LAKE

GAGE 4 BEAUMONT, NECHES RIVER AND BRAKES BAYOU

GAGE 5 ORANGE NAVAL STATION, SABINE RIVER

GAGE 6 CAMERON, CALCASIEU PASS

GAGE 7 HACKBERRY, CALCASIEU RIVER AND PASS

GAGE 8 I.W.K. AT CALCASIEU LOCK, WEST

GAGE 9 LAKE CHARLES, CALCASIEU RIVER

FLOW 1 SABINE PASS INFLOW

FLOW 2 CALCASIEU PASS INFLOW

FLOW 3 FLOW TO NECHES RIVER FROM SABINE LAKE AND INTRACOASTAL WATERWAY

FLOW 4 EASTWARD FLOW VIA INTRACOASTAL CANAL JUST EAST OF SABINE RIVER

FLOW 5 FLOW TO SABINE RIVER FROM SABINE LAKE AND INTRACOASTAL WATERWAY

FLOW 6 FLOW TO CALCASIEU RIVER FROM CALCASIEU LAKE AND INTRACOASTAL W.







## CHANNEL REACH 8

100	.01	4	0.000	0.	0.	.255	0.	.075	.254	0.	0.	0.	0.
101	2	4	0.000	0.	0.	.254	.075	.151	.252	0.	0.	0.	0.
102	3	4	0.000	0.	0.	.252	.151	.230	.248	0.	0.	0.	0.
103	3	5	.248	.230	.310	0.000	0.	0.	.244	0.	0.	0.	0.
104	4	5	0.000	0.	0.	.244	.310	.2015	.251	0.	0.	0.	0.
105	5	5	0.000	0.	0.	.251	.2015	.2092	.272	0.	0.	0.	1651.
106	5	6	.272	.2092	.2163	0.000	0.	0.	.294	0.	0.	0.	0.
107	5	7	.294	0.	0.	0.000	0.	0.	.178	0.	0.	0.	0.
108	5	8	.178	.7511	.3773	.150	.280	.3773	.154	.3711	0.	.3434.	0.
109	4	8	0.000	0.	0.	.143	0.	0.	.150	0.	0.	0.	0.
110	4	9	.150	.280	.241	.145	.208	.221	.147	0.	0.	0.	0.
111	3	9	.143	.077	.105	0.000	0.	0.	.145	0.	0.	0.	0.
112	3	8	0.000	0.	0.	.143	.050	.077	.143	0.	0.	0.	0.
113	2	8	0.000	0.	0.	.142	.025	.050	.143	0.	0.	0.	0.
114	.01	8	0.000	0.	0.	.142	0.	.25	.142	0.	0.	0.	0.

## CHANNEL REACH 9

115	3	10	.145	103	.74	.142	.48	.74	.143	0.	0.	0.	0.
116	2	10	0.000	0.	0.	0.000	0.	0.	.142	0.	0.	0.	0.
117	2	11	.142	.48	.23	0.000	0.	0.	.142	0.	0.	0.	0.
118	.02	12	.142	.23	0.	0.000	0.	0.	.141	0.	0.	0.	0.

## CHANNEL REACH 10

119	6	8	.319	3481	3383	.154	0.	0.	.317	0.	0.	0.	0.
120	.07	8	.332	0.	0.	.317	3383	0.	.314	0.	0.	0.	3354.

## CHANNEL REACH 11

121	.07	9	.344	65342	61083	.334	0.	0.	.296	.3488	0.	0.	0.
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VOLUME OF WATER ABOVE MSL = 1535.4 MILLIONS OF CU FT  
(THE SEAWARD POTS THRU J# 2 ARE EXCLUDED)

## CHANNEL OUTPUT FOR HOURS 60

NTIMES 900

ALL H VALUES IN FEET, ALL Q VALUES IN CFS

K	I	J	HX	QXN	QXP	HY	QYN	QYP	HC	QXT	QXF	QYT	QYF
%													
CHANNEL REACH 1													
1	.08	1	.530	32086	32799	0.000	0.	0.	.544	0.	0.	0.	0.
2	8	2	.544	32799	33386	0.000	0.	0.	.557	0.	0.	0.	0.
3	8	3	.557	33386	33998	.578	.3562	.33998	.569	0.	=0.	0.	0.
4	7	3	0.000	0.	0.	0.000	0.	0.	.578	0.	0.	0.	=1100.
5	7	4	.578	35602	.36317	.589	.12523	.12459	.564	0.	=369	0.	0.
6	6	4	0.000	0.	0.	0.000	0.	0.	.589	0.	0.	0.	0.
7	6	5	.589	12523	.11319	.561	0.	0.	.592	=1242	0.	0.	0.
8	6	6	.592	11319	.11298	.568	.2377	.2398	.588	0.	0.	0.	0.
9	6	7	.588	.8900	.8842	.488	.6602	.6634	.581	0.	0.	0.	0.
10	.07	9	0.000	0.	0.	.581	.35	.50	.579	0.	0.	0.	0.
11	8	7	0.000	0.	0.	.579	.50	.154	.577	0.	0.	0.	0.
12	8	8	.577	.154	.272	.584	0.	0.	.574	0.	0.	0.	0.
13	9	8	0.000	0.	0.	.574	.272	.404	.572	0.	0.	0.	0.
14	9	9	.572	.404	.548	0.000	0.	0.	.569	0.	0.	0.	0.
15	9	10	.569	.548	.394	0.000	0.	0.	.568	0.	=313	0.	0.
16	9	11	.568	.904	.829	.567	.745	.829	.567	0.	0.	0.	0.
17	8	11	0.000	0.	0.	.567	.655	.745	.567	0.	0.	0.	0.
18	7	11	0.000	0.	0.	0.000	0.	0.	.567	0.	0.	0.	0.
19	7	12	.567	.655	.558	.567	.456	.558	.567	0.	0.	0.	0.
20	6	12	0.000	0.	0.	.568	.362	.456	.567	0.	0.	0.	0.
21	5	12	0.000	0.	0.	0.000	0.	0.	.568	0.	0.	0.	0.
22	5	13	.568	362	.265	0.000	0.	0.	.568	0.	0.	0.	0.
23	5	14	.568	265	.165	.570	.63	.165	.569	0.	0.	0.	0.
24	4	14	0.000	0.	0.	0.000	0.	0.	.570	0.	0.	0.	0.
25	4	15	.570	.63	.27	0.000	0.	0.	.572	0.	0.	0.	0.
26	5	15	.569	0.	0.	.572	.27	.149	.572	0.	0.	0.	0.
27	5	16	.572	.149	.272	.570	.397	.272	.573	0.	0.	0.	0.
28	4	16	.572	0.	0.	0.000	0.	0.	.574	0.	0.	0.	0.
29	4	17	.574	.397	.522	.576	.114	.161	.575	0.	0.	0.	0.
30	5	17	.573	0.	0.	.575	.683	.778	.576	0.	0.	0.	0.
31	5	18	.576	.778	.906	.578	.103	.906	.577	0.	0.	0.	0.
32	4	18	.575	0.	0.	.577	0.	0.	.578	0.	0.	0.	0.
33	.04	19	.578	.1003	.1100	0.000	0.	0.	.578	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	17	0.000	0.	0.	0.000	0.	0.	.576	0.	0.	0.	0.
35	3	18	.576	.114	.65	.578	.33	.65	.577	0.	0.	0.	0.
36	.02	18	0.000	0.	0.	.578	0.	.33	.578	0.	0.	0.	0.



## CHANNEL REACH 8

100	-1	4	0.000	0.	0.	.551	0.	*53.	.551	0.	0.	0.	0.
101	2	4	0.000	0.	0.	.551	*53.	=106.	.551	0.	0.	0.	0.
102	3	4	0.000	0.	0.	.551	*106.	=156.	.551	0.	0.	0.	0.
103	3	5	.551	*156.	*205.	0.000	0.	0.	.551	0.	0.	0.	0.
104	4	5	0.000	0.	0.	.551	*205.	*2305.	.553	0.	0.	0.	0.
105	5	5	0.000	0.	0.	.553	*2305.	*2345.	.561	0.	0.	0.	0.
106	5	6	.561	*2345.	*2377.	0.000	0.	0.	.568	0.	0.	0.	0.
107	5	7	.568	*2377.	0.	0.000	0.	0.	.488	0.	0.	0.	0.
108	5	8	.488	6602.	3371.	.474	*254.	*3371.	.475	*3215.	0.	*3075.	0.
109	4	8	0.000	0.	0.	.475	0.	0.	.474	0.	0.	0.	0.
110	4	9	.474	254.	224.	.475	*196.	*224.	.474	0.	0.	0.	0.
111	3	9	.475	*73.	*98.	0.000	0.	0.	.475	0.	0.	0.	0.
112	3	8	0.000	0.	0.	.475	*68.	*73.	.475	0.	0.	0.	0.
113	2	8	0.000	0.	0.	.475	*24.	*48.	.475	0.	0.	0.	0.
114	-1	8	0.000.	0.	0.	.475	0.	*24.	.475	0.	0.	0.	0.

## CHANNEL REACH 9

115	3	10	.475	98.	72.	.475	*47.	*72.	.475	0.	0.	0.	0.
116	2	10	0.000	0.	0.	0.000	0.	0.	.475	0.	0.	0.	0.
117	2	11	.475	47.	23.	0.000	0.	0.	.475	0.	0.	0.	0.
118	-2	12	.475	23.	0.	0.000	0.	0.	.475	0.	0.	0.	0.

## CHANNEL REACH 10

119	6	8	.581	2172.	2135.	.475	0.	0.	.583	0.	0.	0.	0.
120	-7	8	.579	0.	0.	.583	2115.	0.	.584	0.	0.	0.	2121.

## CHANNEL REACH 11

121	-7	5	.584	23858.	23963.	.592	0.	0.	.585	30.	0.	0.	0.
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VOLUME OF WATER ABOVE MSL = 3437.0 MILLIONS OF CU FT  
(THE SEAWARD RDS THRU J# 2 ARE EXCLUDED)

## CHANNEL OUTPUT FOR HOUR 90

NTIME# 1350

ALL H VALUES IN FEET, ALL Q VALUES IN CFS

K	I	J	HX	QXN	QXP	HY	QYN	QYP	HC	QXT	QXF	QYT	QYF
CHANNEL REACH 1													
1	-8	1	-.710-110067.-106011.	0.000	0.	0.	-.411	0.	0.	0.	0.	0.	0.
2	8	2	-.411-110011.-102963.	0.000	0.	0.	-.131	0.	0.	0.	0.	0.	0.
3	8	3	-.131-102963.-100004.	.211	83895.	100004.	.049	0.	0.	0.	0.	0.	*13912.
4	7	3	0.000	0.	0.	0.000	0.	0.	.211	0.	0.	0.	0.
5	7	4	.211	*83895.	*85742.	.418	11810.	12451.	.361	0.	0.	0.	0.
6	6	4	0.000	0.	0.	0.000	0.	0.	.418	0.	0.	0.	0.
7	6	5	.418	*11810.	*6459.	.617	0.	0.	.469	4763.	0.	0.	0.
8	6	6	.469	*6459.	*5977.	.569	46.	185.	.519	0.	0.	0.	0.
9	6	7	.519	*5972.	*5358.	.604	*3537.	*3402.	.569	0.	0.	0.	0.
10	7	7	0.000	0.	0.	.569	*8623.	*8214.	.592	0.	0.	0.	0.
11	8	7	0.000	0.	0.	.592	*8214.	*7809.	.613	0.	0.	0.	0.
12	8	8	.613	*7809.	*7405.	.604	0.	0.	.633	0.	0.	0.	0.
13	9	8	0.000	0.	0.	.633	*7405.	*7002.	.653	0.	0.	0.	0.
14	9	9	.653	*7002.	*6603.	0.000	0.	0.	.671	0.	0.	0.	0.
15	9	10	.671	*6603.	*4292.	0.000	0.	0.	.685	0.	*1915.	0.	0.
16	9	11	.685	*3181.	*3010.	.710	2845.	3010.	.696	0.	0.	0.	0.
17	8	11	0.000	0.	0.	.721	2687.	2845.	.710	0.	0.	0.	0.
18	7	11	0.000	0.	0.	0.000	0.	0.	.721	0.	0.	0.	0.
19	7	12	.721	*2687.	*2535.	.740	2390.	2535.	.731	0.	0.	0.	0.
20	6	12	0.000	0.	0.	.747	2269.	2340.	.740	0.	0.	0.	0.
21	5	12	0.000	0.	0.	0.000	0.	0.	.747	0.	0.	0.	0.
22	5	13	.747	*2269.	*2153.	0.000	0.	0.	.754	0.	0.	0.	0.
23	5	14	.754	*2153.	*2044.	.767	1939.	2044.	.761	0.	0.	0.	0.
24	4	14	0.000	0.	0.	0.000	0.	0.	.767	0.	0.	0.	0.
25	4	15	.767	*1939.	*1858.	0.000	0.	0.	.779	0.	0.	0.	0.
26	5	15	.781	0.	.779	*1858.	*1756.	.783	0.	0.	0.	0.	0.
27	5	16	.783	*1756.	*1658.	.792	1565.	1658.	.787	0.	0.	0.	0.
28	4	16	.779	0.	0.	0.000	0.	0.	.792	0.	0.	0.	0.
29	4	17	.792	*1565.	*1476.	.798	71.	103.	.796	0.	0.	0.	0.
30	5	17	.787	0.	0.	.796	*1373.	*1349.	.799	0.	0.	0.	0.
31	5	18	.799	*1309.	*1225.	.802	1162.	1225.	.800	0.	0.	0.	0.
32	4	18	.796	0.	0.	.800	0.	0.	.802	0.	0.	0.	0.
33	-4	19	.802	*1162.	*1100.	0.000	0.	0.	.803	0.	0.	0.	0.
CHANNEL REACH 2													
34	3	17	0.000	0.	0.	0.000	0.	0.	.798	0.	0.	0.	0.
35	3	18	.798	*71.	*40.	.802	20.	40.	.800	0.	0.	0.	0.
36	-2	18	0.000	0.	0.	.802	0.	20.	.802	0.	0.	0.	0.



## CHANNEL REACH 8

100	-1	4	0.000	0.	0.	.703	0.	.69.	.702	0.	0.	0.	0.
101	2	4	0.000	0.	0.	.702	.69.	.138.	.699	0.	0.	0.	0.
102	3	4	0.000	0.	0.	.699	.138.	.208.	.695	0.	0.	0.	0.
103	3	5	.695	208.	280.	0.000	0.	0.	.688	0.	0.	0.	0.
104	4	5	0.000	0.	0.	.688	.280.	.161.	.662	0.	0.	0.	0.
105	5	5	0.000	0.	0.	.662	.161.	.161.	.617	0.	0.	0.	497.
106	5	6	.617	.68.	.40.	0.000	0.	0.	.569	0.	0.	0.	0.
107	5	7	.569	0.	0.	0.000	0.	0.	.604	0.	0.	0.	0.
108	5	8	.604	3537.	1958.	.649	.71.	.1958.	.635	.1643.	0.	0.	0.
109	4	8	0.000	0.	0.	.690	0.	0.	.649	0.	0.	0.	0.
110	4	9	.649	.71.	.56.	.662	.45.	.56.	.665	0.	0.	0.	0.
111	3	9	.690	.16.	.23.	0.000	0.	0.	.682	0.	0.	0.	0.
112	3	8	0.000	0.	0.	.695	.10.	.16.	.690	0.	0.	0.	0.
113	2	8	0.000	0.	0.	.698	.5.	.10.	.695	0.	0.	0.	0.
114	-1	8	0.000	0.	0.	.699	0.	.5.	.698	0.	0.	0.	0.

## CHANNEL REACH 9

115	3	10	.682	.22.	.14.	.695	8.	.14.	.690	0.	0.	0.	0.
116	2	10	0.000	0.	0.	0.000	0.	0.	.695	0.	0.	0.	0.
117	2	11	.695	.08.	.04.	0.000	0.	0.	.698	0.	0.	0.	0.
118	-2	12	.698	.04.	0.	0.000	0.	0.	.700	0.	0.	0.	0.

## CHANNEL REACH 10

119	6	8	.569	.177.	.3.	.635	0.	0.	.587	0.	0.	0.	0.
120	-7	8	.592	0.	0.	.587	3.	0.	.604	0.	0.	0.	93.

## CHANNEL REACH 11

121	-7	5	.361	.73291.	.61573.	.469	0.	0.	.566	10634.	0.	0.	0.
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VOLUME OF WATER ABOVE MSL = 4120.4 MILLIONS OF CU FT  
(THE SEAMARD ROWS THRU J# 2 ARE EXCLUDED)

## APPENDIX G

### IDENTIFICATION OF GAGES FOR SABINE-CALCASIEU HURRICANE CARLA VERIFICATION

Gages for Sabine-Calcasieu Hurricane Carla verification and time sequences of water level and flow at the identified gage for 60 hours are identified. Also included are listings of detailed channel output at 30 and 60 hours. For explanation of each column see Appendix C,7,b.

HURRICANE CARLA CALIBRATION FOR SABINE-CALCASIEU AREA

PERIOD OF RECORD= 0000 SEP 10 TO 0000 SEP 13, 1901

CALCULATIONS ALLOW FOR SUR-GRID SCALE CHANNELS AND BARRIERS

TIME SEQUENCES OF WATER LEVEL AND FLOW ARE SAVED FOR THE FOLLOWING PLACES=

GAGE 1 SABINE PASS, SOUTHWEST JETTY

GAGE 2 PORT ARTHUR, CE AREA OFFICE

GAGE 3 NORTH SABINE LAKE

GAGE 4 BEAUMONT, NECHES RIVER AND BRAKES BAYOU

GAGE 5 ORANGE NAVAL STATION, SABINE RIVER

GAGE 6 CAMERON, CALCASIEU PASS

GAGE 7 WEST END OF INTRACOASTAL WATERWAY

GAGE 8 SABINE PASS, COAST GUARD STATION

GAGE 9 LAKE CHARLES, CALCASIEU RIVER

FLOW 1 SABINE PASS INFLOW

FLOW 2 CALCASIEU PASS INFLOW

WATER LEVEL HYDROGRAPHS (FT) AND KEY FLOWS (1000 CFS)

HOUR	1	2	3	4	5	6	7	8	9	1	2
0.0	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20	3.20	0.00	0.00
1.0	4.83	3.08	3.16	3.27	3.25	4.49	3.55	4.58	3.08	103.46	92.03
2.0	5.17	3.42	3.15	3.30	3.24	4.50	3.75	4.41	3.14	146.22	128.51
3.0	5.50	3.54	3.59	3.32	3.21	4.67	3.88	4.88	2.97	132.56	171.46
4.0	5.73	3.65	3.80	3.41	3.19	4.79	3.94	5.03	2.86	160.98	109.86
5.0	5.97	3.76	3.95	3.53	3.17	4.93	4.04	5.20	2.89	177.48	216.70
6.0	6.20	3.91	4.16	3.60	3.20	5.08	4.19	5.43	2.83	179.66	233.63
7.0	6.13	4.09	4.36	3.83	3.20	5.09	4.38	5.51	2.90	169.05	235.17
8.0	6.07	4.33	4.57	3.96	3.22	5.03	4.53	5.57	2.88	158.33	233.57
9.0	6.00	4.43	4.73	4.12	3.26	4.97	4.70	5.59	2.91	143.54	231.81
10.0	5.93	4.68	4.92	4.28	3.31	4.91	4.90	5.65	2.94	124.36	229.76
11.0	5.87	5.02	5.07	4.44	3.39	4.86	4.97	5.68	2.92	111.24	225.86
12.0	5.80	5.01	5.21	4.62	3.49	4.79	5.30	5.72	2.94	90.76	223.02
13.0	5.93	5.14	5.34	4.82	3.62	4.81	5.33	5.85	2.92	79.31	229.31
14.0	6.07	5.23	5.43	4.99	3.77	4.86	5.52	6.00	2.89	64.82	239.04
15.0	6.20	5.34	5.56	5.11	3.92	4.93	5.71	6.15	2.85	55.39	248.26
16.0	6.30	5.45	5.09	5.16	4.02	4.95	5.87	6.27	2.80	46.64	257.14
17.0	6.40	5.54	5.83	5.23	4.15	4.95	5.94	6.37	2.79	42.11	269.48
18.0	6.50	5.64	5.45	5.30	4.32	4.99	6.05	6.47	2.79	42.56	240.16
19.0	6.40	5.72	6.04	5.36	4.44	4.93	6.16	6.46	2.82	34.18	242.00
20.0	6.30	5.80	6.19	5.42	4.58	4.85	6.26	6.44	2.91	23.28	240.14
21.0	6.20	5.85	6.26	5.47	4.77	4.76	6.37	6.40	3.03	3.39	278.99
22.0	6.13	5.89	6.30	5.57	4.94	4.69	6.04	6.37	3.18	-21.15	279.25
23.0	6.07	5.90	6.31	5.68	5.12	4.57	6.45	6.29	3.32	-42.18	286.38
24.0	6.06	5.92	6.30	5.61	5.23	4.51	6.48	6.24	3.44	-53.90	284.42
25.0	6.40	5.96	6.31	5.94	5.37	4.88	6.58	6.38	3.55	-27.67	293.24
26.0	6.80	6.06	6.40	6.06	5.63	5.00	6.81	6.70	3.66	16.03	312.52
27.0	7.20	6.35	6.03	6.17	5.89	5.28	7.07	7.07	3.82	74.96	330.25
28.0	7.13	6.56	6.82	6.32	6.04	5.36	7.26	7.14	3.98	93.92	328.14
29.0	7.07	6.73	6.94	6.57	6.08	5.35	7.40	7.16	4.10	99.89	322.78
30.0	7.00	6.86	6.98	6.87	6.20	5.33	7.57	7.10	4.18	92.34	317.63
31.0	6.60	6.93	7.01	7.22	6.42	5.15	7.67	6.90	4.26	62.47	300.85
32.0	6.20	6.97	7.06	7.30	6.63	4.94	7.71	6.70	4.36	21.24	278.01
33.0	5.80	6.98	7.02	7.07	6.77	4.73	7.74	6.45	4.48	-51.13	255.06
34.0	5.93	6.98	6.96	7.59	6.83	4.72	7.70	6.50	4.61	-93.79	256.07
35.0	6.07	7.00	6.92	7.65	6.83	4.79	7.66	6.58	4.68	-118.94	258.51
36.0	6.20	7.07	6.97	7.67	6.83	4.88	7.60	6.66	4.71	-120.59	269.57
37.0	6.33	7.11	7.05	7.71	6.89	4.98	7.59	6.76	4.72	-112.30	278.67
38.0	6.47	7.16	7.19	7.78	7.01	5.09	7.65	6.86	4.76	-98.98	286.56
39.0	6.60	7.32	7.38	7.90	7.23	5.18	7.71	6.90	4.89	-80.85	294.90
40.0	6.33	7.45	7.53	8.03	7.42	5.12	7.79	6.90	5.08	-90.35	290.06
41.0	6.07	7.54	7.60	8.17	7.51	5.03	7.92	6.81	5.28	-109.32	279.24
42.0	5.80	7.56	7.59	8.30	7.65	4.94	7.97	6.88	5.40	-141.29	267.21
43.0	5.30	7.57	7.59	8.42	7.74	4.76	8.00	6.47	5.44	-178.72	247.17
44.0	4.80	7.55	7.62	8.51	7.89	4.51	8.00	6.24	5.42	-223.01	229.47
45.0	4.30	7.52	7.62	8.61	7.96	4.24	7.98	6.07	5.41	-269.28	210.82
46.0	4.40	7.53	7.60	8.73	8.02	4.14	7.92	6.05	5.42	-265.19	208.77
47.0	4.50	7.55	7.61	8.84	8.06	4.13	7.82	6.11	5.42	-261.43	214.50
48.0	4.60	7.59	7.68	8.95	8.13	4.04	7.71	6.11	5.41	-240.30	221.54
49.0	4.87	7.62	7.73	9.07	8.24	4.22	7.57	6.23	5.41	-220.05	234.07
50.0	5.13	7.65	7.77	9.20	8.37	4.38	7.38	6.34	5.43	-190.80	255.91
51.0	5.40	7.70	7.87	9.31	8.56	4.56	7.21	6.46	5.47	-175.81	277.15
52.0	5.17	7.71	7.91	9.42	8.80	4.51	7.03	6.31	5.53	-182.30	274.94
53.0	4.93	7.69	7.81	9.52	8.82	4.43	6.98	6.14	5.63	-196.34	261.53
54.0	4.70	7.63	7.63	9.57	8.81	4.40	6.92	5.98	5.70	-210.66	242.76
55.0	4.20	7.57	7.50	9.53	8.72	4.28	6.88	5.82	5.79	-250.55	217.02
56.0	3.70	7.47	7.46	9.40	8.67	4.14	6.61	5.51	5.90	-267.89	177.37
57.0	3.20	7.38	7.39	9.27	8.69	3.93	6.25	5.05	6.13	-278.51	130.74
58.0	3.03	7.30	7.27	9.17	8.79	3.85	5.97	4.91	6.43	-264.63	102.29
59.0	2.87	7.16	7.04	9.06	8.85	3.91	5.84	4.76	6.69	-266.28	54.85

CHANNEL OUTPUT FOR MONTHS 30

NTIMES 450

ALL H VALUES IN FEET, ALL Q VALUES IN CFS

K	I	J	HY	QXN	QXP	HY	QYN	QYP	HC	QXT	QXF	GYT	QYF
<b>CHANNEL REACH 1</b>													
1	8	1	7.000	92340.	90785.	3.200	0.	0.	7.039	+25145.	+23084.	0.	0.
2	8	2	7.059	90785.	95512.	3.200	0.	0.	7.071	+68402.	+52865.	0.	0.
3	8	3	7.071	95512.	124170.	7.156	+115427.	+124170.	7.101	+52811.	+81663.	32926.	41853.
4	7	3	3.218	0.	0.	3.200	0.	0.	7.156	0.	0.	0.	0.
5	7	4	7.156	115427.	00525.	7.162	+81405.	+100131.	7.191	+55840.	+282.	+4157.	+25058.
6	6	4	3.200	0.	0.	3.200	0.	0.	7.162	0.	0.	0.	0.
7	6	5	7.162	81405.	70369.	7.147	0.	0.	7.135	+47279.	+36036.	0.	0.
8	6	6	7.135	70369.	67385.	7.167	+3989.	+7855.	7.078	+45047.	+4299.	16013.	19912.
9	6	7	7.078	59541.	63865.	7.090	602.	+3562.	6.956	+64978.	+49653.	34173.	38289.
10	7	7	3.200	0.	0.	0.958	+14807.	+15132.	6.906	0.	0.	0.	0.
11	8	7	3.218	0.	0.	6.906	+15132.	+15494.	6.857	0.	0.	0.	0.
12	8	8	6.857	+15494.	+15862.	3.340	0.	0.	6.877	0.	0.	0.	0.
13	9	8	3.200	0.	0.	6.877	+15862.	+16177.	6.832	0.	0.	0.	0.
14	9	9	6.832	+16177.	+16428.	3.200	0.	0.	6.847	0.	0.	0.	0.
15	9	10	6.847	+16428.	+7211.	3.200	0.	0.	6.863	0.	+9546.	0.	0.
16	9	11	6.863	+4296.	2074.	6.925	+8744.	+2074.	6.887	+92762.	+99309.	98131.	91328.
17	8	11	3.200	0.	0.	6.958	+14445.	+8744.	6.925	0.	0.	-5916.	0.
18	7	11	3.200	0.	0.	3.200	0.	0.	6.958	0.	0.	0.	0.
19	7	12	6.958	14485.	18298.	6.969	+18199.	+18298.	6.974	+33927.	+37896.	25680.	25087.
20	6	12	3.200	0.	0.	6.997	+19635.	+18919.	6.989	0.	0.	-926.	0.
21	5	12	3.200	0.	0.	3.200	0.	0.	6.997	0.	0.	0.	0.
22	5	13	6.997	19635.	20504.	3.200	0.	0.	6.987	0.	+976.	0.	0.
23	5	14	6.987	20504.	22691.	6.981	+22657.	+22691.	6.976	+45761.	+48437.	39196.	38930.
24	4	14	3.200	0.	0.	3.200	0.	0.	6.981	0.	0.	0.	0.
25	4	15	6.981	22657.	20181.	3.200	0.	0.	6.874	0.	2270.	0.	0.
26	5	15	6.976	0.	0.	6.874	+20181.	+18747.	6.866	0.	0.	29302.	30322.
27	5	16	6.846	18747.	15432.	6.844	+149400.	+15432.	6.845	+17510.	+20478.	-92.	0.
28	4	16	6.874	0.	0.	3.200	0.	0.	6.844	0.	0.	0.	0.
29	4	17	6.844	14940.	629.	6.843	+312.	+455.	6.840	+14055.	0.	0.	0.
30	5	17	6.845	0.	0.	6.840	173.	+130.	6.811	0.	0.	0.	0.
31	5	18	6.811	+130.	+539.	6.839	R00.	539.	6.816	0.	0.	0.	0.
32	4	18	6.840	0.	0.	6.806	0.	0.	6.839	0.	0.	0.	0.
33	-4	19	6.839	+846.	+1152.	3.200	0.	0.	6.847	0.	0.	0.	0.
<b>CHANNEL REACH 2</b>													
34	3	17	3.200	0.	0.	3.200	0.	0.	6.863	0.	0.	0.	0.
35	3	18	6.863	312.	174.	6.892	+85.	+174.	6.866	0.	0.	0.	0.
36	-2	18	3.200	0.	0.	6.923	0.	+85.	6.892	0.	0.	0.	0.
<b>CHANNEL REACH 3</b>													
37	10	10	3.200	0.	0.	6.863	+2915.	+208.	6.795	0.	0.	53830.	51051.
38	11	10	3.200	0.	0.	6.745	+248.	2148.	6.720	0.	0.	26697.	22353.
39	12	10	3.200	0.	0.	6.720	2148.	4322.	6.617	0.	0.	39781.	37519.
40	12	11	6.517	4322.	6078.	6.721	5033.	8748.	6.618	+23417.	+25233.	-9157.	-12971.
41	13	11	3.200	0.	0.	6.618	14824.	16063.	6.436	0.	0.	26771.	24830.
42	14	11	3.200	0.	0.	6.436	16063.	17614.	6.221	0.	0.	90991.	97048.
43	14	12	6.221	+1396.	+1719.	6.235	653.	1709.	6.222	+85475.	+85711.	17485.	16328.
44	13	12	6.436	0.	0.	3.200	0.	0.	6.235	0.	+518.	0.	0.
45	13	13	6.735	+653.	+252.	3.200	0.	0.	6.236	0.	+518.	0.	0.
46	14	13	6.222	0.	0.	6.236	+252.	206.	6.204	0.	0.	689.	0.
47	14	14	6.204	206.	+1660.	3.200	0.	0.	6.222	0.	1812.	0.	0.
48	15	14	3.200	0.	0.	6.222	+1666.	+2165.	6.191	0.	0.	10228.	10285.
49	15	15	6.191	+2165.	+1245.	6.235	930.	1205.	6.207	1161.	0.	470.	0.
50	12	15	6.222	0.	0.	3.200	0.	0.	6.235	0.	0.	0.	0.
51	14	16	6.735	+930.	+1113.	3.200	0.	0.	6.245	0.	0.	0.	0.
52	14	17	6.245	+1113.	+1325.	3.200	0.	0.	6.259	0.	0.	0.	0.
53	14	18	6.259	+1325.	+1440.	3.200	0.	0.	6.309	0.	0.	0.	0.
54	-14	19	6.309	+1440.	+1560.	3.200	0.	0.	6.373	0.	0.	0.	0.
<b>CHANNEL REACH 4</b>													
55	11	11	6.720	0.	0.	3.200	0.	0.	6.721	0.	0.	0.	0.
56	11	12	6.721	+6033.	+606.	6.768	172.	006.	6.750	0.	+4408.	361.	0.
57	10	12	3.200	0.	0.	3.200	0.	0.	6.768	0.	0.	0.	0.
58	10	13	6.768	+172.	+82.	6.829	36.	82.	6.801	0.	0.	0.	0.
59	9	13	3.200	0.	0.	3.200	0.	0.	6.829	0.	0.	0.	0.
60	-9	14	6.829	+36.	0.	3.200	0.	0.	6.840	0.	0.	0.	0.
<b>CHANNEL REACH 5</b>													
61	15	11	3.200	0.	0.	6.221	19010.	17622.	5.953	0.	0.	0.	1298.
62	16	11	3.200	0.	0.	5.953	17622.	15606.	5.692	0.	0.	0.	1711.
63	17	11	3.200	0.	0.	5.692	15606.	15608.	5.437	0.	0.	0.	0.
64	18	11	3.200	0.	0.	5.437	15608.	15512.	5.182	0.	0.	0.	0.
65	19	11	6.720	+12460.	+15395.	5.162	15512.	15385.	5.927	0.	2942.	0.	0.
66	19	10	3.200	0.	0.	3.200	0.	0.	6.729	0.	0.	0.	0.
67	20	10	3.200	0.	0.	4.729	12460.	12246.	6.506	0.	0.	0.	117.
68	21	10	3.200	0.	0.	4.546	12246.	4617.	4.419	0.	0.	0.	7718.
69	22	10	3.200	0.	0.	4.419	4617.	4511.	4.327	0.	0.	0.	0.
70	23	10	6.192	+534.	5173.	4.327	4511.	7738.	4.206	0.	0.	0.	+3400.

## CHANNEL REACH 6

71	-22	1	7.000	317627.	335432.	3.200	0.	0.	6.062	+14111.	+32523.	0.	0.
72	23	1	3.200	0.	0.	6.092	114327.	99863.	5.334	0.	0.	32077.	47784.
73	23	2	5.334	99863.	45294.	5.135	9628.	-45264.	5.072	9258.	67216.	42334.	138703.
74	22	2	6.062	221125.	181244.	3.200	0.	0.	5.135	0.	41793.	0.	0.
75	22	3	5.135	171616.	136769.	3.200	0.	0.	4.768	0.	36195.	0.	0.
76	22	4	4.768	134769.	91854.	3.200	0.	0.	4.318	+27878.	19793.	0.	0.
77	22	5	4.318	91854.	62016.	3.200	0.	0.	4.265	+121662.	-92366.	0.	0.
78	23	5	3.200	0.	0.	4.265	62618.	53652.	4.214	0.	0.	91177.	100122.
79	23	6	4.214	53652.	46660.	3.200	0.	0.	4.205	+6737.	0.	0.	0.
80	23	7	4.205	46660.	41436.	3.200	0.	0.	4.201	+5363.	0.	0.	0.
81	23	8	4.201	41436.	41319.	3.200	0.	0.	4.198	0.	0.	0.	0.
82	23	9	4.198	41319.	31814.	3.200	0.	0.	4.192	+9464.	0.	0.	0.
83	24	9	3.200	0.	0.	4.192	26510.	18343.	4.149	0.	0.	+14730.	+6813.
84	24	10	4.149	18343.	11745.	4.200	12911.	9212.	4.164	+34137.	+27870.	23952.	27297.
85	24	11	4.164	53333.	6483.	3.200	0.	0.	4.183	447.	0.	0.	0.
86	24	12	4.183	6483.	13347.	3.200	0.	0.	4.206	7214.	0.	0.	0.
87	24	13	4.206	13347.	12997.	3.200	0.	0.	4.225	0.	0.	0.	0.
88	25	13	3.200	0.	0.	4.225	12997.	12064.	4.204	0.	0.	0.	0.
89	25	14	4.204	12064.	10648.	3.200	0.	0.	4.220	0.	1897.	0.	0.
90	26	14	3.200	0.	0.	4.220	10648.	6910.	4.196	0.	0.	+3524.	0.
91	27	14	3.200	0.	0.	4.196	6910.	442.	4.168	0.	0.	+7142.	0.
92	27	15	4.158	+662.	+683.	3.200	0.	0.	4.184	0.	0.	0.	0.
93	+28	15	3.200	0.	0.	4.184	+683.	-900.	4.155	0.	0.	0.	0.

## CHANNEL REACH 7

94	25	10	3.200	0.	0.	4.154	14665.	13275.	3.928	0.	0.	+1345.	0.
95	26	10	3.608	+903.	+11328.	3.928	13275.	11328.	3.736	+11871.	-10231.	+1907.	0.
96	26	9	3.200	0.	0.	3.200	0.	0.	3.608	0.	0.	0.	0.
97	27	9	3.459	+200.	+5435.	3.518	9603.	5435.	3.502	+5108.	0.	13392.	17490.
98	27	8	3.200	0.	0.	3.200	0.	0.	3.459	0.	0.	0.	0.
99	+28	8	3.200	0.	0.	3.459	200.	0.	3.397	0.	0.	0.	0.

## CHANNEL REACH 8

100	-1	4	3.200	0.	0.	7.603	0.	4713.	7.573	0.	0.	18851.	13965.
101	2	4	3.200	0.	0.	7.573	4713.	6561.	7.467	0.	0.	24550.	22500.
102	3	4	3.200	0.	0.	7.407	6561.	5027.	7.352	0.	0.	25048.	25845.
103	3	5	7.352	5627.	4621.	3.200	0.	0.	7.385	+25209.	+24380.	0.	0.
104	4	5	3.200	0.	0.	7.385	4621.	3341.	7.269	0.	0.	22611.	23746.
105	5	5	3.200	0.	0.	7.269	3341.	+208.	7.107	0.	0.	32412.	35683.
106	5	6	7.147	+248.	+3989.	3.200	0.	0.	7.167	+21252.	+17670.	0.	0.
107	5	7	7.167	0.	0.	3.200	0.	0.	7.090	0.	0.	0.	0.
108	5	8	7.090	+602.	+5247.	7.240	4687.	5207.	7.153	29979.	34510.	63646.	62858.
109	4	8	3.200	0.	0.	7.456	0.	0.	7.240	0.	0.	0.	0.
110	4	9	7.240	+607.	+3041.	7.412	94.	3041.	7.365	+32403.	+34062.	8351.	5303.
111	3	9	7.456	3557.	6915.	3.200	0.	0.	7.412	+42020.	+45538.	0.	0.
112	3	8	3.200	0.	0.	7.599	229.	3557.	7.456	0.	0.	+35191.	+38636.
113	2	8	3.200	0.	0.	7.732	270.	229.	7.599	0.	0.	0.	0.
114	-1	8	3.200	0.	0.	7.841	0.	270.	7.732	0.	0.	5977.	5706.

## CHANNEL REACH 9

115	3	10	7.412	6822.	10086.	7.268	+5073.	+10086.	7.325	0.	+3453.	0.	4156.
116	2	10	3.200	0.	0.	3.200	0.	0.	7.268	0.	0.	0.	0.
117	2	11	7.268	+973.	2157.	3.200	0.	0.	7.256	0.	3805.	0.	0.

## CHANNEL REACH 10

118	-7	8	6.906	0.	0.	6.023	68077.	78461.	5.390	0.	0.	3729.	-7978.
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VOLUME OF WATER ABOVE MSL = 162254.6 MILLIONS OF CU FT  
(THE SEA AND RLS THRU JK 2 AFE EXCLUDED)



## CHANNEL REACH 6

71	-22	1	3.475	+16073.	+15629.	3.200	0.	0.	3.856	0.	0.	0.	0.
72	23	1	3.200	0.	0.	3.856	33840.	+253.	0.007	0.	0.	0.	34916.
73	23	2	0.007	+253.	+28355.	0.196	43708.	28350.	0.162	32354.	60717.	4972.	20711.
74	22	2	3.456	+0.69.	+69333.	3.200	0.	0.	0.196	0.	20486.	0.	0.
75	22	3	0.196	+113.01.	+10587.	3.200	0.	0.	0.534	0.	+4476.	0.	0.
76	22	4	0.534	+10587.	+104670.	3.200	0.	0.	5.071	5752.	1513.	0.	0.
77	22	5	5.071	+104670.	+105541.	3.200	0.	0.	5.289	136440.	137361.	0.	0.
78	23	5	3.200	0.	0.	5.289	+105541.	+105977.	5.448	0.	0.	+103736.	+103290.
79	23	6	5.448	+105977.	+106455.	3.200	0.	0.	5.628	+415.	0.	0.	0.
80	23	7	5.628	+106455.	+100833.	3.200	0.	0.	5.800	5720.	0.	0.	0.
81	23	8	5.800	+100833.	+66750.	3.200	0.	0.	5.953	14151.	0.	0.	0.
82	23	9	5.953	+66750.	+65829.	3.200	0.	0.	6.080	21081.	0.	0.	0.
83	24	9	3.200	0.	0.	6.080	+21081.	+21081.	6.112	0.	0.	+117662.	+114253.
84	24	10	6.112	+25245.	+19486.	6.159	+24283.	+13677.	6.180	+70164.	-75321.	+45764.	+56227.
85	24	11	6.180	+21455.	+7980.	3.200	0.	0.	6.261	+14442.	+47852.	0.	0.
86	24	12	6.261	+7980.	14947.	3.200	0.	0.	6.368	22855.	0.	0.	0.
87	24	13	6.368	14947.	15121.	3.200	0.	0.	6.457	0.	0.	0.	0.
88	25	13	3.200	0.	0.	6.457	15121.	15264.	6.518	0.	0.	0.	0.
89	25	14	6.518	15264.	13196.	3.200	0.	0.	6.622	0.	2184.	0.	0.
90	26	14	3.200	0.	0.	6.622	13196.	6648.	6.678	0.	0.	+7210.	0.
91	27	14	3.200	0.	0.	6.678	6648.	+1312.	6.708	0.	0.	+7464.	0.
92	27	15	6.708	+1312.	+1310.	3.200	0.	0.	6.758	0.	0.	0.	0.
93	+28	15	3.200	0.	0.	6.758	+1310.	+1310.	6.770	0.	0.	0.	0.

## CHANNEL REACH 7

94	25	10	3.200	0.	0.	6.169	+12279.	+8707.	6.275	0.	0.	29729.	26265.
95	26	10	6.297	1269.	4078.	6.275	+8747.	+4078.	6.343	+40345.	+43033.	+3023.	+8250.
96	26	9	3.200	0.	0.	3.200	0.	0.	6.297	0.	0.	0.	0.
97	27	9	6.271	103.	+136.	6.297	+1289.	+136.	6.346	+351.	0.	46643.	45335.
98	27	8	3.200	0.	0.	3.200	0.	0.	6.271	0.	0.	0.	0.
99	+28	8	3.200	0.	0.	6.271	+103.	0.	6.295	0.	0.	0.	0.

## CHANNEL REACH 8

100	-1	4	3.200	0.	0.	5.751	0.	+5487.	5.802	0.	0.	-6551.	+958.
101	2	4	3.200	0.	0.	5.802	+547.	+9614.	5.883	0.	0.	-8345.	+4704.
102	3	4	3.200	0.	0.	5.803	+9014.	+10935.	5.992	0.	0.	+11431.	+9411.
103	3	5	5.992	+10935.	+49793.	3.200	0.	0.	6.173	14330.	13229.	0.	0.
104	4	5	3.200	0.	0.	6.173	+9703.	+6408.	6.265	0.	0.	-1830.	+21673.
105	5	5	3.200	0.	0.	6.205	+6408.	+2200.	6.338	0.	0.	-47662.	+51939.
106	5	5	6.338	+2200.	+3568.	3.200	0.	0.	6.466	31688.	25302.	0.	0.
107	5	7	6.466	0.	0.	3.200	0.	0.	7.273	0.	0.	0.	0.
108	5	8	7.273	+21326.	+18045.	7.400	+14077.	+18045.	7.395	+13961.	+16634.	+66581.	+49765.
109	4	8	3.200	0.	0.	7.850	0.	0.	7.400	0.	0.	0.	0.
110	4	9	7.400	+10477.	+13041.	7.937	+12990.	+13041.	7.686	+41371.	+40056.	+4913.	+4860.
111	3	9	7.650	+1072.	+3796.	3.200	0.	0.	7.937	+67047.	+69285.	0.	0.
112	3	8	3.200	0.	0.	7.859	1864.	+1072.	7.850	0.	0.	49891.	53586.
113	2	8	3.200	0.	0.	7.875	1221.	+1564.	7.859	0.	0.	15.	+487.
114	-1	8	3.200	0.	0.	7.867	0.	+1221.	7.875	0.	0.	18.	+1069.

## CHANNEL REACH 9

115	3	10	7.937	+16776.	+18740.	8.809	11104.	+18748.	8.522	550.	2676.	0.	+7978.
116	2	10	3.200	0.	0.	3.200	0.	0.	8.809	0.	0.	0.	0.
117	2	11	8.809	+11104.	+6372.	3.200	0.	0.	8.987	9948.	5134.	0.	0.
118	-2	12	8.987	+6372.	0.	3.200	0.	0.	9.062.	0.	+6380.	0.	0.

## CHANNEL REACH 10

119	6	8	6.925	+47088.	+36475.	7.395	0.	0.	7.171	+31386.	+42017.	0.	0.
120	-7	8	6.971	0.	0.	7.171	+36475.	+33139.	7.698	0.	+66590.	+69932.	

## CHANNEL REACH 11

121	-7	5	5.610	+152593.	+158599.	6.024	0.	0.	5.784	137602.	144301.	0.	0.
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VOLUME OF WATER ABOVE MSL = 195850.6 MILLIONS OF CU FT  
(THE SEAWARD ROAD THRU JR 2 ARE EXCLUDED)

SURGE II Program with application to the Sabine-Hurricane Carla and design hurricanes / by Robert O. Reid, Andrew C. Vastano...[et al.]. - Fort Belvoir, Va. : U.S. Coastal Center ; Springfield, Va. : available from National Technical Information Service, 1977.

Technical Paper - U.S. Coastal Engineering Research Center ; no. 77-13 (Contract - U.S. Coastal Engineering Research Center DACW64-74-C-0015)

7.

Program for calculation of storm surges and tides in a bay or estuary of the type where frictional resistance dominates over Coriolis force.

Hurricane Carla. I. Title. II. Title: SURGE II

Program. III. Vastano, Andrew C., joint author. IV. Series: U.S. Coastal Engineering Research Center. Technical paper no. 77-13. V.

Series: U.S. Coastal Engineering Research Center. Contract DACW64-74-C-0015.

TC203 .U581tp no. 77-13 627

Reid, Robert O.

Development of SURGE II Program with application to the Sabine-Calcasieu area for Hurricane Carla and design hurricanes / by Robert O. Reid, Andrew C. Vastano...[et al.]. - Fort Belvoir, Va. : U.S. Coastal Engineering Research Center ; Springfield, Va. : available from National Technical Information Service, 1977.

218 p. : ill. (Technical paper - U.S. Coastal Engineering Research Center ; no. 77-13) Also (Contract - U.S. Coastal Engineering Research Center DACW64-74-C-0015)

Bibliography: p. 117.

SURGE II is a program for calculation of storm surges and tides in a bay or estuary of the type where frictional resistance dominates over Coriolis force.

1. Hurricanes. 2. Hurricane Carla. I. Title. II. Title: SURGE II

Program. III. Vastano, Andrew C., joint author. IV. Series: U.S. Coastal Engineering Research Center. Technical paper no. 77-13. V.

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